

*Appendix E – Local Model Validation Report
(LMVR)*

REPORT N° 3

LAKE LOTHING THIRD CROSSING

LOCAL MODEL VALIDATION REPORT

CONFIDENTIAL

LAKE LOTHING THIRD CROSSING

LOCAL MODEL VALIDATION REPORT

Suffolk County Council

Type of document (version)
Confidential

Project no: 70012367

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QUALITY MANAGEMENT

ISSUE/REVISION	FIRST ISSUE	REVISION 1	REVISION 2	REVISION 3
Remarks				
Date	15/12/2015			
Prepared by	Michael Johns			
Signature				
Checked by	Alan Cowan			
Signature				
Authorised by	Craig Drennan			
Signature				
Project number	70012367			
Report number	3			
File reference	S:\70012367 - Lowestoft Traffic Model Update\C Documents\Reports\LMVR\151215 Lake Lothing Lowestoft LMVR.docx			

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1 INTRODUCTION

1.1 PROJECT BACKGROUND

- 1.1.1 WSP | Parsons Brinckerhoff has been commissioned by Suffolk County Council (SCC) to undertake traffic modelling in support of a Transport Business Case (TBC) for a third crossing of Lake Lothing in Suffolk.
- 1.1.2 There is an existing Lowestoft SATURN highway assignment model representing a base year of 2001. It is therefore considerably outside the Department for Transport (DfT) Transport Analysis Guidance (TAG) requirement (TAG Unit M3.1, January 2014) that trips with both trip ends within the Fully Modelled Area are based on survey data that is less than six years old. The existing model has sub-optimal demand segmentation and assignment, with a single user class matrix representing all cars assigned on top of an HGV pre-load and there is no interpeak model.
- 1.1.3 There have also been improvements in software, techniques and best practice in the intervening period, which, combined with advice provided by SCC, make the existing model unsuitable for use as the basis for assessment.
- 1.1.4 The existing network and zone system will serve as the starting point for development of the updated Lowestoft model, with further refinement where necessary. Given their age, the existing matrices will not be used, but matrices will be rebuilt from new survey data.

1.2 REPORT STRUCTURE

- 1.2.1 This Local Model Validation Report (LMVR) sets out information relating to the development, calibration and validation of the updated highway assignment model. It is structured as follows:
- Section 2 – Proposed uses of the model and key design considerations
 - Section 3 – Model standards
 - Section 4 – Key features of the model
 - Section 5 – Calibration and validation data
 - Section 6 – Network development
 - Section 7 – Trip matrix development
 - Section 8 – Network calibration and validation
 - Section 9 – Route choice calibration and validation
 - Section 10 – Trip matrix calibration and validation
 - Section 11 – Assignment calibration and validation
 - Section 12 – Summary of model development, standards achieved and fitness for purpose

1.3 DISCLAIMER

- 1.3.1 This report, and information or advice which it contains, has been prepared for the purposes set out in the instructions commissioning it (June 2015) and has been prepared with reasonable skill, care and diligence. This report has been prepared by WSP | Parsons Brinckerhoff in their professional capacity as Consultants and in performance of WSP | Parsons Brinckerhoff's duties and liabilities under its contract with Suffolk County Council. Any advice, opinions, or recommendations within this report should be read and relied upon only in the context of the report as a whole. The advice and opinions in this report are based upon the information made available to WSP | Parsons Brinckerhoff at the date of this report and on current UK standards, codes, technology and construction practices as at the date of this report. The contents of the report do not, in any way, purport to include any manner of legal advice or opinion.
- 1.3.2 The transport modelling that has been carried out under the terms of our appointment (June 2015) and described in this report has been carried out using SATURN (version 11.3.12F). Transport modelling software of this type provides predictions of transport flows on the basis of a number of assumptions. The assumptions made in developing the transport model have been identified within this report.
- 1.3.3 The liability of WSP | Parsons Brinckerhoff in respect of the information contained in the report will not extend to any third party. WSP | Parsons Brinckerhoff accept no responsibility for any costs or losses howsoever incurred as a result of the use of the output from this report unless it is proved to have failed to exercise the degree of skill and care embodied in the terms and conditions of the governing appointment (June 2015) having regard to the use of the software and the assumptions made.

2 PROPOSED USES OF THE MODEL AND KEY DESIGN CONSIDERATIONS

2.1 SCENARIOS TO BE FORECAST AND INTERVENTIONS TO BE TESTED

2.1.1 The Lowestoft Traffic Model (LTM) has been developed and validated for the sole purpose of assessing a third crossing of Lake Lothing in Lowestoft. The town centre currently has two river crossings as shown in figure 2.1.

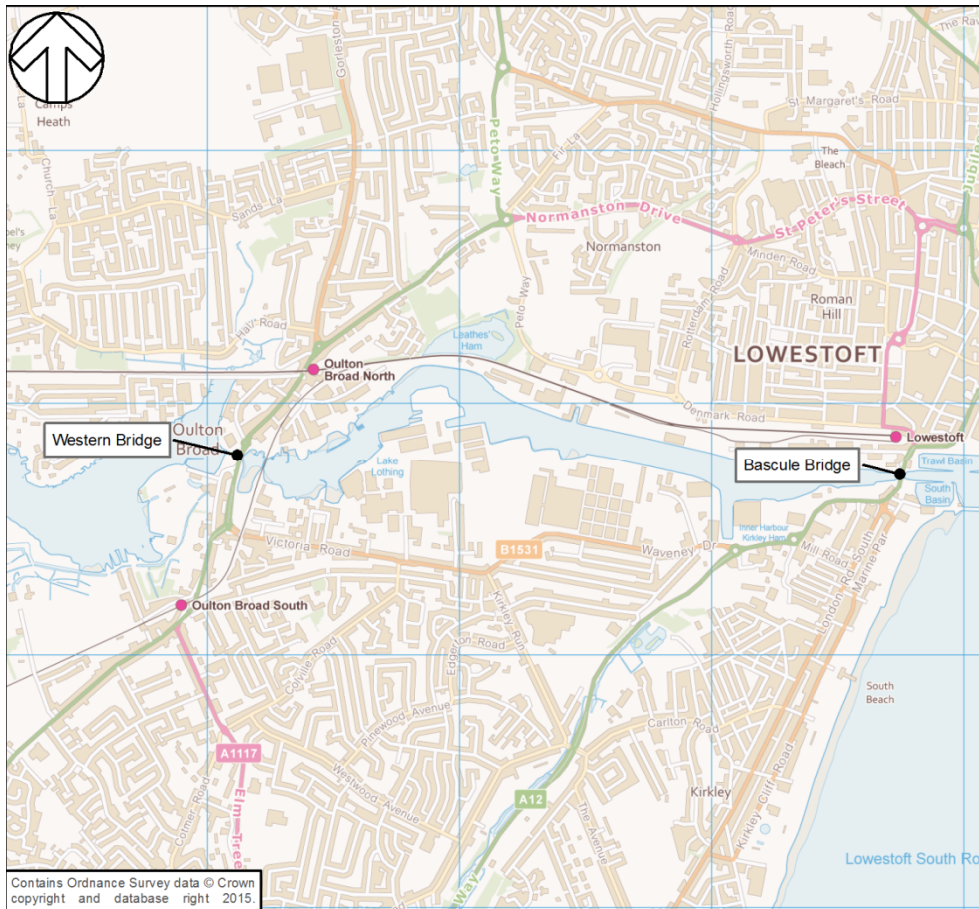


Figure 2.1 - Bridge locations

- 2.1.2 Various options regarding a third crossing are under consideration, and these will be discussed in the relevant reports relating to scheme design and the Transport Business Case (TBC).
- 2.1.3 In addition, the updated LTM may be used for other assessment purposes subsequent to work relating to the third crossing, but this will require further review of validation to ensure it is fit for other purposes.

2.2 KEY MODEL DESIGN CONSIDERATIONS

- 2.2.1 There are a number of key characteristics of Lowestoft town centre that require particular treatment, and these predominantly relate to the operation of the existing bridges.
- 2.2.2 The western bridge carries the A146 over Lake Lothing, and is significantly constrained by a level crossing to the north of the bridge. Barriers are frequently closed for long periods, leading to lengthy queues that block back across the bridge and other junctions.
- 2.2.3 The eastern Bascule bridge is a lifting bridge which, when raised, can lead to similarly long queues in the town centre. The majority of bridge openings occur in the interpeak period, but not exclusively so. The low height of this bridge and its proximity to the harbour mouth means that it is required to be opened for relatively small craft on a fairly frequent basis. Observations carried out between 14 and 16 April 2015, provided by AECOM, indicate the eastern bridge lifted once during the AM peak (0800-0900) and once during the PM peak (1700-1800) during the days surveyed. In the interpeak (1000-1600) the bridge lifted a maximum of three times during the observed period.
- 2.2.4 An additional design consideration for the traffic model is that there is a tidal flow lane running across the bridge along the A12 between Belvedere Road and Station Square. This was handled by changing the capacity and number of lanes in each direction according to the time of day.
- 2.2.5 Observed data for the tidal flow section was provided to WSP | Parsons Brinckerhoff by AECOM, covering 14 to 16 April 2015, detailing the direction of the central lane between 07:00-10:00 and 16:00-19:00. For the AM peak (08:00-09:00) the central lane was modelled as allowing northbound traffic, in the PM peak (17:00-18:00) the central lane is southbound. No observed information was available for the interpeak, however journey time information covering the Bascule Bridge indicated delays similar to the PM peak, and therefore the central lane was modelled as allowing southbound traffic in the interpeak.
- 2.2.6 A key issue that has arisen from previous work on this scheme is that it is important to understand the broad origins and destinations of the users of each bridge, and if there is any difference in the split between long-distance strategic users and local users at each bridge crossing that may inform the impacts of a third crossing.

3 MODEL STANDARDS

3.1 VALIDATION CRITERIA AND ACCEPTABILITY GUIDELINES

3.1.1 The validation of specific aspects of the model is discussed in following sections of this report. In general, the following criteria will apply, drawn from WebTAG Unit M3.1, section 3.2 (January 2014):

SCREENLINE VALIDATION

3.1.2 Screenline validation is undertaken as a check on the trip matrix, and is assessed in terms of the percentage difference between observed and modelled flows as shown in table 3.1.

Table 3.1 - Screenline acceptability

CRITERIA	DESCRIPTION OF CRITERIA	ACCEPTABILITY GUIDELINE
1	Differences between modelled flows and counts should be less than 5% of counts	All or nearly all screenlines

3.1.3 Screenlines are presented for each time period, for cars and total vehicles. Although TAG requires information for all vehicle types to be presented, counts of LGV and HGV are not sufficiently high in this study area to allow useful comparisons to be made.

3.1.4 "Nearly all" is interpreted here as relating to 85% of cases, in keeping with link validation standards.

LINK FLOW AND TURNING MOVEMENT VALIDATION

3.1.5 Measures used for link validation are:

- Absolute and percentage differences between absolute and modelled flows
- GEH statistic

3.1.6 The GEH statistic is a modified Chi-squared statistic incorporating both relative and absolute errors, defined as follows:

$$GEH = \sqrt{\frac{(M - C)^2}{(M + C)/2}}$$

3.1.7 The link flow and turning movement validation criteria are shown in table 3.2.

Table 3.2 - Link acceptability

CRITERIA	DESCRIPTION OF CRITERIA	ACCEPTABILITY GUIDELINE
1	Individual flows within 100 veh/hr of counts for flows less than 700 veh/hr	> 85% of cases
	Individual flows within 15% of counts for flows from 700 veh/hr to 2,700 veh/hr	> 85% of cases
	Individual flows within 400 veh/hr of counts for flows more than 2,700 veh/hr	> 85% of cases
2	GEH < 5 for individual flows	> 85% of cases

3.1.8 Both link flows and turning movements are presented using the above criteria, although turning movements are not generally expected to fully meet the criteria.

3.1.9 Information is presented for cars and total vehicles in all modelled time periods.

JOURNEY TIME VALIDATION

3.1.10 Criteria for journey time validation are presented in table 3.3.

Table 3.3 - Journey time acceptability

CRITERIA	DESCRIPTION OF CRITERIA	ACCEPTABILITY GUIDELINE
1	Modelled times along routes should be within 15% of surveyed times (or minute, if higher than 15%)	> 85% of routes

3.1.11 The model does not feature different speed/flow relationships or link speeds for different vehicle types, the comparisons are presented for all vehicles combined.

3.1.12 Comparisons are presented separately for all modelled time periods.

3.2 CONVERGENCE CRITERIA AND STANDARDS

3.2.1 An element of calibrating the model is ensuring that a satisfactory convergence is achieved. Model convergence is needed to ensure traffic flows remain stable between successive iterations of the model. This is particularly important when model outputs are used to inform the economic benefits of scheme appraisal, as it is critical that calculated benefits arise from the impact of the scheme and not as a result of difference in convergence.

3.2.2 In accordance with criteria set out in TAG Unit M3.1 (January 2014), the parameters %Flow, %GAP and Delta (δ) have been monitored to determine the level of convergence. %Flow measures the proportion of links in the network with flows changing by less than 1% from the previous iteration. δ is the difference between costs on chosen routes and costs on minimum cost paths. %GAP is a generalisation of the δ function to include the interaction effects within the simulation.

3.2.3 The convergence criteria used to assess when a model is considered to have converged is shown in table 3.4.

Table 3.4 - Convergence criteria

MEASURE OF CONVERGENCE	ACCEPTABLE VALUE
'Delta' and %GAP	Less than 0.1% or at least stable with convergence fully documented and all other criteria met
Percentage of links with flow change < 1%	Four consecutive iterations greater than 98%
Percentage of links with cost change < 1%	Four consecutive iterations greater than 98%
Percentage change in total user costs	Four consecutive iterations less than 0.1%

3.2.4 TAG Unit M3.1 indicates that delta (δ) and %GAP values of less than 0.1% is the most fundamental indicator of model convergence and should be achieved as a minimum.

4 KEY FEATURES OF THE MODEL

4.1 FULLY MODELLED AREA AND EXTERNAL AREA

4.1.1 The hierarchy of the model area is shown in figure 4.1

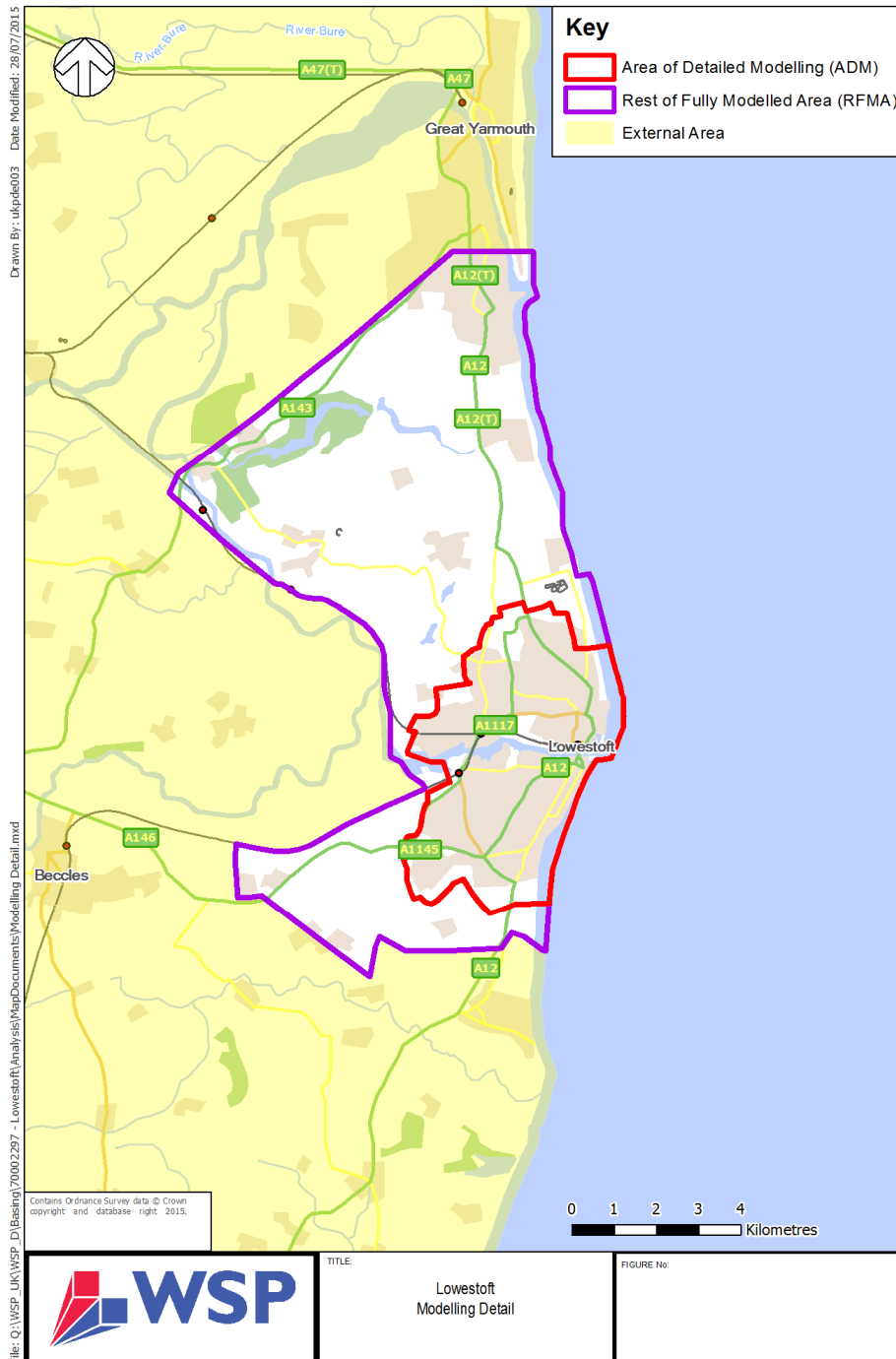


Figure 4.1 - Modelled area

4.2 ZONING SYSTEM

4.2.1 The zoning system is based initially on census LSOA and MSOA boundaries, split to better fit realistic zoning points. The zones within the area of detailed modelling are shown in figure 4.2.

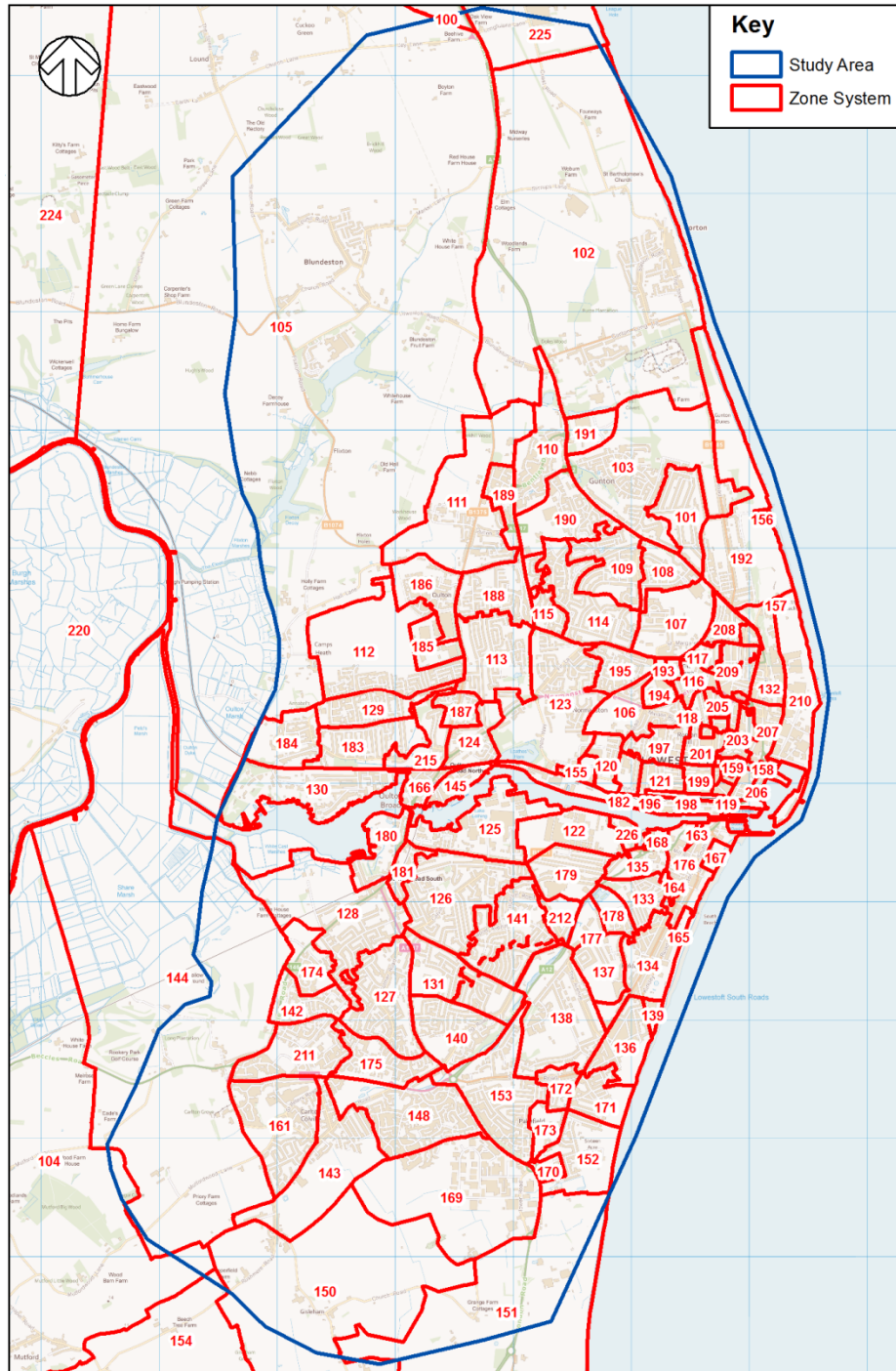


Figure 4.2 - Zoning in area of detailed modelling

4.2.2

Figure 4.3 shows the zoning in the rest of the fully modelled area and the external area.

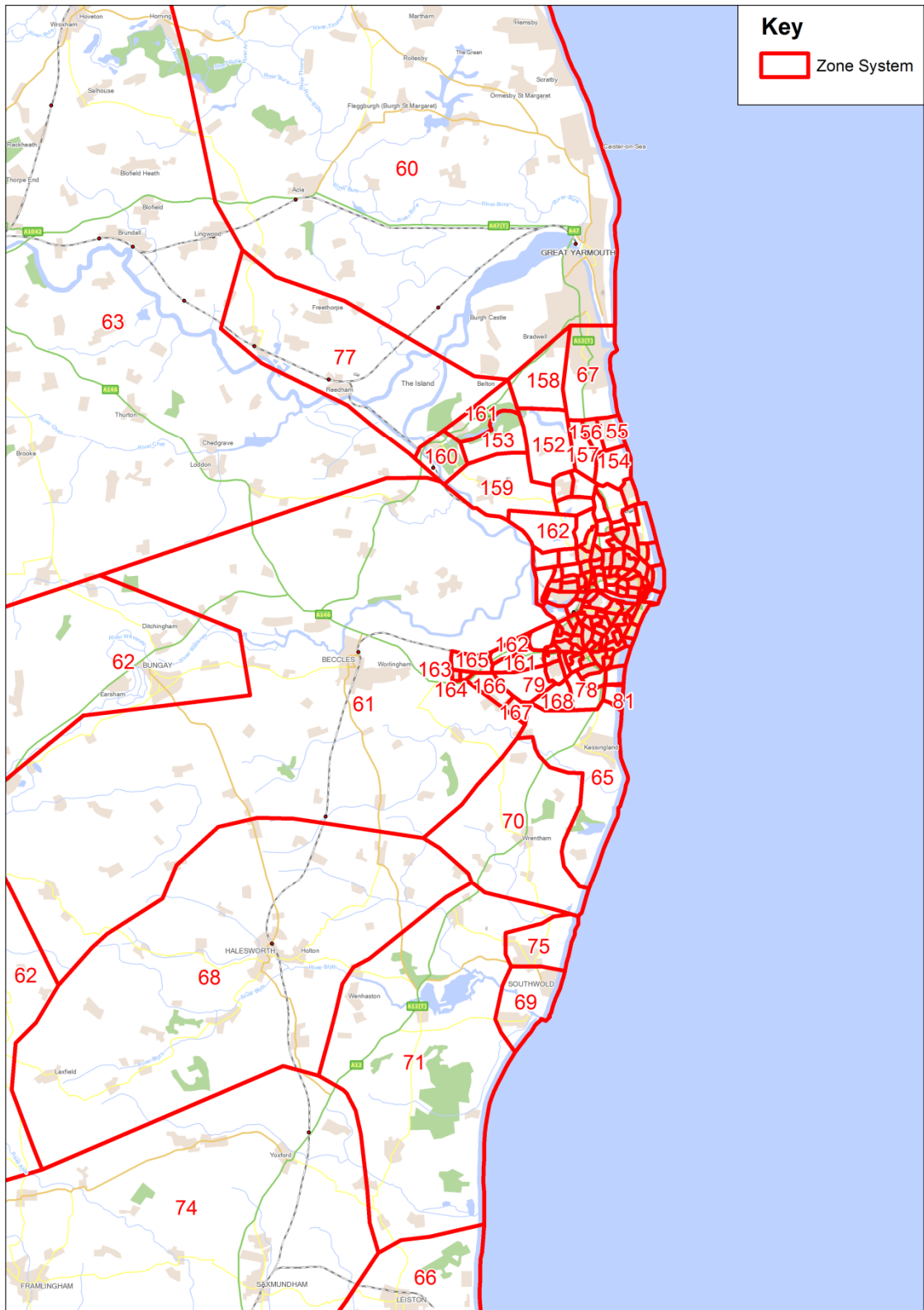


Figure 4.3 - Zoning in external area

4.2.3

The zones have been grouped to create five sectors. The locations of these can be seen in Figure 4.4.

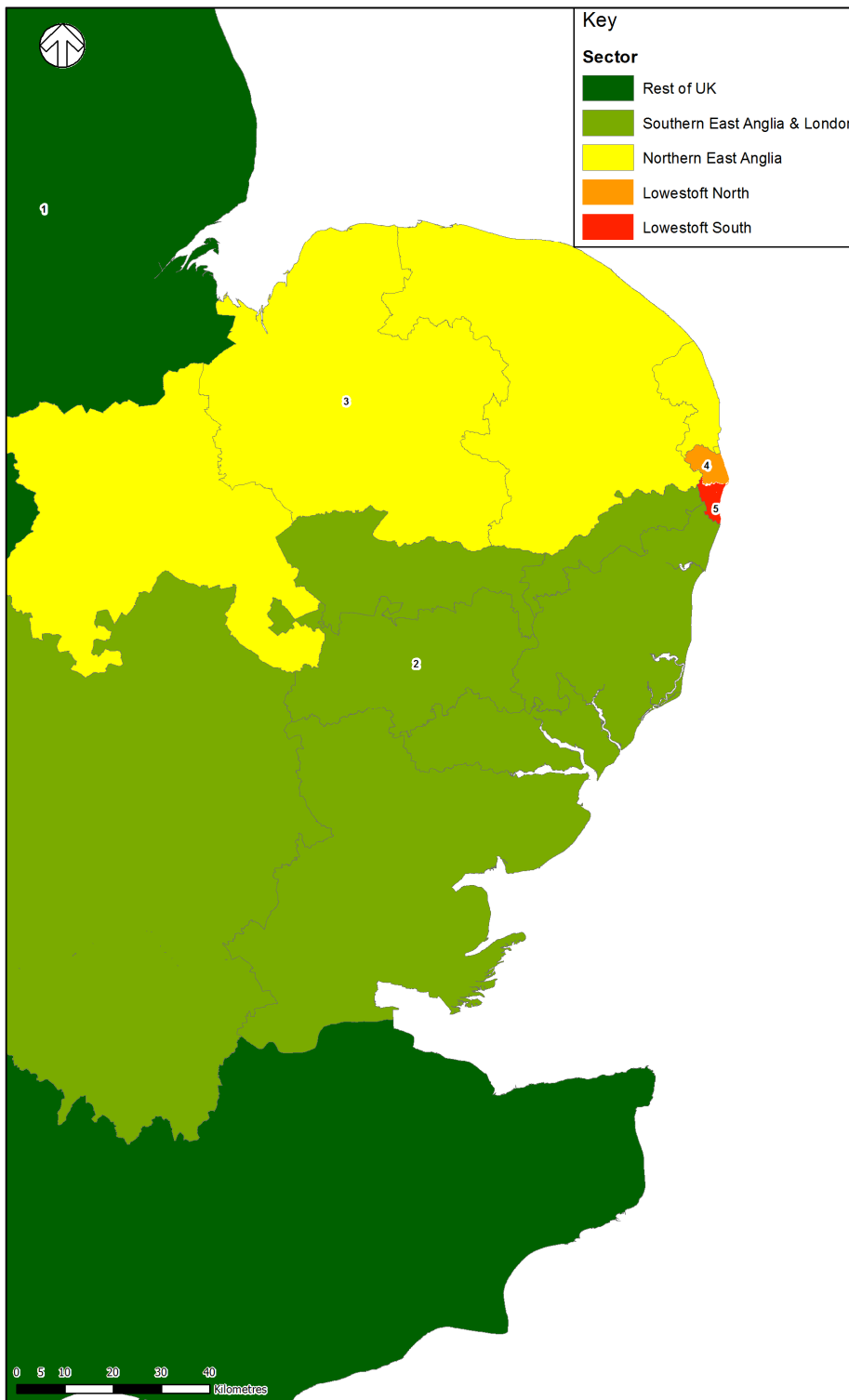


Figure 4.4 - Sector Plan

4.3 NETWORK STRUCTURE

- 4.3.1 The area of detailed modelling covers the town of Lowestoft. In this area, the extent of the network is sufficient to cover all roads with significant traffic volumes and all realistic route choice available to drivers. All major junctions are modelled. In the Rest of the Fully Modelled Area, detail is reduced, with all principal strategic routes modelled and capacity restraint characterised through the use of speed/flow relationships as well as strategically important junctions.
- 4.3.2 In the External Area, the network is simplified to the extent that traffic is able to enter the Fully Modelled Area at the correct locations, without capacity restraint.
- 4.3.3 The network structure is shown in Figure 4.5.

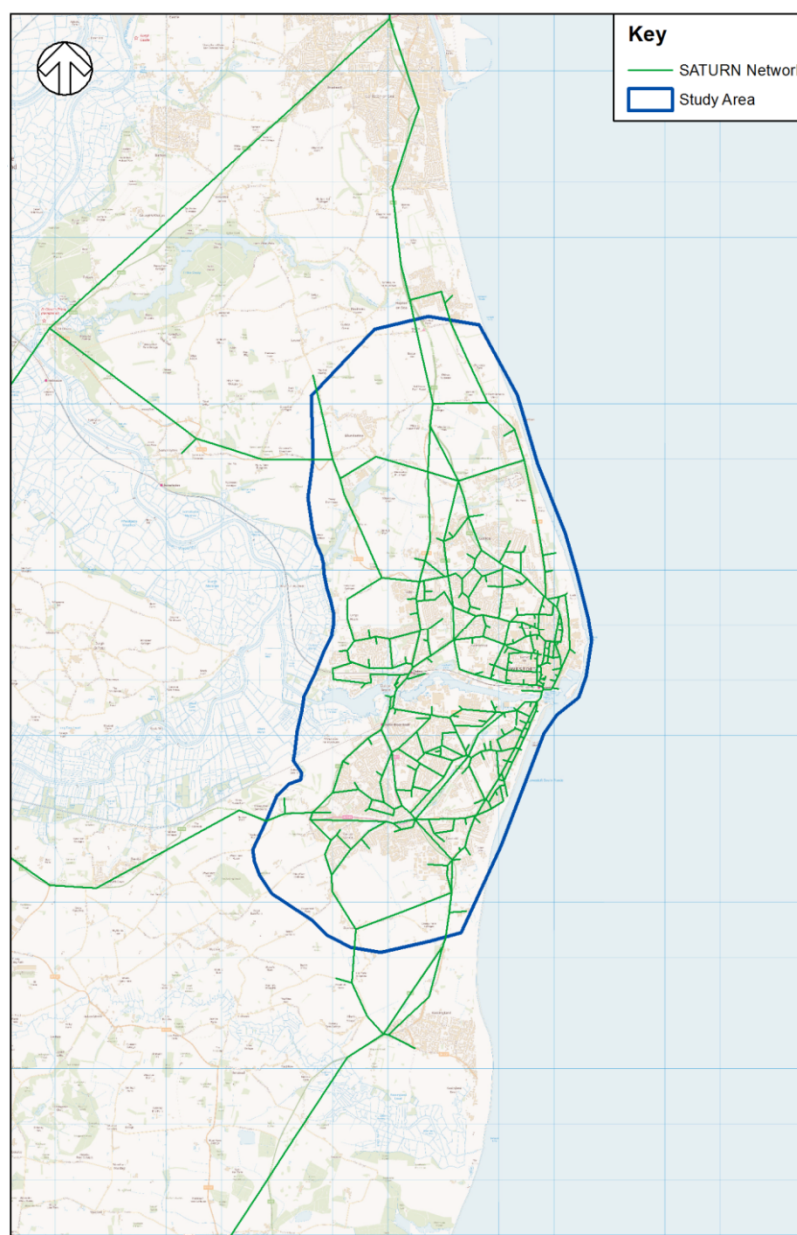


Figure 4.5 - Network structure

4.4 CENTROID CONNECTORS

- 4.4.1 Centroid connectors connect the zoning system to the model network, allowing trips to load onto the network for assignment. It is critical that centroids connectors represent realistic loading points, particularly in the fully modelled area. Centroid connectors in the fully modelled area have been designed to represent actual loading points to specific residential and commercial areas, generally via a spur link to represent the actual access point. In this way, turns into and out of zones can be clearly understood.
- 4.4.2 The number of centroid connectors has been minimised, with most zones having a single centroid connector except in cases where a zone has clear multiple points of access, and sub-dividing the zone would not be realistic.
- 4.4.3 Centroid connectors have been designed so that they do not cross the network, further ensuring that loading is realistic. Connectors for different zones are loaded at different points, to ensure trips between adjacent zones are loaded on to the network. Centroid connectors are also loaded away from count locations, to avoid inconsistencies between the counted flow and loaded trips.
- 4.4.4 In the fully modelled area, zones are sufficiently small such that average costs to access the model are sufficiently represented by the spur access links, so centroids themselves do not have costs associated with them.
- 4.4.5 In the external area, centroid connectors are linked to the network with appropriate parameters for distance and average speed to represent the average cost of accessing the network.

4.5 TIME PERIODS

- 4.5.1 The following time periods are modelled:

- AM peak (08:00 – 09:00)
- Average interpeak (10:00 – 16:00)
- PM peak (17:00 – 18:00)

This is consistent with advice presented in Section 2.5 of TAG Unit M3.1 (January 2014). The choice of peak hour has been confirmed through analysis of the available long term ATC sites obtained through the Highways England TRADS database. Average counts are presented in Table 4.1 across each hour of the peak period, with the maximum peak identified in red.

Table 4.1 - Peak hour identification

MONTH	AM PEAK			PM PEAK		
	0700-0800	0800-0900	0900-1000	1600-1700	1700-1800	1800-1900
January	1714	3020	2321	2495	2508	1624
February	1780	2679	2330	2462	2460	1724
March	1901	3078	2491	2620	2,594	1818
April	1818	2865	2470	2679	2772	1918
May	1837	2895	2438	2581	2677	1946
June	1858	3026	2466	2600	2735	1952
July	1766	2776	2424	2558	2700	1966
August	1629	2455	2351	2584	2746	2056
September	1850	2867	2387	2529	2591	1912
October	1817	2812	2350	2512	2565	1813
November	1797	2928	2399	2521	2476	1770
December	1548	2628	2380	2425	2275	1654

4.6 USER CLASSES

4.6.1 The following user classes are modelled:

- UC1: Car – Commuting
- UC2: Car – Employer's Business
- UC3: Car – Other
- UC4: LGV
- UC5: HGV

4.6.2 This is consistent with advice presented in Section 2.6 of TAG Unit M3.1 (January 2014).

4.7 ASSIGNMENT METHODOLOGY

4.7.1 Model assignment of trips to the highway network was undertaken using a standard approach based on a 'Wardrop User Equilibrium', which seeks to minimise travel costs for all vehicles in the network. The Wardrop User Equilibrium is based on the following proposition:

"Traffic arranges itself on congested networks such that the cost of travel on all routes used between each origin-destination pair is equal to the minimum cost of travel and unused routes have equal or greater costs."

- 4.7.2 The Wardrop User Equilibrium as implemented in SATURN is based on the 'Frank-Wolfe Algorithm', which employs an iterative process. This process is based on successive 'All or Nothing' iterations, which are combined to minimise an 'Objective Function'. The travel costs are recalculated after each iteration and compared to those from the previous iteration. The process is terminated once successive iteration costs have not changed significantly. This process enables multi-routing between any origin-destination pair.

4.8 GENERALISED COST FORMULATIONS AND PARAMETER VALUES

- 4.8.1 Generalised cost is defined in keeping with the guidance in section 2.8 of TAG Unit M3.1 (January 2014), and is as follows:

$$\text{Generalised cost} = \text{Time} + \left(\frac{\text{Vehicle operating cost}}{\text{Value of time}} \right) \text{Distance}$$

- 4.8.2 Value of time is calculated in pence per minute (PPM) and vehicle operating cost is calculated in pence per kilometre (PPK). The adopted parameters were calculated from the WebTAG databook (November 2014).
- 4.8.3 The parameters adopted for a 2015 base year are shown in Table 4.2. For the HGV class, local ATC data was used to determine the split of vehicles which could be classified as OGV1 and OGV2 by peak hour. This split was used to calculate average generalised cost parameters for HGV.

Table 4.2 – 2015 generalised cost parameters

USER CLASS	AM		IP		PM	
	PPM	PPK	PPM	PPK	PPM	PPK
Car Commuting	13.74	6.70	13.63	6.70	13.44	6.70
Car Business	46.57	13.38	45.51	13.38	44.78	13.38
Car Other	17.49	6.70	18.18	6.70	18.72	6.70
LGV	20.98	13.66	20.98	13.66	20.98	13.66
HGV	21.25	37.35	21.25	37.35	21.25	37.35

4.9 CAPACITY RESTRAINT MECHANISMS: JUNCTION MODELLING AND SPEED/FLOW RELATIONSHIPS

JUNCTION MODELLING

- 4.9.1 The following key junctions were directly measured in terms of their geometric characteristics with accurate saturation flows included within the model:
- Normanston Drive / Peto Way
 - Normanston Drive / Bridge Road / B1375
 - Bridge Road / A146
 - A12 / A146

→ A12 / Mill Road

→ A12 / A1145

SPEED/FLOW RELATIONSHIPS

4.9.2 Speed flow curves consistent with COBA 10 values were allocated to specific links, detailed in section 6.3

4.10 RELATIONSHIPS WITH DEMAND MODELS AND PUBLIC TRANSPORT ASSIGNMENT MODELS

4.10.1 The highway assignment model will be used as a component of a DIADEM-based variable demand model. No public transport assignment model is included, as it is not required for this assessment. The Traffic Forecasting Report sets out the interactions between the highway assignment model and the demand model, as well as providing further consideration of the need for public transport assignment modelling.

5 CALIBRATION AND VALIDATION DATA

5.1 INTRODUCTION

5.1.1 WSP | Parsons Brinckerhoff commissioned a range of surveys which are detailed in the data collection report (October 2015). This included the types of data shown in Table 5.2.

Table 5.1 – Commissioned survey data

SURVEY TYPE	SURVEY PERIOD	TIME
Automatic Number Plate Recognition (ANPR)	Tuesday 14 July 2015	07:00-19:00
Automatic Traffic Counts (ATC)	Monday 29 June 2015 to Monday 27 July 2015	All day
Manual Classified Counts (MCC)	Tuesday 14 ^h July 2015	07:00-19:00

5.1.2 Manual Classified Counts (MCC) were also obtained from AECOM which were carried out between Tuesday 14 April 2015 and Thursday 16 April 2015, count data was only available at these sites for 0700-1000 and 1600-1900.

5.2 TRAFFIC COUNTS AT ANPR SITES

5.2.1 Traffic counts were collected via Automatic Number Plate Recognition (ANPR) cameras at 29 locations described in

5.2.2 Table 5.2. The ANPR were located:

- on links at key entry points into the study area and on links within the study area including the western Mutford Bridge and eastern Bascule Bridge
- at car parks within the study area.

5.2.3 The purpose of this data was to observe the major origin-destination movements within the study area from which to build the prior matrix.

5.2.4 MCC were conducted at each ANPR location to capture the total volume of traffic with vehicles classified into the following types:

- Car
- LGV
- OGV1
- OGV2
- Bus / Coach

5.2.5 Overall the sample rates at each of the external ANPR locations in both inbound and outbound directions were consistently high at around 95%. The overall match rate compared the ANPR trips to the total number of inbound trips recorded by the associated MCC was 51%. Matched trips represented inbound trips into the study area which are then picked up by another ANPR within the cordon. The overall match rate achieved was considered acceptable from which to produce a prior matrix though entails inferring the distribution for 49% of the traffic from the matched trips.

- 5.2.6 As detailed in the data collection report, there were a number of issues with the ANPR surveys. At Site 3, on the A12 Yarmouth Road to the north of the study area there was a low inbound sample rate of 36%. The survey company reported the camera at this location appears to have been moved by a member of the public during the survey report which caused the ANPR camera to be at an angle which affected the ability to capture vehicle number plates.
- 5.2.7 As detailed in the data collection report, there were a number of issues with the ANPR surveys. At site 13 on Flixton Road, the survey company reported that the ANPR camera failed and no vehicle number plates were captured at this location during the survey period. However an MCC was still carried out at this location.
- 5.2.8 At site 17, Swimming Pool car park, the MCC camera failed and no classified data could be provided at this location.
- 5.2.9 At Site 18 (Battery Green Road Car Park), Intelligent Data reported a corruption of the MCC recording which lead to some data loss leading to the sample rate calculated being above 100%. The ANPR data was therefore scaled to match the MCC data.
- 5.2.10 As detailed in the data collection report, the inbound sample rate and subsequent match rate were low at this site, 36% and 26% respectively. The survey company explained this occurred due to a member of the public moving the camera during the survey period.

Table 5.2 – ANPR count location descriptions

ID	DESCRIPTION	TYPE
1	A12 London Road	Link
2	A146 Beccles Road	Link
3	A12 Yarmouth Road	Link
4	Coast Road	Link
5	A12 Pier Terrace (Eastern Bascule bridge)	Link
6	A146 Bridge Road (Western Mutford bridge)	Link
7	B1375 Gorleston Road	Link
8	A1117 Millennium Way	Link
9	A12 Yarmouth Road	Link
10	A12 Tom Crisp Way	Link
11	B1532 London Road South	Link
12	B1074 Blundeston Road	Link
13	Flixton Road	Link
14	B1531 Waveney Drive	Link
15	North Quay Retail Park	Car Park

ID	DESCRIPTION	TYPE
16	Links Road Car Park	Car Park
17	Swimming Pool Road Car Park	Car Park
18	Shopping Centre Car Park (Battery Green Rd exit)	Car Park
19	Shopping Centre Car Park (Gordon Road entry)	Car Park
20	Surrey St Car Park entry	Car Park
21	Surrey St Car Park exit (onto Clapham Road)	Car Park
22	Clapham Road Car Park	Car Park
23	St Johns Rd Car Park	Car Park
24	Kirkley Rise Car Park (Northern access)	Car Park
25	Kirkley Rise Car Park (Southern access)	Car Park
26	Kirkley Cliff Road Car Park	Car Park
27	Claremont Road Car Park	Car Park
28	Marine Parade Car Park	Car Park
29	Asda Car Park	Car Park

5.2.11 Figure 5.1 shows the locations of the ANPR counts located on links within the study area.



Figure 5.1 – ANPR traffic counts on links

5.2.12 Figure 5.2 shows the locations of ANPR counts carried out at car parks.



Figure 5.2 – ANPR traffic counts at car parks

5.3 TRAFFIC COUNTS FOR MATRIX ESTIMATION

5.3.1 Table 5.3 describes the count locations which were used for matrix estimation.

5.3.2 The count locations used for matrix estimation were predominantly Automatic Traffic Counts (ATC). Monday to Thursday data was taken from the ATC data to provide an average flow at each location as per section 3.3 of TAG Unit M1.2. The majority of sites provided at least two continuous weeks of data, however data loss occurred at some sites as detailed in the data collection report, most notably at ATC Site 4 (A146 Beccles) Road for which only one observation was available.

5.3.3 The ANPR at site 3 (A12 Yarmouth Road) was used for matrix estimation; this site was located on a national speed limit dual carriageway. It was not possible for the survey company to safely lay an ATC at this location. The MCC count associated with ANPR site 3 (A12 Yarmouth Road) was used for matrix estimation at this location as it represents a key route for traffic entering and exiting the main study area.

Table 5.3 – Description of traffic counts used for matrix estimation

ID	DESCRIPTION	TYPE
3	Gisleham Road	ATC
4	A146 Beccles Road	ATC
7	London Road South	ATC
8	A12 Tom Crisp Way	ATC
9	A1117 Elm Tree Road	ATC
11	Kirkley Run	ATC
12	A146 Waveney Drive	ATC
15	Katwijk Way	ATC
16	A12 Battery Green Road	ATC
19	Denmark Road	ATC
21	Peto Way	ATC
22	A1117 Normanston Drive	ATC
23	A1144 Normanston Drive	ATC
24	Oulton Road	ATC
25	B1375 Gorleston Road	ATC
26	A1117 Millennium Way	ATC
27	A12 Yarmouth Road	ATC

ID	DESCRIPTION	TYPE
28	B1385 Corton Road	ATC
29	A12 Yarmouth Road	ATC
30	B1375 Parkhill	ATC
31	B1074 Blundeston Road	ATC
32	Flixton Road	ATC
3	A12 Yarmouth Road	ANPR - MCC

5.3.4 Figure 5.3 shows the locations of the counts used for matrix estimation and the calibration screenlines.

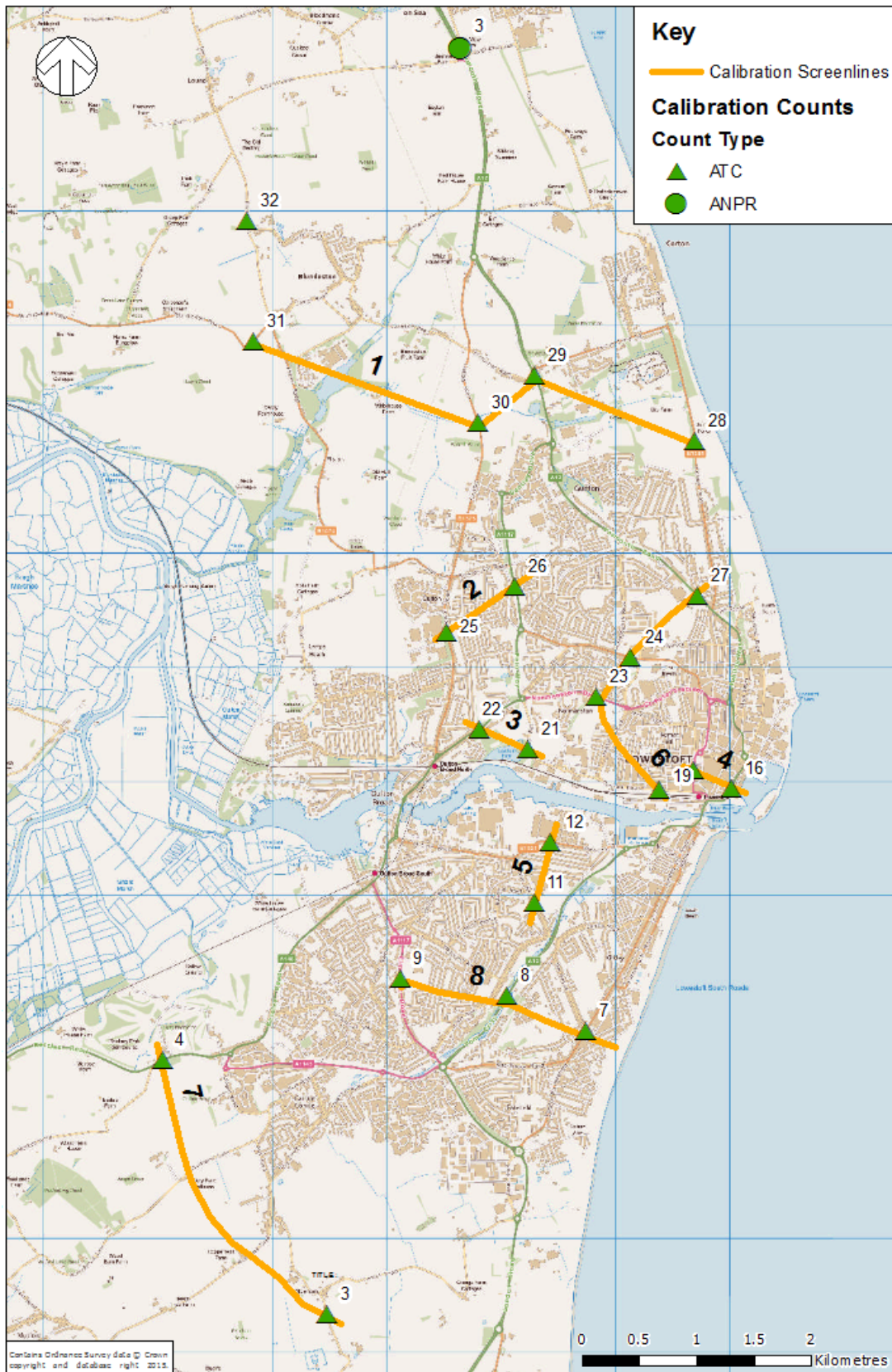


Figure 5.3 – Counts and screenlines used for calibration

5.4 TRAFFIC COUNTS FOR VALIDATION

LINK COUNTS

5.4.1 Table 5.4 describes the count locations which were used for validation. As with the calibration counts the ATC data was processed to provide a Monday to Thursday average. For counts located at ANPR sites, the associated MCC link count / car park access count was used.

Table 5.4 – Description of traffic counts used for validation

ID	DESCRIPTION	TYPE
5	A1145 Castleton Avenue	ATC
6	A12 London Road	ATC
10	A146 Beccles Road	ATC
17	A12 Old Nelson Street	ATC
18	St Peter's Street	ATC
20	Rotterdam Road	ATC
33	Coast Road	ATC
6053	Katwijk Way	TRADS
5	A12 Pier Terrace (eastern Bascule Bridge)	ANPR - Link
6	A146 Saltwater Way (western Mutford Bridge)	ANPR - Link
15	North Quay Retail Park	ANPR – Car Park
16	Links Road Car Park	ANPR – Car Park
17	Swimming Pool Road Car Park	ANPR – Car Park
18	Shopping Centre Car Park (Battery Green Rd exit)	ANPR – Car Park
19	Shopping Centre Car Park (Gordon Road entry)	ANPR – Car Park
20	Surrey St Car Park entry	ANPR – Car Park
21	Surrey St Car Park exit (onto Clapham Road)	ANPR – Car Park
22	Clapham Road Car Park	ANPR – Car Park
23	St Johns Rd Car Park	ANPR – Car Park
24	Kirkley Rise Car Park (Northern access)	ANPR – Car Park
25	Kirkley Rise Car Park (Southern access)	ANPR – Car Park
26	Kirkley Cliff Road Car Park	ANPR – Car Park

ID	DESCRIPTION	TYPE
28	Marine Parade Car Park	ANPR – Car Park
29	Asda Car Park	ANPR – Car Park

5.4.2 At ANPR site 5 (A12 Pier Terrace) on the eastern Bascule Bridge, only the MCC was available. The survey company were unable to find a suitable location at which to place an ATC.

5.4.3 At ANPR site 6 (A146 Saltwater Way) on the western Mutford Bridge, the MCC was used for validation. This was because the ATC laid at this location showed a notable difference in traffic flow southbound in the AM peak and PM peak, compared to the MCC (see Table 5.5). As discussed in section 7, regarding matrix development, the prior matrix was developed using the ANPR data, therefore the flow on the western Mutford Bridge would be more closely aligned to the MCC associated with the ANPR rather than the ATC data.

Table 5.5 – Comparison of ATC and MCC at western Mutford Bridge (A146 Saltwater Way)

ID	ATC COUNT TOTAL - NB	MCC COUNT TOTAL - NB	ATC COUNT TOTAL - SB	MCC COUNT TOTAL - SB
AM peak	942	944	777	904
Interpeak	920	983	861	931
PM peak	1064	1114	997	1133

5.4.4 The vehicle split for the western Mutford Bridge observed data was taken from the ATC data on the A146 Saltwater Way. This is because ATC was used for matrix estimation throughout the model. There was a notable difference in the vehicle splits between Car and LGV for the MCC and ATC at the western Mutford Bridge. This meant in terms of validation by vehicle type, the modelled flow was not ideally matched compared to the observed data if the MCC splits were used, which was out of keeping with adjacent counts based on ATC data. The comparison is shown in Table 5.6.

Table 5.6 – Comparison of ATC and MCC vehicle split for observed count total at western Mutford Bridge (A146 Saltwater Way)

ID	COUNT TOTAL	CAR TOTAL - MCC	LGV TOTAL - MCC	HGV TOTAL - MCC	CAR TOTAL - ATC	LGV TOTAL - ATC	HGV TOTAL - ATC
AM - NB	944	780	139	25	656	242	46
AM - SB	904	716	152	36	506	367	31
IP - NB	983	819	130	35	718	228	38
IP - SB	931	780	120	31	575	329	27
PM - NB	1114	957	140	17	852	228	34
PM - SB	1133	1003	119	11	749	370	13

5.4.5 This issue was also prevalent for the MCC observed data on the eastern Bascule Bridge. As no ATC was available at this location, ATC 16 (A12 Battery Green Road) was used to provide the

vehicular split to the observed data, as shown in Table 5.7.

Table 5.7 – Comparison of ATC and MCC vehicle split for observed count total at eastern Bascule Bridge (A12 Pier Terrace)

ID	COUNT TOTAL	CAR TOTAL - MCC	LGV TOTAL - MCC	HGV TOTAL - MCC	CAR TOTAL - ATC	LGV TOTAL - ATC	HGV TOTAL - ATC
AM - NB	1531	1324	164	43	656	810	65
AM - SB	898	691	169	38	530	322	46
IP - NB	1041	870	127	44	453	532	56
IP - SB	1051	875	125	31	612	394	45
PM - NB	1104	966	110	28	582	499	24
PM - SB	1591	1419	147	25	1014	539	38

5.4.6

Figure 5.4 shows the locations of the counts used for validation and the validation screenlines.

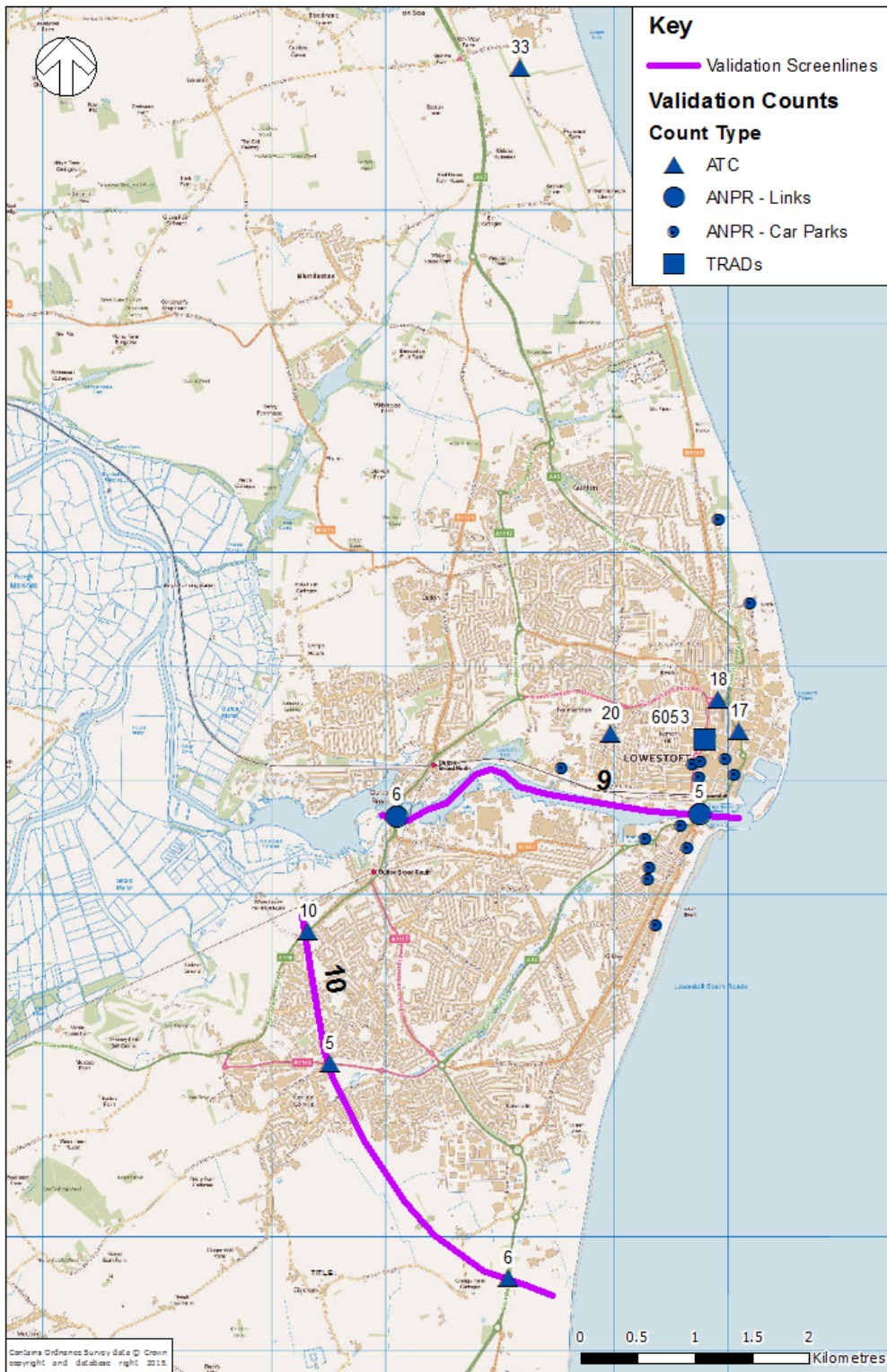


Figure 5.4 – Counts and screenlines used for validation

MANUAL CLASSIFIED COUNTS

5.4.7 Manual classified counts were used for validation of turning movements at key junctions. Table 5.8 describes the locations of the turning counts.

Table 5.8 – Manual classified count locations

ID	DESCRIPTION
1	London Road/Arbor Lane/A12/Tower Road
2	Tom Crisp Way/Stadbroke Road/Elm Tree Road
3	Somerleyton Road/Oulton Street/Hall Lane/Gorleston Road
4	Yarmouth Road/Gorleston Road
5	Yarmouth Road/Leisure Way/Foxburrow Hill/Bentley Drive
6	Yarmouth Road/Corton Road
7	Millennium Way/Oulton Road/Peto Way
8	Horn Hill/Maconochie Way/A12/Waveney Drive
9	A12/Corton Long Lane/A12/Lowestoft Link Road
10	A12 Waveney Road/Station Square
11	Commercial Road/Station Square
12	A12 Pier Terrace/London Road South/Belvedere Road
13	A12 Belvedere Road/Kirkley Rise
14	Denmark Road/Katwijk Way
15	Katwijk Way/Raglan Street
16	A12 Waveney Road/Suffolk Road
17	A12 Tom Crisp Way/Blackheath Road
18	Saltwater Way/Victoria Road
19	Normanston Drive/Gorleston Road
20	Fir Lane/A117 Normanston Drive/Peto Way
21	A12/Gordon Road/Whapload Road
22	A12/St Peters Street
23	A1144/Katwijk Way
24	A146 Beccles Road/Cotmer Road

5.4.8 Figure 5.5 shows the location of the MCCs commissioned for this study.

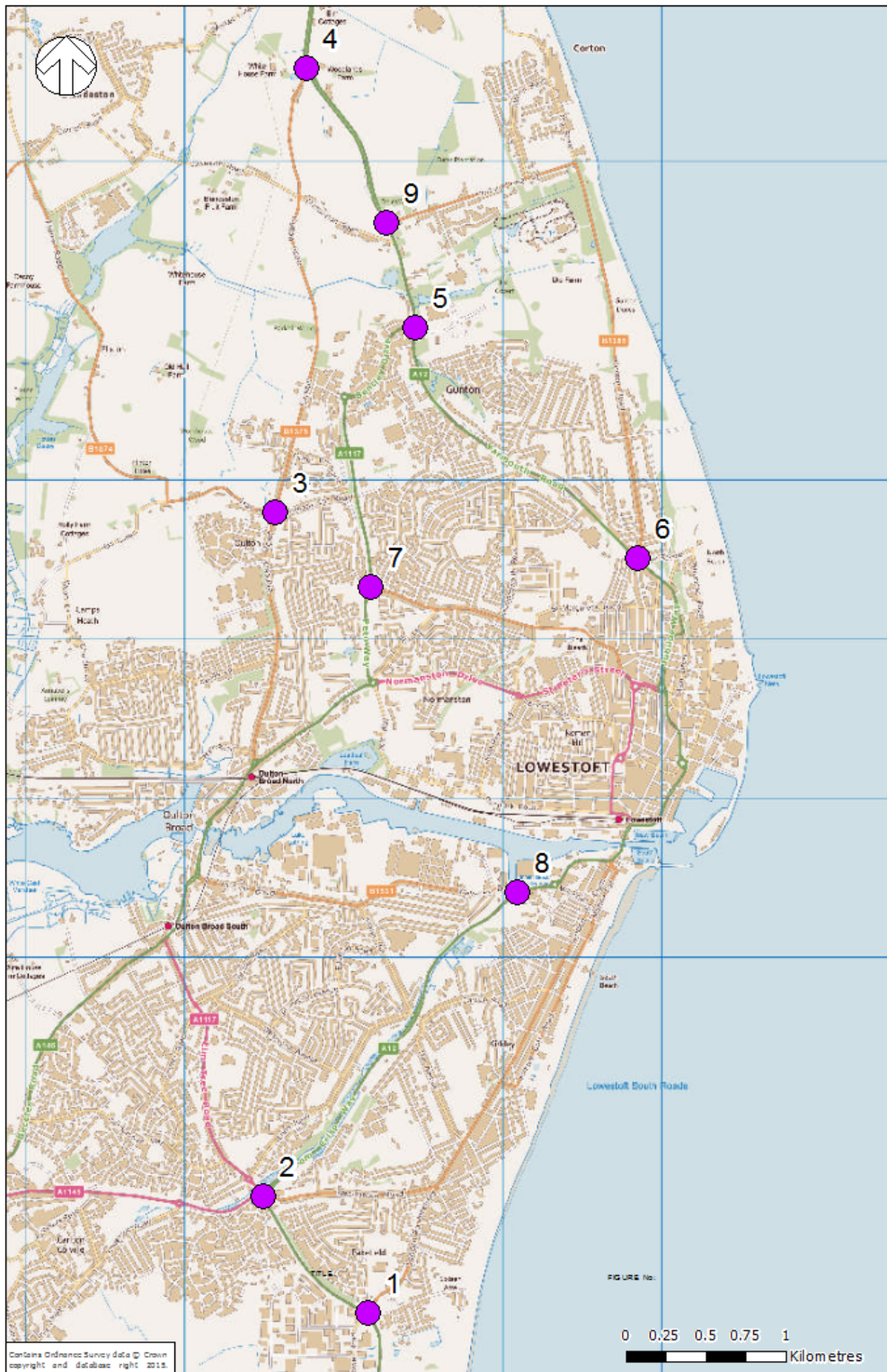


Figure 5.5 – Wider network MCC

5.4.9 Figure 5.6 shows the location of the MCCs carried out by AECOM.

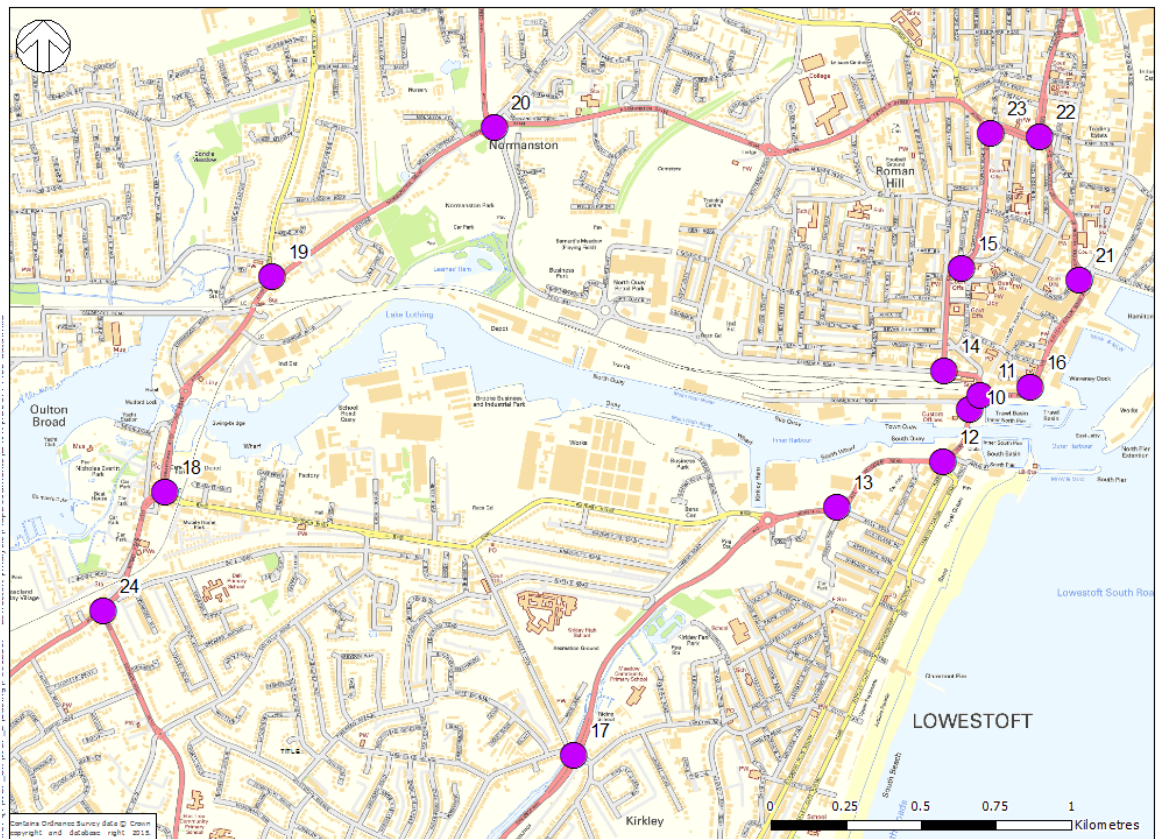


Figure 5.6 – AECOM town centre MCC

5.5 JOURNEY TIME SURVEYS FOR CALIBRATION AND VALIDATION

5.5.1 Table 5.9 describes the journey time routes which were used for calibrating and validating the model. These represent the key routes through the model study area

Table 5.9 – Description of journey time routes

ID	DESCRIPTION	LENGTH
1	B1375 Gorleston Road	4.5km
2	A12 Yarmouth Road / Katwijk Way	6.2km
3	A1117 Normanston Drive / A1144 St Peter's Street	3.3km
4	A1117 Bentley Drive / Millennium Way / Peto Way	2.5km
5	A12 London Road / B1532 London Road South	6.7km
6	A1145 Castleton Avenue / A12 Tom Crisp Way / A12 Belvedere Road	6.7km
7	B1074 / A1117 Millennium Way / Oulton Road	3.9km
8	A146 Beccles Road / A146 Waveney Drive	9.5km
9	A12 Bloodmoor Road / A1117 Elm Tree Road	3.6km

5.5.2 Trafficmaster journey time data covering key links within Suffolk was obtained from the DfT with the average journey time calculated from June 2015 data, for Monday to Thursdays only. The Trafficmaster data was related to an ITN road network, using ArcGIS this network was related to the SATURN network to allow the comparison of observed and modelled journey time.

5.5.3 Due to the variability of the data, the average and standard deviation of the data was analysed in order to determine the high and low confidence interval for the data. Observations outside these high and low intervals were deemed to be outliers and excluded. This ensured the average travel time was not unduly distorted by unusually high or low values.

5.5.4 Figure 5.7 shows the extent of the journey time routes.

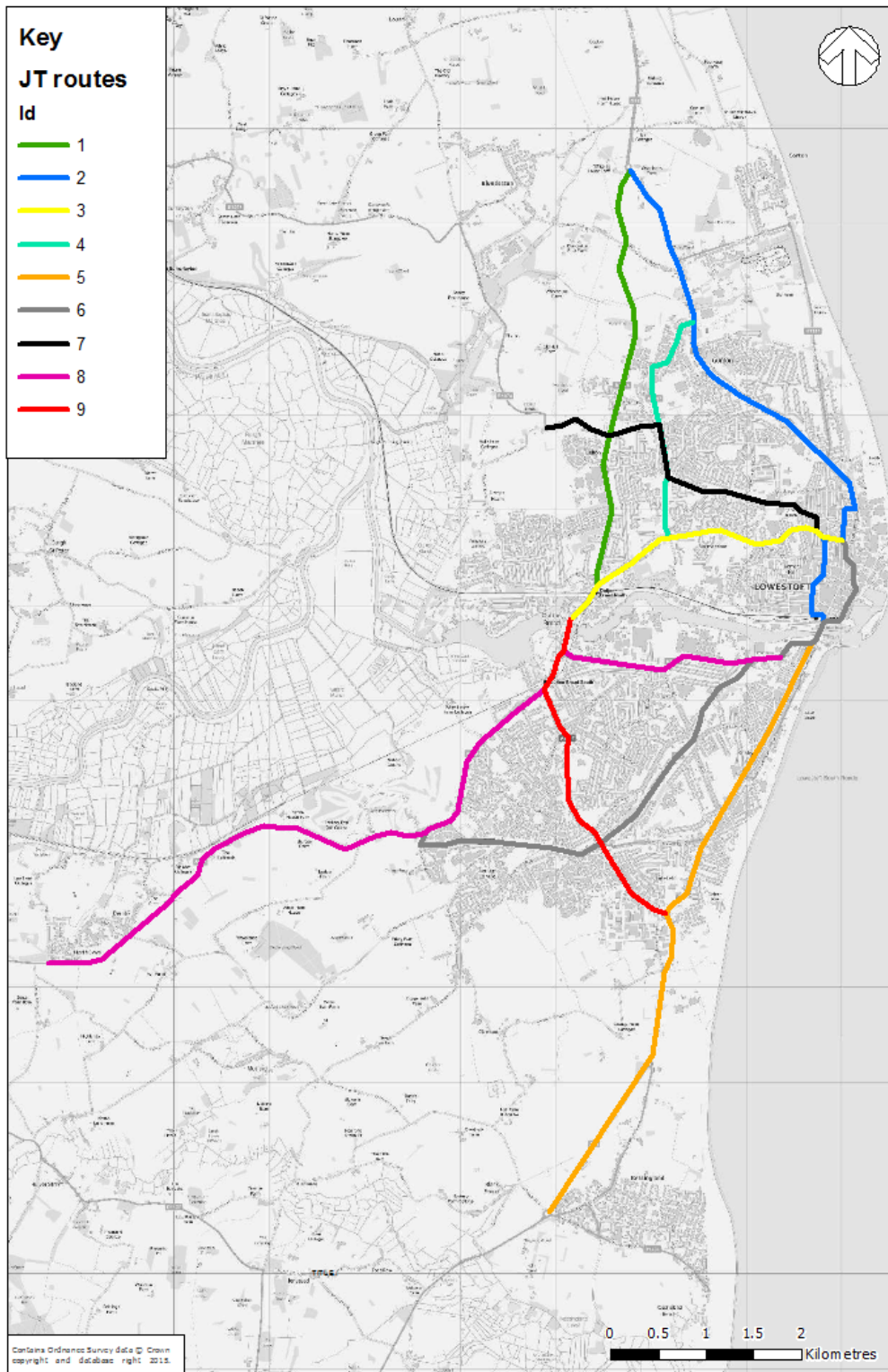


Figure 5.7 – Journey time routes

6 NETWORK DEVELOPMENT

6.1 NETWORK DATA, CODING AND CHECKING

6.1.1 The model network was based on the existing Lowestoft SATURN model network, updated to take account of any changes to the network that had occurred since the original Lowestoft model was built, or any significant coding errors.

6.1.2 The network was verified through the use of ArcGIS, site visits and aerial photography. In particular, checks were carried out to verify:

- Node co-ordinates
- Link length check against measured GIS distance
- Speed/flow relationship
- Link type
- Link capacity
- One way/two way operation
- Number of (effective) lanes
- Length and position of flares
- Any observed turn delays/penalties
- Location of public transport routes
- Access points.

6.1.3 The network errors and warnings generated by SATURN were checked to ensure the model network is free of coding errors.

6.1.4 Traffic loads onto the model network from zones in the form of centroid connectors. The centroid zone connectors in the updated Lowestoft model have been reviewed and refined to realistically represent the way in which traffic joins the road network. In the Fully Modelled Area, where the zoning system is fine, specific access roads from residential and commercial areas have been used as a basis for connecting zones to the network via centroid connectors.

6.1.5 Zones in the External Area, which have a large geographical coverage and significant demand associated with them, are generally connected to major routes to enter the network.

6.2 JUNCTIONS

6.2.1 Each junction included in the ADM network required several parameters as detailed below:

- Lane allocations
- Junction type
- Saturation flows at signal-controlled and priority-controlled junctions
- Signal times, stages and phases
- Circulation and saturation flows at roundabouts
- Gap times.

FLOW/DELAY RELATIONSHIPS

- 6.2.2 Observations on when the eastern Bascule Bridge and western Mutford Bridge were lifted and not open to road traffic were obtained. This information was covered Tuesday 14 April 2015 to Thursday 16 April 2015. Table 6.1 details the observed data for the eastern bridge.

Table 6.1 – Eastern Bascule Bridge observed lifting times

PEAK HOUR	AM OBS OPENING	OPENING TIME	IP OBS OPENING	OPENING TIME	PM OBS OPENING	OPENING TIME
Tues 14 April 2015	0	00:00:00	1	00:04:42	1	00:06:54
Weds 15 April 2015	0	00:00:00	3	00:12:22	0	00:00:00
Thurs 16 April 2015	1	00:04:17	0	00:00:00	0	00:00:00

- 6.2.3 The western Mutford Bridge was observed to only open once during the observation period, for 2 minutes 40 seconds on Wednesday 15 April 2015 at 09:51. As this occurred outside the modelled time periods, no swing bridge delay was included in the model for the western bridge.
- 6.2.4 Table 6.2 shows the timings input into the model for the eastern Bascule Bridge. The delay was coded as a signalised node with a single stage, the red time representing when the swing bridge was lifted and with overall cycle time adding up to 3,600 seconds (1 hour). During the validation process, the AM peak bridge timings were initially coded as 257 seconds of red time to match the observed data, however during the validation process this was reduced to 227 seconds to ensure the flow validation improved on the bridge.

Table 6.2 – Eastern Bascule Bridge timings input into model

PEAK HOUR	GREEN TIME (SECONDS)	RED TIME (SECONDS)
AM peak	3,373	227
Inter peak	3,344	256
PM peak	3,186	414

- 6.2.5 Observations were also obtained for the two level crossings in the vicinity of the western Mutford Bridge – Bridge Road and Victoria Road. As with the swing bridge opening observations, the level crossings were observed between Tuesday 14 April 2015 and Thursday 16 April 2015. Table 6.3 shows the timings input for the Bridge Road level crossing. On average the barrier was down three times in the AM peak at this crossing, therefore the cycle time was set to 1,200 seconds (20 minutes), whereas in the interpeak and PM peak the barrier was down twice in an hour on average, the cycle time was therefore set to 1,800 seconds (30 minutes).

Table 6.3 – Bridge Road level crossing timings input into the model

PEAK HOUR	GREEN TIME (SECONDS)	RED TIME (SECONDS)
AM peak	1,017	183
Inter peak	1,485	315
PM peak	1,653	147

- 6.2.6 Table 6.4 outlines the timings input for the Victoria Road level crossing. On average the barrier was down twice in an hour at this location, therefore the cycle time was set to 1,800 seconds (30 minutes).

Table 6.4 – Victoria Road level crossing timings input into the model

PEAK HOUR	GREEN TIME (SECONDS)	RED TIME (SECONDS)
AM peak	1,693	107
Inter peak	1,553	247
PM peak	1,676	124

SIGNAL TIMINGS

- 6.2.7 Signal timings were extracted from controlled specs which were provided for eight key junctions within the study area, the junctions comprising of:

- Station Square / Denmark Road / Waveney Road
- London Road South / Belvedere Road
- Katwijk Way / Denmark Road
- Waveney Road / Suffolk Road / Battery Green Road
- Tom Crisp Way / Blackheath Road
- Beccles Road / Cotmer Road
- St Peters Street / Boston Road
- London Road / Mill Road.

- 6.2.8 The data from the controller specs were put into basic LinSig models to obtain realistic signal timings, stages and phases. The output results were inputted into the SATURN model for each time period.

SATURATION FLOWS

- 6.2.9 Default saturation flows were used for all junctions within the model. The default saturation flows per lane for priority junctions are:

- Major straight ahead movement (unopposed) – 1,980 pcu/hr
- Major left turn movement (unopposed) – 1,500 pcu/hr
- Major right turn movement (opposed) – 745 pcu/hr
- Minor left turn movement (opposed) 700 pcu/hr
- Minor right turn movement (opposed) – 800 pcu/hr
- Minor straight ahead movement (opposed) – 600 pcu/hr

- 6.2.10 Default saturation flows at signalised junctions are set to:

- Straight ahead movement – 1,980 pcu/hr
- Left or right turn movement – 1,740 pcu/hr

- 6.2.11 By default, SATURN assumes that opposing right turns at signalised junctions are “hooked” i.e. they interfere with each other. At larger junctions where there is sufficient space for traffic to turn right without being affected by the opposing right turn, it is possible to code these turns in the model so they do not interfere with each other. This was implemented at relevant junctions.
- 6.2.12 Roundabouts require special consideration. Unlike with other junction types, each turn needs to be given the total saturation flow for the approach e.g. if a roundabout has a two-lane approach, with one lane to turn left and one to turn right, each turn should be coded with a saturation flow of 2,200. Default saturation flows (pcu/hr) adopted for roundabouts are given in Table 6.5. These values have been adopted to replicate typical ARCADY capacity estimates and have previously been utilised.

Table 6.5 - Roundabout entry capacity saturation flows

APPROACH LANES	NUMBER OF ENTRY LANES			
	1	2	3	4
Single (3.5m)	1,130	1,670	2,030	
Single (5.0m)	1,510	1,940	2,250	2,450
Dual 2 lane		2,200	2,780	3,190
Dual 3 lane			3,330	3,940

- 6.2.13 Equivalent gap acceptance parameters are provided in Table 6.6.

Table 6.6- Roundabout gap acceptance parameters (seconds)

APPROACH LANES	NUMBER OF ENTRY LANES			
	1	2	3	4
Single (3.5m)	1.8	1.3	1.2	
Single (5.0m)	1.4	1.2	1.1	1.1
Dual 2 lane		1.1	1.0	0.9
Dual 3 lane			0.9	0.8

- 6.2.14 The overall circulatory saturation flow was set to be the same as the highest saturation flow on the approach arms of the roundabout. Large gyratory systems were coded as a series of priority junctions for a better representation of journey times through the junction

6.3 LINKS

- 6.3.1 Each link included in the ADM network required several parameters as detailed below:

- Distance
- Speed

- Speed flow curve
- Number of lanes
- Penalties/bans.

SPEED/FLOW RELATIONSHIPS

6.3.2 Highway capacity is restrained by junctions and by the speed-flow curves allocated to links in the study area. Speed flow curves are based on standard COBA 10 values and allocated to specific links based on assessment of the road speed, width and capacity. Speed-flow curves have generally only been used on rural or inter-urban links where the characteristics of the link itself, rather than junction capacity, have an impact on traffic speed. It has been necessary in some circumstances to use speed-flow curves in suburban areas to replicate the impacts of un-modelled minor junctions.

6.3.3 The speed flow curves that were used are shown in Table 6.7.

Table 6.7 - Model speed flow curves

DESCRIPTION	ID	FREE FLOW SPEED	SPEED AT CAPACITY	CAPACITY	POWER VALUE
Rural - D4M Motorway - 4 lanes	1	116	45	9,999	3.8
Rural - D3M Motorway - 3 lanes	2	116	45	7,560	3.8
Rural - D2M Motorway - 2 lanes	3	112	45	4,860	3.9
Rural - D3AP All-purpose - 3 lanes	4	109	45	6,780	3.7
Rural - D2AP All-purpose - 2 lanes	5	105	45	4,360	3.7
Rural - S10(Good) - 2 lanes	6	91	45	3,720	2.2
Rural - S10(Good) - 1 lane	7	91	45	1,860	2.2
Rural - S10(Typical) - 2 lanes	8	84	45	3,320	2.2
Rural - S10(Typical) - 1 lane	9	84	45	1,660	3.1
Rural - S7.3(Good) - 2 lanes	10	87	45	3,280	2.2
Rural - S7.3(Good) - 1 lane	11	87	45	1,640	2.2
Rural - S7.0(Typical) - 2 lanes	12	78	45	2,760	2.1
Rural - S7.0(Typical) - 1 lane	13	78	45	1,380	2.1
Rural - S6.5(Bad) - 2 lanes	14	67	45	2,020	1.8
Rural - S6.5(Bad) - 1 lane	15	67	45	1,010	1.8
Suburban - Dual(Slight level)	16	78	35	3,460	3.3
Suburban - Single(Slight level)	19	68	25	3,460	3.7

DESCRIPTION	ID	FREE FLOW SPEED	SPEED AT CAPACITY	CAPACITY	POWER VALUE
Suburban - Single(Slight level)	20	68	25	1,730	3.7
Suburban - Dual(Typical level)	21	61	25	2,540	2.3
Suburban - Dual(Typical level)	22	61	25	1,570	2.3
Suburban - Dual(Heavy level)	23	48	25	1,000	1.6
Suburban - Dual(Heavy level)	24	48	25	500	1.6
Urban - Non-central(Good) - 2 lanes	25	54	25	1,960	1.7
Urban - Non-central(Good) - 1 lane	26	54	25	980	1.7
Urban - Non-central(Typical) - 2 lanes	27	49	25	1,560	1.6
Urban - Non-central(Typical) - 1 lane	28	49	25	780	1.6
Urban - Non-central(Poor) - 2 lanes	29	45	25	1,300	1.5
Urban - Non-central(Poor) - 1 lane	30	45	25	650	1.5
Urban - Central(Good) - 2 lanes	31	37	15	1,480	1.8
Urban - Central(Good) - 1 lane	32	37	15	740	1.8
Urban - Central(Typical) - 2 lanes	33	34	15	1,260	1.7
Urban - Central(Typical) - 1 lane	34	34	15	630	1.7
Urban - Central(Poor) - 2 lanes	35	29	15	900	1.6
Urban - Central(Poor) - 1 lane	36	29	15	450	1.6
Small Town - Light level - 2 lanes	37	66	30	2,600	3.0
Small Town - Light level - 1 lane	38	66	30	1,300	3.0
Small Town - Typical level - 2 lanes	39	57	30	2,000	3.4
Small Town - Typical level - 1 lane	40	57	30	1,000	3.4
Small Town - Heavy level - 2 lanes	41	47	30	1,760	2.5
Small Town - Heavy level - 1 lane	42	47	30	880	2.5
Suburban - Single(Slight level)	43	78	35	1,730	3.7
Centroid Connector - Internal	50	87	87	9,999	3.3

FIXED SPEEDS

- 6.3.4 Within the urban area of the model speed flow curves were not necessary due to capacity restraints from the junctions at either end of the link.
- 6.3.5 These links were given fixed speeds based on their individual speed limit as obtained from imagery and site visits.
- 6.3.6 These speeds will reflect the free flow speed whilst the delay at junctions will reflect the conditions in busier periods.

7 TRIP MATRIX DEVELOPMENT

7.1 TRAVEL DEMAND DATA

7.1.1 The matrix was initially built from observed data based on the ANPR surveys detailed in section 5. ANPR data was filtered and factored to match the associated MCC. Following assignment of the observed matrix using select link analysis, a gravity model was devised utilising synthetic trip ends based on NTEM version 6.2 and the 2011 census. The gravity model produced a synthetic matrix which was then combined with the observed matrix to produce the initial prior matrix.

7.2 PARTIAL TRIP MATRICES FROM SURVEYS

7.2.1 For each site, trip chains were split into the following peak periods:

- AM peak period (07:00-10:00)
- Inter peak period (10:00-16:00)
- PM peak period (16:00-19:00)

7.2.2 Peak periods were used for the AM peak and PM peak rather than peak hours to increase the size of the sample which would be used to infer O-D movements within the study area.

7.2.3 Timestamps were available which detailed the exact time a vehicle was detected by an ANPR camera. Each unique matched vehicle was given an anonymised Vehicle Registration Number (VRN) and its data split into the different ANPR locations based on the first site at which the vehicle was detected. The site at which the vehicle was first detected was taken to be its origin for the purposes of matrix building.

7.2.4 The peak period for a vehicle was based on the timestamp for when it was first detected.

7.2.5 The total travel time for a trip chain was provided within the ANPR data, journeys taking longer than 60 minutes were excluded from the matrix building process. This was done because examination of the data highlighted many instances when vehicles were initially tracked at one location during a specific peak. However the next detection of the vehicle occurred over an hour later, often falling within a later peak period making such examples problematic to infer the origin-destination movement within the prescribed peak periods.

7.2.6 Table 7.1 shows the total number of observed trips with a duration of 60 minutes or less by peak period by the ANPR location at which they were first detected (for matrix purposes taken to be their origin).

Table 7.1 – Total observed ANPR trips (Less than 60 minutes in duration)

SITE	SITE DESCRIPTION	AM PEAK PERIOD (07:00-10:00)			INTER PEAK PERIOD (10:00-16:00)			PM PEAK PERIOD (16:00-19:00)		
		CAR	LGV	HGV	CAR	LGV	HGV	CAR	LGV	HGV
1	A12 London Road	907	156	80	1,569	220	152	927	150	63
2	A146 Beccles Road	788	100	95	1,173	196	175	975	127	69
3	A12 Yarmouth Road	375	42	13	745	97	24	902	75	9
4	Coast Road	36	0	7	79	7	17	48	2	8
5	A12 Pier Terrace (Eastern Bascule bridge)	494	51	27	1,089	102	53	1,044	100	13
6	A146 Bridge Road (Western Mutford bridge)	1,293	175	68	1,488	155	87	806	80	31
7	B1375 Gorleston Road	680	124	40	926	100	60	640	67	23
8	A1117 Millennium Way	449	41	31	695	81	44	412	33	9
9	A12 Yarmouth Road	628	100	70	1,271	133	116	835	84	32
10	A12 Tom Crisp Way	787	96	32	711	70	46	508	37	16
11	B1532 London Road South	379	60	26	499	51	30	326	28	14
12	B1074 Blundeston Road	159	20	9	211	35	20	142	42	6
13	Flixton Road	0	0	0	0	0	0	0	0	0
14	B1531 Waveney Drive	341	34	12	375	22	10	240	18	3
15	North Quay Retail Park	60	6	2	280	12	4	122	12	0
16	Links Road Car Park	4	1	0	13	1	0	3	0	0
17	Swimming Pool Road Car Park	1	0	0	12	0	0	20	0	0
18	Shopping Centre Car Park (Battery Green Rd exit)	0	1	0	48	1	0	25	1	0
19	Shopping Centre Car Park (Gordon Road entry)	0	0	0	5	0	0	0	0	0
20	Surrey St Car Park entry	7	0	0	43	2	0	4	0	0

SITE	SITE DESCRIPTION	AM PEAK PERIOD (07:00-10:00)			INTER PEAK PERIOD (10:00-16:00)			PM PEAK PERIOD (16:00-19:00)		
		CAR	LGV	HGV	CAR	LGV	HGV	CAR	LGV	HGV
21	Surrey St Car Park exit (onto Clapham Road)	0	0	0	38	1	0	9	2	0
22	Clapham Road Car Park	3	0	1	56	0	0	10	0	0
23	St Johns Rd Car Park	1	0	0	4	1	0	4	0	0
24	Kirkley Rise Car Park (Northern access)	3	0	0	12	0	0	10	0	1
25	Kirkley Rise Car Park (Southern access)	1	0	0	2	0	0	2	0	0
26	Kirkley Cliff Road Car Park	0	0	0	0	0	0	1	0	0
27	Claremont Road Car Park	0	0	0	8	1	0	11	0	0
28	Marine Parade Car Park	0	1	0	44	0	0	17	1	0
29	Asda Car Park	135	11	1	282	14	3	161	8	3
Total		7,531	1,019	514	11,678	1,302	841	8,204	867	300

7.2.7 The observed ANPR trips were then factored using MCC totals for the peak hour carried out at each of the ANPR sites. The relevant direction for the MCC was used for factor the ANPR trips depending on whether it was an origin or a destination. For ANPR trips internal to the study area, the MCC directions had to be split proportionally based on the origin and destination totals at each site. Appendix A details the MCC totals used at each site

7.2.8 The MCC totals were used by site for doubly constrained furnishing of the observed ANPR trips taking an average of the origin and destination factor. This produced the observed peak hour matrix totals shown in Table 7.2.

Table 7.2 – Observed ANPR matrix totals following furnishing to MCC totals

USER CLASS	AM PEAK HOUR (08:00-09:00)			INTER PEAK AVG. HOUR (10:00-16:00)			PM PEAK HOUR (17:00-18:00)		
	TOTAL	INTER- ZONALS	INTRA- ZONALS	CAR	LGV	HGV	CAR	LGV	HGV
UC1 – Car	7,175	7,100	75	7,602	7,489	112	9,286	9,239	48
UC2 – LGV	1,115	1,110	5	980	973	7	1,039	1,031	7
UC3 – HGV	298	298	0.00	301	297	4.	148	147	2
Total	8,587	8,508	80	8,882	8,759	123	10,473	10,417	57

- 7.2.9 In order to distribute the ANPR matrices within the model, select link analysis at each ANPR site location was carried out using the 2001 Lowestoft SATURN model. This meant the observed data was distributed within the old zone system used for the 2001 Lowestoft SATURN model.
- 7.2.10 Analysis of the demand for each zone in this matrix showed that two zones adjacent to the eastern Bascule bridge had high numbers of trips associated with them relative to observed data available at these locations:
- Zone 2: covering the docks east of the A12 Battery Green Road / Waveney Road, linking to a single access at the A12 / Suffolk Road signals
 - Zone 9: covering the commercial developments linking onto Commercial Road, linking on to a single access at the A12 Pier Terrace / Commercial Road.
- 7.2.11 Origin and destination totals for these zones were factored to match the arm in the respective MCC site 10 and site 11. The excess traffic was then distributed using select link analysis of the A12 Pier Terrace to ensure the overall matrix total was retained.
- 7.2.12 The observed trip matrix was then converting into the new zone system used for the 2015 Lowestoft SATURN model. Correspondence between the old 2001 zone system and new 2015 zone system was done based on area, but took into account an urban outline boundary covering all built-up areas within the study area. This ensured that for instances in which zones contained large amounts of open space, the trip totals were concentrated only in the built-up area. This ensured land-use density was taken into account when the correspondence between the old and new zone system was carried out.
- 7.2.13 Following this process there was a small loss in the overall number of trips due to rounding issues as shown in Table 7.3

Table 7.3 – Observed ANPR matrix totals following correspondence between 2001 zone system and 2015 zone system

USER CLASS	AM PEAK HOUR (08:00-09:00)			INTER PEAK AVG. HOUR (10:00-16:00)			PM PEAK HOUR (17:00-18:00)		
	TOTAL	INTER-ZONALS	INTRA-ZONALS	CAR	LGV	HGV	CAR	LGV	HGV
UC1 – Car	7,173	6,952	221	7,609	7,322	286	9,286	9,024	262
UC2 – LGV	1,114	1,083	31	981	948	33	1038	1,011	28
UC3 – HGV	298	287	11	301	292	9	149	145	4
Total	8,585	8,322	263	8,891	8,563	323	10,473	10,179	293

- 7.2.14 Intra-zonal trips were removed from the matrix, prior to splitting the car user class into the following three user classes:
- Car commuting
 - Car employers business
 - Car other

- 7.2.15 This was carried out based on trip synthesis factors derived on a zone by zone basis as detailed in section 7.3. As factors were applied to both row and column values on a zone by zone basis this lead to a change in the overall size of the matrix.

Table 7.4 - Summary ANPR matrix totals

USER CLASS	AM PEAK HOUR (08:00-09:00)	INTER PEAK AVG. HOUR (10:00-16:00)	PM PEAK HOUR (17:00-18:00)
UC1 – Car Commuting	4,079	1,417	3,259
UC2 – Car EmpBus	485	499	524
UC3 – Car Other	2,373	5,391	5,232
UC4 – LGV	1,083	948	1,011
UC5 – HGV	287	292	145
Total	8,307	8,548	10,171

7.3 TRIP SYNTHESIS

- 7.3.1 To account for trips that were otherwise unobserved by the ANPR surveys, a synthetic gravity model was created.
- 7.3.2 Peak period trip end totals were calculated for each model zone using trip end information from NTEM version 6.2 accessed via TEMPRO. Intersecting the model zoning system with the NTEM zones, NTEM trip ends were split proportionally to create synthetic trip ends totals for each model zone. 2011 work place zones and census output areas were used to determine the employment and housing numbers in the model zone system, these totals were used to help proportion the NTEM synthetic trip end totals.
- 7.3.3 NTEM trips were output by trip purpose. Three car user classes were made up of the following NTEM trip purposes:
- Car commuting:
 - Home-Based Work
 - Home-Based Education
 - Car employers business:
 - Home-Based Employers Business
 - Non-Home-Based Employers Business
 - Car other:
 - Home-Based Shopping
 - Home-Based Recreation / Social
 - Home-Based Personal Business
 - Home-Based Visiting Friends & Relatives
 - Home-Based Holiday / Day Trip
 - Non-Home-Based Work

- Non-Home-Based Education
- Non-Home-Based Shopping
- Non-Home-Based Recreation / Social
- Non-Home-Based Personal Business
- Non-Home-Based Holiday / Day Trip

7.3.4 The trips were factored from the peak periods available within NTEM to represent a peak hour using factors from local ATC as shown in Table 7.5.:

Table 7.5 – Peak period to peak hour factors

USER CLASS	FACTOR
AM peak period (07:00-10:00) to AM peak hour factor (08:00-09:00)	2.63
Inter peak period (10:00-16:00) to average hour factor	6.00
PM peak period (16:00-18:00) to PM peak hour factor (17:00-18:00)	1.84

7.4 MERGING DATA FROM SURVEYS AND TRIP SYNTHESIS

7.4.1 The observed trip matrix was combined with the synthetic matrix using the gravity model.

7.4.2 The SATURN model was skimmed to produce generalised cost matrices, which were then used to distribute the synthetic trip end totals according to the following formula:

$$T_{ij} = O_i * D_j * \exp(-\beta * C_{ij})$$

7.4.3 Thus a trip (T) between any given origin-destination pair is defined as a function of total origin trips (O), total destination trips (D), generalised cost (C), and a deterrence parameter (β). The resultant trip matrices were assigned, and the costs re-skimmed. This process was repeated until the relative gap between the matrices was less than 1% on three successive iterations.

7.4.4 Due to there being few zero values in the observed matrix, a threshold was set at which trip ends within the gravity model would replace the values in the observed matrix. The thresholds for each peak were set as shown in Table 7.6.

Table 7.6 – Gravity model infill thresholds

PEAK	THRESHOLD
AM peak (08:00-09:00)	5
Inter peak (10:00-16:00)	0.1
PM peak (17:00-18:00)	0.05

7.4.5 Trip length distributions were calculated and compared for known movements in the observed ANPR matrix. The results were analysed and the beta value adjusted with the gravity model re-run to produce an optimal fit between synthetic and observed trips. Attention was also paid to the distribution of the full matrix of synthetic trips to ensure close approximation to likely trip lengths.

7.4.6 The β values used within the gravity model are shown in Table 7.7.

Table 7.7 – Beta (β) values used within gravity model

USER CLASS	AM PEAK HOUR (0800-0900)	INTER PEAK AVG. HOUR (1000-1600)	PM PEAK HOUR (1700-1800)
Car commuting	0.91	0.39	0.9
Car emp business	0.96	0.33	0.9
Car other	0.71	0.3	0.9

7.4.7 The r-squared values that were achieved for each car user class across the three peaks are shown in Table 7.8.

Table 7.8 – R-square results from gravity model

USER CLASS	AM PEAK HOUR (0800-0900)	INTER PEAK AVG. HOUR (1000-1600)	PM PEAK HOUR (1700-1800)
Car commuting	0.729	0.887	0.592
Car emp business	0.697	0.872	0.629
Car other	0.911	0.881	0.838
Car overall	0.816	0.892	0.761

7.4.8 Appendix B contains details of the trip length distribution and changes applied to the matrix by the gravity model.

7.4.9 Following the gravity model infill, the prior matrix totals are shown in Table 7.9.

Table 7.9 – Prior matrix totals

USER CLASS	AM PEAK HOUR (0800-0900)	INTER PEAK AVG. HOUR (1000-1600)	PM PEAK HOUR (1700-1800)
UC1 – Car Commuting	6716	1970	4631
UC2 – Car EmpBus	7401	723	1034
UC3 – Car Other	3529	6513	6850
UC4 – LGV	1083	948	1011
UC5 – HGV	287	292	145
Total	12355	10447	13672

8

NETWORK CALIBRATION AND VALIDATION

8.1 NETWORK CALIBRATION

8.1.1 Network calibration was carried out using the initial prior matrix to assist with checks of the network. The initial checks included:

- Link speeds
- Link flows
- Junction delays
- Volume over Capacity (V/C) ratios.

8.1.2 The junctions were also checked to ensure that the capacity of the junction was not less than the counts at any arms.

8.1.3 The modelled delay was not analysed due to no observed data being collected.

8.1.4 Following on from these adjustments the initial matrix was re-run and in addition the observed flows were checked against the modelled flow to ensure they were not significantly higher or lower.

8.1.5 The delays were rechecked to isolate any that were unacceptably lower than the observed delays.

8.1.6 The routes through the network were checked which focussed on ensuring that the routing over the two existing bridges are correct as well as the other main strategic routes through Lowestoft.

8.1.7 The routes taken by HGV were also checked to confirm that certain links with weight or height restrictions had the appropriate ban on them.

8.1.8 The route choice will be discussed in greater detail in Section 9 of this report.

8.2 NETWORK VALIDATION

8.2.1 The journey time routes were assessed with the initial prior matrix to sense check the time it takes to travel on certain links. This indicated any junctions that had unexpected delays or link speeds that were consistently less than the speed limit.

8.2.2 Journey times that had a difference of greater than 25% versus the observed time were checked to confirm that all the characteristics as set out in Section 6 for both links and junctions were consistent with the actual road network.

8.2.3 The link characteristics were also checked on any links where the observed count and modelled flow had a difference of 25% or more.

8.2.4 These checks allowed full confidence that the model reflected the real situation as close as possible.

9 ROUTE CHOICE CALIBRATION AND VALIDATION

9.1 ROUTE CHOICE CALIBRATION

9.1.1 The generalised costs have an effect on the route choice made by different user class and trip purposes.

9.1.2 Generalised costs were calculated using values of time, GDP growth rates, purpose splits, and vehicle operating costs recommended by the DfT for use in economic appraisals of transport projects in England. These values are presented in the November 2014 TAG data book and follow the guidance within the latest version of WebTAG Unit A1.3. The values calculated for use in the base year models are shown in Table 9.1 and Table 9.2. Table 9.1 outlines the Pence per Minute (PPM) values by peak period and vehicle class, whilst Table 9.2 shows this in terms of Price per Kilometre (PPK).

Table 9.1 - Generalised cost parameters - pence per minute (PPM)

PEAK	CAR – COMMUTING	CAR – EMPLOYERS BUSINESS	CAR – OTHER	LGV	HGV
AM	13.74	46.57	17.49	20.98	21.25
IP	13.63	45.51	18.18	20.98	21.25
PM	13.44	44.78	18.72	20.98	21.25

Table 9.2 - Generalised cost parameters - pence per kilometre (PPK)

PEAK	CAR – COMMUTING	CAR – EMPLOYERS BUSINESS	CAR – OTHER	LGV	HGV
AM	6.70	13.38	6.70	13.66	37.35
IP	6.70	13.38	6.70	13.66	37.35
PM	6.70	13.38	6.70	13.66	37.35

9.1.3 Due to heavy goods vehicles favouring shorter slower routes over the longer faster routes such as trunk roads the HGV routes were looked at in greater detail.

9.1.4 The HGV route check looked at any roads that were unsuitable for HGV such as London Road and the appropriate ban was added to these.

9.2 ROUTE CHOICE VALIDATION

9.2.1 The routes that were chosen to validate the route choice were based on the criteria set out in TAG Unit M3.1 (January 2014):

- Relate to significant number of trips
- Are of significant length or cost

- Pass through areas of interest
- Include both directions of travel
- Link different compass areas
- Coincide with journey time routes as appropriate.

9.2.2 Routes were plotted for all user classes. Guidance presented in section 7.3 of TAG Unit M3.1 (January 2014), with the number of OD pairs determined as follows:

$$\text{Number of OD pairs} = (\text{number of zones})^{0.25} \times \text{number of user classes}$$

Based on the initial proposed zoning system, this equates to 14 routes. The routes that were chosen in the appraisal specification report (Sept 2015) and can be seen in Table 9.3 were used to validate the route choice.

Table 9.3 - OD route checks

ROUTE	ORIGIN	ORIGIN NAME	DESTINATION	DESTINATION NAME
1	102	Corton	119	Katwijk Way, Lowestoft
2			122	Waveney Drive (between A12 & Kirkley Run)
3			130	Borrow Road, Oulton
4			131	Windward Way, Lowestoft
5			136	Pakefield Street, Pakefield
6			143	The Street, Carlton Colville
7			149	Kessingland
8	149	Kessingland	101	Corton Road, Gunton
9			104	Blundeston
10			113	Higher Drive, Normanston
11			114	Spashett Road, Lowestoft
12			120	Rotterdam Road, Lowestoft
13			122	Waveney Drive (between A12 & Kirkley Run)
14			128	A146 Beccles Road (near Burnt Hill Lane), Oulton Broad

- 9.2.3 The results of these routes can be seen in Appendix C.
- 9.2.4 The routes were assessed based on the best fit route taking particular interest in which bridge was used and if the key strategic routes were being utilised for the longer journeys.
- 9.2.5 The O-D trees in Appendix C look at user class 1 (Car – Commuting) for the three time periods.
- 9.2.6 In the AM peak all the routes starting from zone 102 use a logical route. The choice of bridges is accurate especially between Corton to Pakefield (zone 102 and zone 136) and Corton to Oulton (zone 102 and zone 130) while longer trips are using the strategic links for example between Corton to Kessingland (zone 102 and zone 149) and Corton to Carlton Colville (zone 102 and zone 143). The trips between Corton and Waveney Drive (zone 102 and zone 122) use the eastern bascule bridge when either bridge would be appropriate for these trips.
- 9.2.7 The routes starting from Kessingland (zone 149) use the most strategic routes especially for longer journeys such as Kessingland to Gunton (zone 149 and zone 101) and Kessingland to Blundeston (zone 149 and zone 104). The split between the bridges is also acceptable with trips included between Kessingland to Normanston (zone 149 and zone 113) and Kessingland to Rotterdam Road (zone 149 and zone 120) uses the bridge which provides the best fit route.
- 9.2.8 The inter peak shows that some routes have multiple options with traffic using more than one route in some of the OD pairs for example between Corton to Oulton (zone 102 and zone 130) and Kessingland to Gunton (zone 149 and zone 101). The route between Kessingland to Normanston (zone 149 and zone 113) shows a split between both the eastern and western bridges.
- 9.2.9 The PM peak routes also show some route choice between certain OD pairs. All of the routes in the PM peak match the routes in either the AM peak or the interpeak if not both.
- 9.2.10 All of the routes generally remain consistent between the three peaks.
- 9.2.11 User class 2 and user class 3 (Car – Employer’s Business & Car – Other) show the same route choices as user class 1 for all routes in the AM peak and PM peak.
- 9.2.12 The interpeak shows an increase amount of route choice for user class 2 and user class 3. These are all minor route choices and the main strategic routes remain consistent.
- 9.2.13 User class 4 (LGV) has only one small difference compared to the car user classes within the AM peak and PM peak. In the interpeak two routes show a change in local route whilst retaining the same strategic routes.
- 9.2.14 HGV (user class 5) show increased variance in their route choice which is expected. The HGV user class will prioritise distance over time as well as having to avoid banned turns and therefore show some alternative route choice.

10 TRIP MATRIX CALIBRATION AND VALIDATION

10.1 TRIP MATRIX VALIDATION

- 10.1.1 The initial prior matrix was created as explained in Section 7 of this report. The initial prior matrix was assigned within the model and the screenline performance analysed.
- 10.1.2 The observed data was split into calibration and validation counts, the validation counts were not used in any matrix adjustment or matrix estimation.
- 10.1.3 Section 3.2 of TAG Unit M3.1 (January 2014) stipulates modelled flows across screenlines for each vehicle type should be within 5% of observed flows. WebTAG recommends that this should apply to “all, or nearly all” screenlines. However, due to the relatively low overall flows through the screenlines a difference between the modelled and observed flow of within 5% was considered difficult to. Therefore in this instance a GEH across the screenline of 4.0 or below has been considered in this report when looking at screenline performance. This approach is compliant with previous versions of WebTAG. We have applied a threshold of 85% of screenline totals to meet this criterion.

10.1.4

There are six screenlines which are used as part of the validation process and five which is part of the calibration process as set out in section 5 of this report. The results of the screenlines for the AM peak can be seen in Table 10.1.

Table 10.1 - Initial prior matrix screenline validation and calibration results - AM Peak

SCREENLINE			OBSERVED	MODELLED	DIFFERENCE	GEH
ID	Name	Type				
1	Screenline 1 - NB	Calibration	1,304	761	-42%	16.893
2	Screenline 1 - SB	Calibration	1,032	1,019	-1%	0.419
3	Screenline 2 - NB	Calibration	959	1,007	5%	1.525
4	Screenline 2 - SB	Calibration	933	781	-16%	5.202
5	Screenline 3 - NB	Calibration	1,082	1,233	14%	4.434
6	Screenline 3 - SB	Calibration	824	833	1%	0.330
7	Screenline 4 - NB	Calibration	1,086	965	-11%	3.778
8	Screenline 4 - SB	Calibration	705	761	8%	2.078
9	Screenline 5 - EB	Calibration	440	380	-14%	2.982
10	Screenline 5 - WB	Calibration	313	224	-28%	5.438
11	Screenline 6 - EB	Calibration	1,749	1,703	-3%	1.109
12	Screenline 6 - WB	Calibration	1,200	917	-24%	8.691
13	Screenline 7 - EB	Calibration	660	1,199	82%	17.694
14	Screenline 7 - WB	Calibration	687	1,075	56%	13.055
15	Screenline 8 - NB	Calibration	1,585	1,702	7%	2.883
16	Screenline 8 - SB	Calibration	1,096	1,364	24%	7.644
19	Screenline 9 - NB	Validation	2,475	2,682	8%	4.076
20	Screenline 9 - SB	Validation	1,802	1,959	9%	3.620
21	Screenline 10 - NB	Validation	1,639	1,895	16%	6.096
22	Screenline 10 - SB	Validation	1,373	1,586	16%	5.543

10.1.5 The results for the Interpeak can be seen in Table 10.2.

Table 10.2: Initial prior matrix screenline validation and calibration results - Inter Peak

SCREENLINE			OBSERVED	MODELLED	DIFFERENCE	GEH
ID	Name	Type				
1	Screenline 1 - NB	Calibration	963	470	-51%	18.406
2	Screenline 1 - SB	Calibration	935	695	-26%	8.411
3	Screenline 2 - NB	Calibration	884	718	-19%	5.852
4	Screenline 2 - SB	Calibration	864	667	-23%	7.136
5	Screenline 3 - NB	Calibration	1,086	1,247	15%	4.720
6	Screenline 3 - SB	Calibration	1,200	981	-18%	6.621
7	Screenline 4 - NB	Calibration	788	587	-25%	7.659
8	Screenline 4 - SB	Calibration	814	782	-4%	1.142
9	Screenline 5 - EB	Calibration	367	193	-47%	10.407
10	Screenline 5 - WB	Calibration	398	300	-25%	5.254
11	Screenline 6 - EB	Calibration	1,353	1,190	-12%	4.578
12	Screenline 6 - WB	Calibration	1,391	752	-46%	19.530
13	Screenline 7 - EB	Calibration	609	1,089	79%	16.491
14	Screenline 7 - WB	Calibration	610	955	57%	12.328
15	Screenline 8 - NB	Calibration	1,257	1,239	-1%	0.511
16	Screenline 8 - SB	Calibration	1,304	1,456	12%	4.085
19	Screenline 9 - NB	Validation	2,024	1,944	-4%	1.803
20	Screenline 9 - SB	Validation	1,982	2,126	7%	3.181
21	Screenline 10 - NB	Validation	1,365	1,512	11%	3.864
22	Screenline 10 - SB	Validation	1,385	1,615	17%	5.942

10.1.6 The screenline results for the PM peak can be seen in Table 10.3.

Table 10.3 - Initial prior matrix screenline validation and calibration results - PM Peak

SCREENLINE			OBSERVED	MODELLED	DIFFERENCE	GEH
ID	Name	Type				
1	Screenline 1 - NB	Calibration	1,294	751	-42%	16.969
2	Screenline 1 - SB	Calibration	1,450	1,005	-31%	12.704
3	Screenline 2 - NB	Calibration	1,088	985	-9%	3.199
4	Screenline 2 - SB	Calibration	1,212	1,003	-17%	6.287
5	Screenline 3 - NB	Calibration	1,031	1,586	54%	15.353
6	Screenline 3 - SB	Calibration	1,257	1,413	12%	4.265
7	Screenline 4 - NB	Calibration	764	615	-20%	5.676
8	Screenline 4 - SB	Calibration	1,136	1,013	-11%	3.756
9	Screenline 5 - EB	Calibration	404	278	-31%	6.823
10	Screenline 5 - WB	Calibration	650	472	-27%	7.502
11	Screenline 6 - EB	Calibration	1,470	1,618	10%	3.757
12	Screenline 6 - WB	Calibration	1,783	1,340	-25%	11.214
13	Screenline 7 - EB	Calibration	938	1,294	38%	10.648
14	Screenline 7 - WB	Calibration	690	1,086	57%	13.271
15	Screenline 8 - NB	Calibration	1,443	1,434	-1%	0.234
16	Screenline 8 - SB	Calibration	1,624	1,999	23%	8.817
19	Screenline 9 - NB	Validation	2,218	2,354	6%	2.844
20	Screenline 9 - SB	Validation	2,724	2,988	10%	4.940
21	Screenline 10 - NB	Validation	1,753	1,854	6%	2.367
22	Screenline 10 - SB	Validation	1,713	2,049	20%	7.748

10.2 TRIP MATRIX CALIBRATION

ADJUSTED PRIOR MATRIX

- 10.2.1 To improve on these results scaling was used on a selection of the calibration counts to produce an adjusted prior matrix. This looked at the difference between the modelled and observed data and adjusted the matrix to either add or remove trips between OD pairs which used those routes.
- 10.2.2 The counts on the eastern and western bridge were not scaled as these already had acceptable GEH values and could act as a check that the matrix was not being distorted by the scaling process.
- 10.2.3 Table 10.4 shows the number of trips in each user class for the initial and adjusted prior matrix.

Table 10.4 - Pre and post prior matrix adjustment trip totals

USER CLASS	AM PEAK HOUR (08:00-09:00)		INTER PEAK AVG. HOUR (10:00-16:00)		PM PEAK HOUR (17:00-18:00)	
	INITIAL PRIOR	ADJ PRIOR	INITIAL PRIOR	ADJ PRIOR	INITIAL PRIOR	ADJ PRIOR
UC1 – Car Commuting	6,716	5,735	1,970	1,724	4,631	4,019
UC2 – Car EmpBus	741	646	723	693	1,034	932
UC3 – Car Other	3,529	3,491	6,513	6,183	6,850	6,166
UC4 – LGV	1,083	2,454	948	2,760	1,011	3,019
UC5 – HGV	287	842	292	814	145	416
Total	12,355	13,167	10,447	12,174	13,672	14,553

10.2.4

Table 10.5 to Table 10.7 show the screenline performance of the adjusted prior matrix. These results show a major improvement in how the modelled flows match the observed screenlines, with the majority of screenlines showing a GEH of 4.0 or below.

Table 10.5 - Adjusted prior matrix screenline validation and calibration results - AM Peak

SCREENLINE			OBSERVED	MODELLED	DIFFERENCE	GEH
ID	Name	Type				
1	Screenline 1 - NB	Calibration	1,304	1,157	-11%	4.185
2	Screenline 1 - SB	Calibration	1,032	1,352	31%	9.256
3	Screenline 2 - NB	Calibration	959	897	-6%	2.041
4	Screenline 2 - SB	Calibration	933	951	2%	0.576
5	Screenline 3 - NB	Calibration	1,082	960	-11%	3.823
6	Screenline 3 - SB	Calibration	824	840	2%	0.572
7	Screenline 4 - NB	Calibration	1,086	1,003	-8%	2.568
8	Screenline 4 - SB	Calibration	705	723	3%	0.683
9	Screenline 5 - EB	Calibration	440	422	-4%	0.886
10	Screenline 5 - WB	Calibration	313	263	-16%	2.953
11	Screenline 6 - EB	Calibration	1,749	1,701	-3%	1.157
12	Screenline 6 - WB	Calibration	1,200	1,232	3%	0.925
13	Screenline 7 - EB	Calibration	660	747	13%	3.296
14	Screenline 7 - WB	Calibration	687	695	1%	0.287
15	Screenline 8 - NB	Calibration	1,585	1,503	-5%	2.090
16	Screenline 8 - SB	Calibration	1,096	1,075	-2%	0.634
19	Screenline 9 - NB	Validation	2,475	2,370	-4%	2.133
20	Screenline 9 - SB	Validation	1,802	1,812	1%	0.235
21	Screenline 10 - NB	Validation	1,639	1,540	-6%	2.477
22	Screenline 10 - SB	Validation	1,373	1,289	-6%	2.297

Table 10.6 - Adjusted prior matrix screenline validation and calibration results - Inter Peak

SCREENLINE			OBSERVED	MODELLED	DIFFERENCE	GEH
ID	Name	Type				
1	Screenline 1 - NB	Calibration	963	1,076	12%	3.551
2	Screenline 1 - SB	Calibration	935	918	-2%	0.562
3	Screenline 2 - NB	Calibration	884	996	13%	3.666
4	Screenline 2 - SB	Calibration	864	943	9%	2.613
5	Screenline 3 - NB	Calibration	1,086	1,117	3%	0.940
6	Screenline 3 - SB	Calibration	1,200	1,163	-3%	1.066
7	Screenline 4 - NB	Calibration	788	697	-12%	3.332
8	Screenline 4 - SB	Calibration	814	828	2%	0.480
9	Screenline 5 - EB	Calibration	367	281	-23%	4.786
10	Screenline 5 - WB	Calibration	398	422	6%	1.177
11	Screenline 6 - EB	Calibration	1,353	1,162	-14%	5.393
12	Screenline 6 - WB	Calibration	1,391	1,386	0%	0.144
13	Screenline 7 - EB	Calibration	609	697	15%	3.462
14	Screenline 7 - WB	Calibration	610	642	5%	1.273
15	Screenline 8 - NB	Calibration	1,257	1,360	8%	2.846
16	Screenline 8 - SB	Calibration	1,304	1,419	9%	3.110
19	Screenline 9 - NB	Validation	2,024	1,899	-6%	2.830
20	Screenline 9 - SB	Validation	1,982	2,181	10%	4.366
21	Screenline 10 - NB	Validation	1,365	1,350	-1%	0.418
22	Screenline 10 - SB	Validation	1,385	1,281	-8%	2.845

Table 10.7 - Adjusted prior matrix screenline validation and calibration results - PM Peak

SCREENLINE			OBSERVED	MODELLED	DIFFERENCE	GEH
ID	Name	Type				
1	Screenline 1 - NB	Calibration	1,294	1,385	7%	2.499
2	Screenline 1 - SB	Calibration	1,450	1,180	-19%	7.448
3	Screenline 2 - NB	Calibration	1,088	1,142	5%	1.618
4	Screenline 2 - SB	Calibration	1,212	1,235	2%	0.650
5	Screenline 3 - NB	Calibration	1,031	1,071	4%	1.244
6	Screenline 3 - SB	Calibration	1,257	1,268	1%	0.305
7	Screenline 4 - NB	Calibration	764	719	-6%	1.654
8	Screenline 4 - SB	Calibration	1,136	1,126	-1%	0.301
9	Screenline 5 - EB	Calibration	404	404	0%	0.000
10	Screenline 5 - WB	Calibration	650	596	-8%	2.150
11	Screenline 6 - EB	Calibration	1,470	1,357	-8%	3.015
12	Screenline 6 - WB	Calibration	1,783	1,692	-5%	2.186
13	Screenline 7 - EB	Calibration	938	1,006	7%	2.172
14	Screenline 7 - WB	Calibration	690	640	-7%	1.957
15	Screenline 8 - NB	Calibration	1,443	1,483	3%	1.049
16	Screenline 8 - SB	Calibration	1,624	1,682	4%	1.433
19	Screenline 9 - NB	Validation	2,218	2,203	-1%	0.319
20	Screenline 9 - SB	Validation	2,724	2,765	2%	0.783
21	Screenline 10 - NB	Validation	1,753	1,608	-8%	3.548
22	Screenline 10 - SB	Validation	1,713	1,529	-11%	4.570

10.3 TRIP MATRIX ESTIMATION

- 10.3.1 The matrix estimation process employed as part of the calibration process is designed to modify the travel patterns using the observed traffic counts. Trips are adjusted in the matrix to produce the estimated matrix, which is most likely to be consistent with the traffic counts. The matrix of trips input to matrix estimation is known as the prior matrix, while the output matrix from matrix estimation is known as the post matrix. The calibration process has used matrix estimation procedures as contained in the SATME2 program in the SATURN suite of software.
- 10.3.2 SATME2 requires a PIJA file which represents the proportion (P) of trips between a particular origin-destination pair (IJ) which uses the counted link (A). The PIJA data is obtained through the program SATPIJA following a SATURN assignment using the SAVEIT option.
- 10.3.3 This produces PIJA output files for each user class which are used by SATME2 along with the prior matrix to produce an updated 2015 estimated highway demand matrices which were then combined into a 'stacked' estimated matrix for assignment. No cells were frozen and there were no zonal constraints applied.
- 10.3.4 Matrix estimation often involves an iterative process, where the first post matrix is assigned to the network and is used to update assignment costs, creating a new set of PIJA factors to create a second post matrix. This process can continue to be repeated, updating assignment costs but retaining the original prior matrix each time to prevent undue distortion of observed trip patterns. The benefit of this approach is that the post matrix will contain a better representation of the PIJA factors on counted links than the prior matrix assignment, which should result in an improved post matrix the second time around.
- 10.3.5 Further iterations can be undertaken, but typically after 3 or 4 iterations the additional benefits in terms of improved goodness of fit are small. There are no specific convergence criteria for matrix estimation, but the aim of the procedure is to improve the goodness of fit between modelled flows and counts.
- 10.3.6 The matrix estimation was carried out using the calibration counts only based on the scaled matrices for each time period.

- 10.3.7 Table 10.8 compares the matrix totals for the adjusted prior and post matrix estimation matrices for each modelled peak hour by user class. Overall, following matrix estimation, the matrix increased by 0.76% in the AM peak, there was a decrease of 1% in the inter peak and an increase of 0.87% in the PM peak.

Table 10.8: Prior and Post Matrix Totals

USER CLASS	AM PEAK HOUR (0800-0900)		INTER PEAK AVG. HOUR (1000-1600)		PM PEAK HOUR (1700-1800)	
	ADJ PRIOR	POST ME	ADJ PRIOR	POST ME	ADJ PRIOR	POST ME
UC1 – Car Commuting	5,735	5,447	1,724	1,696	4,019	3,964
UC2 – Car EmpBus	646	622	693	658	932	904
UC3 – Car Other	3,491	3,474	6,183	5,954	6,166	6,141
UC4 – LGV	2,454	2,855	2,760	2,919	3,019	3,215
UC5 – HGV	842	868	814	825	416	456
Total	13,167	13,267	12,174	12,052	14,553	14,680

- 10.3.8 Appendix D contains a breakdown of the individual count performance within the screenlines for the final post-ME assignment.

10.3.9 Table 10.9 to Table 10.11 detail the screenline performance of the post ME assignment.

Table 10.9: Post ME screenline validation and calibration results - AM Peak

SCREENLINE			OBSERVED	MODELLED	DIFFERENCE	GEH
ID	Name	Type				
1	Screenline 1 - NB	Calibration	1,304	1,277	-2%	0.746
2	Screenline 1 - SB	Calibration	1,032	1,139	10%	3.235
3	Screenline 2 - NB	Calibration	959	960	0%	0.027
4	Screenline 2 - SB	Calibration	933	937	0%	0.121
5	Screenline 3 - NB	Calibration	1,082	1,085	0%	0.086
6	Screenline 3 - SB	Calibration	824	795	-3%	1.002
7	Screenline 4 - NB	Calibration	1,086	1,041	-4%	1.380
8	Screenline 4 - SB	Calibration	705	693	-2%	0.444
9	Screenline 5 - EB	Calibration	440	438	-1%	0.114
10	Screenline 5 - WB	Calibration	313	298	-5%	0.864
11	Screenline 6 - EB	Calibration	1,749	1,599	-9%	3.668
12	Screenline 6 - WB	Calibration	1,200	1,238	3%	1.096
13	Screenline 7 - EB	Calibration	660	644	-2%	0.611
14	Screenline 7 - WB	Calibration	687	688	0%	0.020
15	Screenline 8 - NB	Calibration	1,585	1,581	0%	0.103
16	Screenline 8 - SB	Calibration	1,096	1,097	0%	0.033
19	Screenline 9 - NB	Validation	2,475	2,368	-4%	2.174
20	Screenline 9 - SB	Validation	1,802	1,670	-7%	3.168
21	Screenline 10 - NB	Validation	1,639	1,518	-7%	3.040
22	Screenline 10 - SB	Validation	1,373	1,293	-6%	2.186

Table 10.10: Post ME screenline validation and calibration results - Inter Peak

SCREENLINE			OBSERVED	MODELLED	DIFFERENCE	GEH
ID	Name	Type				
1	Screenline 1 - NB	Calibration	963	926	-4%	1.192
2	Screenline 1 - SB	Calibration	935	894	-4%	1.360
3	Screenline 2 - NB	Calibration	884	887	0%	0.114
4	Screenline 2 - SB	Calibration	864	907	5%	1.430
5	Screenline 3 - NB	Calibration	1,086	1,127	4%	1.238
6	Screenline 3 - SB	Calibration	1,200	1,198	0%	0.047
7	Screenline 4 - NB	Calibration	788	764	-3%	0.854
8	Screenline 4 - SB	Calibration	814	781	-4%	1.177
9	Screenline 5 - EB	Calibration	367	368	0%	0.044
10	Screenline 5 - WB	Calibration	398	400	0%	0.092
11	Screenline 6 - EB	Calibration	1,353	1,289	-5%	1.768
12	Screenline 6 - WB	Calibration	1,391	1,381	-1%	0.278
13	Screenline 7 - EB	Calibration	609	627	3%	0.742
14	Screenline 7 - WB	Calibration	610	612	0%	0.075
15	Screenline 8 - NB	Calibration	1,257	1,291	3%	0.951
16	Screenline 8 - SB	Calibration	1,304	1,305	0%	0.021
19	Screenline 9 - NB	Validation	2,024	1,912	-6%	2.532
20	Screenline 9 - SB	Validation	1,982	2,064	4%	1.827
21	Screenline 10 - NB	Validation	1,365	1,285	-6%	2.209
22	Screenline 10 - SB	Validation	1,385	1,228	-11%	4.340

Table 10.11: Post ME screenline validation and calibration results - PM Peak

SCREENLINE			OBSERVED	MODELLED	DIFFERENCE	GEH
ID	Name	Type				
1	Screenline 1 - NB	Calibration	1,294	1,324	2%	0.842
2	Screenline 1 - SB	Calibration	1,450	1,368	-6%	2.187
3	Screenline 2 - NB	Calibration	1,088	1,092	0%	0.122
4	Screenline 2 - SB	Calibration	1,212	1,256	4%	1.245
5	Screenline 3 - NB	Calibration	1,031	999	-3%	0.994
6	Screenline 3 - SB	Calibration	1,257	1,229	-2%	0.798
7	Screenline 4 - NB	Calibration	764	749	-2%	0.547
8	Screenline 4 - SB	Calibration	1,136	1,102	-3%	1.020
9	Screenline 5 - EB	Calibration	404	399	-1%	0.250
10	Screenline 5 - WB	Calibration	650	623	-4%	1.057
11	Screenline 6 - EB	Calibration	1,470	1,332	-9%	3.697
12	Screenline 6 - WB	Calibration	1,783	1,755	-2%	0.669
13	Screenline 7 - EB	Calibration	938	951	1%	0.414
14	Screenline 7 - WB	Calibration	690	673	-3%	0.669
15	Screenline 8 - NB	Calibration	1,443	1,480	3%	0.971
16	Screenline 8 - SB	Calibration	1,624	1,699	5%	1.846
19	Screenline 9 - NB	Validation	2,218	2,185	-1%	0.703
20	Screenline 9 - SB	Validation	2,724	2,600	-5%	2.403
21	Screenline 10 - NB	Validation	1,753	1,570	-10%	4.501
22	Screenline 10 - SB	Validation	1,713	1,477	-14%	5.909

- 10.3.10 In the AM peak, all screenlines are shown to perform well with a GEH below 4.
- 10.3.11 The interpeak performs similarly with all the calibration screenlines achieving a GEH of less than 4. Screenline 10 southbound does not meet these criteria is close with a GEH value of 4.340.
- 10.3.12 The PM peak again has all the calibration screenlines meeting the criteria whilst validation screenline 10 is slightly over with a GEH of 4.501 in the northbound direction. In the southbound direction, screenline 10 has a higher GEH of 5.909. This is due to ATC 10 (A146 Beccles Road) showing a high GEH in the south-west bound direction. The other two ATCs (ATC 5 on A1145 Castleton and A12 London Road) show the modelled flow closely matches the observed flow with GEH of 0.3 and 2.2 respectively.
- 10.3.13 Overall the screenline results echo the results of the link flows in indicating that the model represents and accurately reflects the observed data and indicates that the key movements around the model are accurate.

IMPACT OF MATRIX ESTIMATION

- 10.3.14 It is important to look at the different in the trip length distribution between the adjusted prior matrix assignment and post ME matrix assignment.
- 10.3.15 Figure 10.1, Figure 10.2 and Figure 10.3 show the trip length distributions for each peak hour model graphically comparing the adjusted prior assignment to the post matrix estimation assignment. The graphs show matrix estimation does not fundamentally alter the trip length distribution between the two assignments.

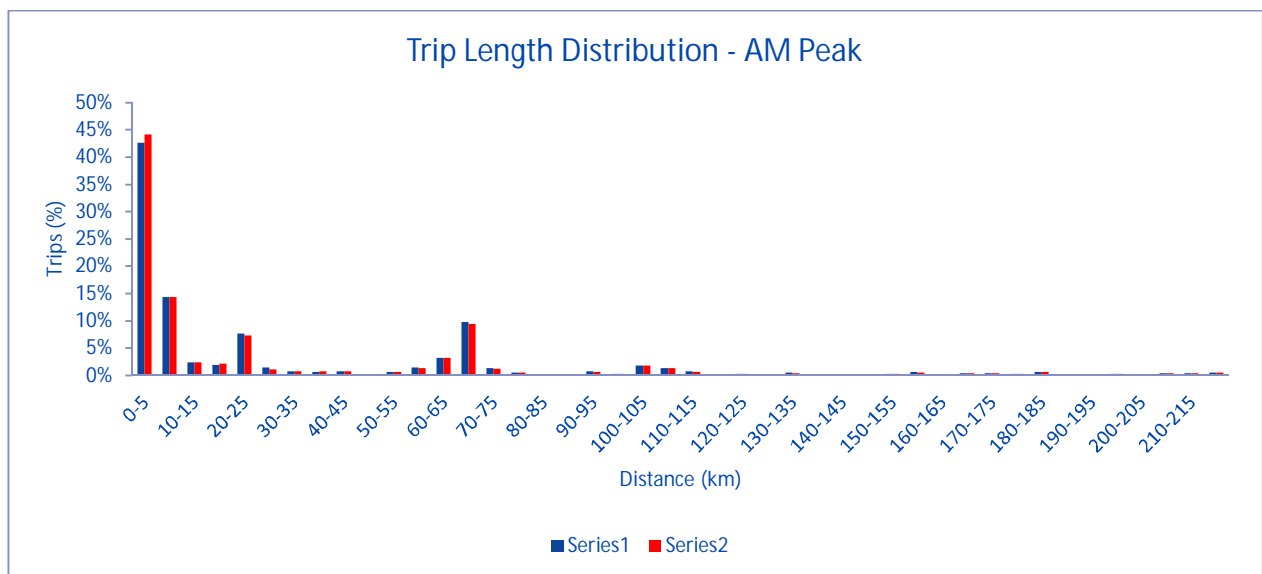


Figure 10.1: AM peak trip length distribution

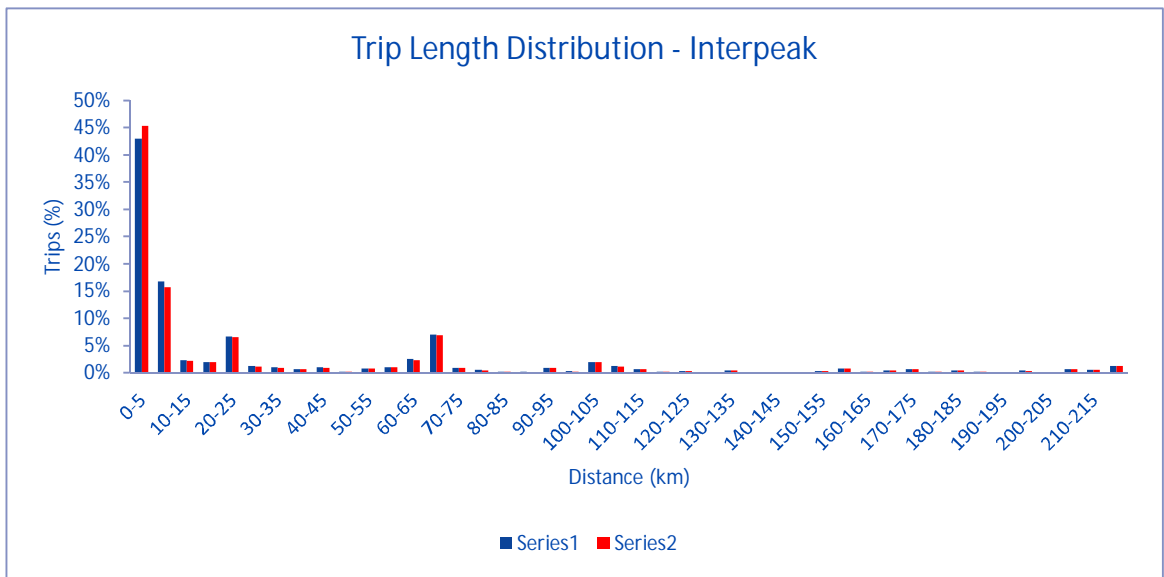


Figure 10.2: Inter peak trip length distribution

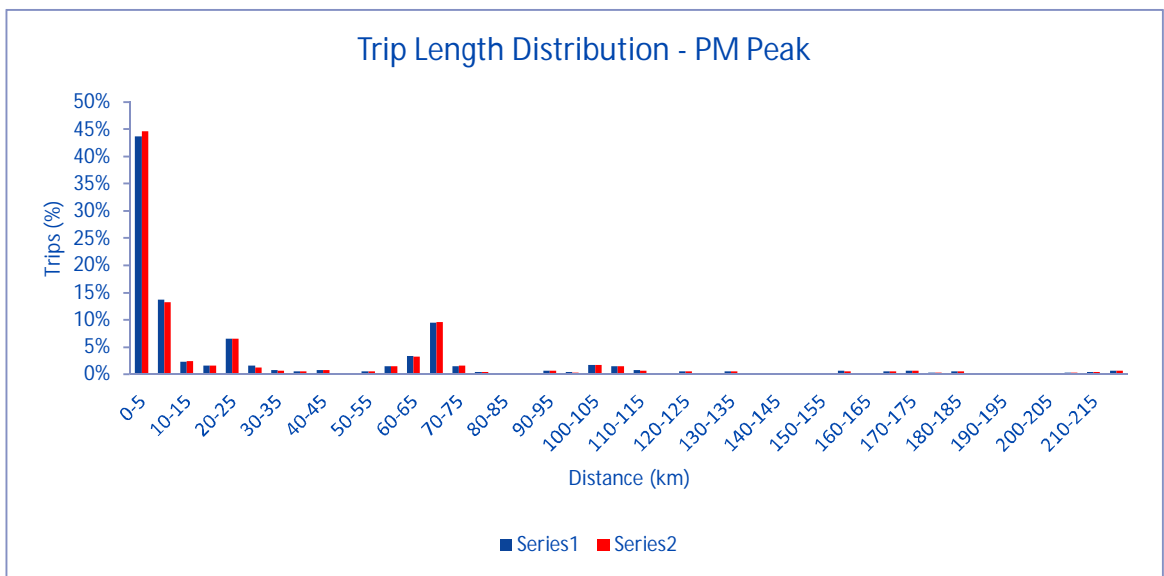


Figure 10.3: PM peak trip length distribution

Table 10.12: Regression statistics AM Peak

MEASUREMENT		REQUIREMENT	VALUE	PASS?
Cells	Slope	Within 0.98 and 1.02	0.965	No
	Intercept	Near 0	0.035	Yes
	R-Sq	> 0.95	0.8341	No
Rows	Slope	Within 0.99 and 1.01	1.007	Yes
	Intercept	Near 0	0.019	Yes
	R-Sq	> 0.98	0.9752	No
Columns	Slope	Within 0.99 and 1.01	0.938	No
	Intercept	Near 0	7.221	No
	R-Sq	> 0.98	0.9569	No
Mean	Prior	Within 5%	32.033	Yes
	Post		30.914	
	Diff		3.5%	
SD	Prior	Within 5%	54.763	Yes
	Post		54.353	
	Diff		0.7%	

Table 10.13: Regression statistics Inter Peak

MEASUREMENT		REQUIREMENT	VALUE	PASS?
Cells	Slope	Within 0.98 and 1.02	0.944	No
	Intercept	Near 0	0.034	Yes
	R-Sq	> 0.95	0.9304	No
Rows	Slope	Within 0.99 and 1.01	0.96	No
	Intercept	Near 0	2.876	No
	R-Sq	> 0.98	0.99	Yes
Columns	Slope	Within 0.99 and 1.01	0.959	No
	Intercept	Near 0	2.942	No
	R-Sq	> 0.98	0.9854	Yes
Mean	Prior	Within 5%	33.083	Yes
	Post		31.943	
	Diff		3.4%	
SD	Prior	Within 5%	57.523	Yes
	Post		54.353	
	Diff		0.7%	

Table 10.14: Regression statistics PM Peak

MEASUREMENT		REQUIREMENT	VALUE	PASS?
Cells	Slope	Within 0.98 and 1.02	0.989	Yes
	Intercept	Near 0	0.02	Yes
	R-Sq	> 0.95	0.8325	No
Rows	Slope	Within 0.99 and 1.01	0.959	No
	Intercept	Near 0	5.983	No
	R-Sq	> 0.98	0.9782	No
Columns	Slope	Within 0.99 and 1.01	1.036	No
	Intercept	Near 0	-2.913	No
	R-Sq	> 0.98	0.975	No
Mean	Prior	Within 5%	32.999	Yes
	Post		32.688	
	Diff		0.9%	
SD	Prior	Within 5%	52.491	Yes
	Post		52.143	
	Diff		0.7%	

10.3.16 The regression statistics show that for all three time periods both the mean and standard deviation are both within the criteria of 5%.

10.3.17 The sector to sector movements were analysed to ensure the matrix estimation process did not distort the matrix.

10.3.18 Table 10.15 shows the percentage difference in sector to sector movements for the AM peak.

Table 10.15: Sector to sector movement results - AM Peak

	1	2	3	4	5	TOTAL
1	0%	-13%	0%	0%	-4%	-2%
2	0%	-10%	-8%	-9%	11%	-3%
3	0%	-6%	0%	9%	-7%	2%
4	-6%	-4%	-7%	4%	-11%	-2%
5	2%	2%	-18%	8%	8%	5%
Total	-2%	-3%	-9%	4%	1%	1%

10.3.19 The interpeak results can be seen in Table 10.16.

Table 10.16: Sector to sector movement results - Inter Peak

	1	2	3	4	5	TOTAL
1	0%	-6%	0%	2%	-13%	-3%
2	0%	-5%	-5%	-6%	-3%	-5%
3	-3%	-7%	0%	3%	-10%	-2%
4	-6%	-9%	-2%	1%	-5%	-2%
5	-6%	-3%	-12%	5%	3%	2%
Total	-5%	-6%	-4%	2%	-2%	-1%

10.3.20 The results for the PM peak can be seen in Table 10.17.

Table 10.17: Sector to sector movement results - PM Peak

	1	2	3	4	5	TOTAL
1	0%	-8%	0%	-1%	-15%	-5%
2	10%	2%	4%	-12%	-2%	-5%
3	0%	-9%	0%	9%	-18%	-3%
4	7%	-2%	6%	5%	-2%	2%
5	7%	7%	6%	0%	3%	3%
Total	6%	-1%	5%	3%	-2%	1%

11 ASSIGNMENT CALIBRATION AND VALIDATION

11.1 MODEL CONVERGENCE

11.1.1 The model convergence criterion has been set out in Section 3.2.

11.1.2 Table 11.1, table 11.2 and table 11.3 show the convergence statistics for the AM peak, interpeak and PM peak respectively. The results show that all the models converge within the guidance that was set out in Table 3.4.

Table 11.1 - AM peak convergence results

ITERATION	DELTA	%FLOW	%GAP
11	0.0006	95.4	0.00045
12	0.0002	96.8	0.00026
13	0.0001	98.4	0.00023
14	0.0001	99	0.00013
15	0.0002	99.6	0.00009
16	0.0001	99.5	0.00006

Table 11.2 - Inter peak convergence results

ITERATION	DELTA	%FLOW	%GAP
9	0.0136	95.3	0.01
10	0.0071	97.3	0.012
11	0.0083	98.4	0.0064
12	0.0046	99.2	0.008
13	0.0062	99.1	0.0043
14	0.0033	99.4	0.0058

Table 11.3 - PM peak convergence results

ITERATION	DELTA	%FLOW	%GAP
26	0.0016	99.5	0.0042
27	0.0044	97.2	0.0017
28	0.0011	98.3	0.0018
29	0.0015	99.1	0.0027
30	0.0017	99.8	0.00089
31	0.0009	98.5	0.0013

11.2 ASSIGNMENT CALIBRATION

11.2.1 Assignment calibration involved steps to identify any issues that prevented an acceptable level of calibration of the network, route choice and trip matrix. This will included:

- Checking appropriateness of centroid connectors
- Production of forests to understand nature of competing routes between OD pairs
- Checking representation of queues on surveyed journey time routes

11.3 ASSIGNMENT VALIDATION

11.3.1 Link flow validation and calibration results for the final post-matrix estimation show an improved situation compared to the scaled matrix.

OVERALL MODEL PERFORMANCE

11.3.2 The calibration and validation results for all user classes in the AM peak are shown in table 11.4. The results for the car as an individual user class are shown in table 11.5.

Table 11.4 - AM Peak hour all user classes calibration and validation results

CRITERIA AND MEASURE			CALIBRATION			VALIDATION		
			ACCEPTABILITY GUIDELINE	Total Counts	Meet Criteria	%	Total Counts	Meet Criteria
Flow Criteria								
< 700 vph	±100 vph	> 85 % of links	43	43	100%	38	32	84%
700 - 2,700 vph	±15%	> 85 % of links	3	3	100%	5	5	100%
> 2,700 vph	±400 vph	> 85 % of links	0	0	0%	0	0	0%
GEH Criteria								
GEH Statistic for individual links < 5		> 85 % of links	46	46	100%	43	36	84%

11.3.3

The AM peak results show that 100% of the calibration links and 84% of the validation links pass the criteria set out in WebTAG to achieve GEH values with a value of less than 5. The car only results show a very similar pattern.

Table 11.5 - AM Peak hour car only calibration and validation results

CRITERIA AND MEASURE			CALIBRATION			VALIDATION		
			ACCEPTABILITY GUIDELINE	Total Counts	Meet Criteria	%	Total Counts	Meet Criteria
Flow Criteria								
< 700 vph	±100 vph	> 85 % of links	44	44	100%	43	38	88%
700 - 2,700 vph	±15%	> 85 % of links	2	2	100%	0	0	0%
> 2,700 vph	±400 vph	> 85 % of links	0	0	0%	0	0	0%
GEH Criteria								
GEH Statistic for individual links < 5		> 85 % of links	46	46	100%	43	36	84%

- 11.3.4 The interpeak results for all user classes can be seen in Table 11.6 whilst car only is seen in Table 11.7.

Table 11.6 - Inter Peak hour all user classes calibration and validation results

CRITERIA AND MEASURE			ACCEPTABILITY GUIDELINE			CALIBRATION			VALIDATION		
						Total Counts	Meet Criteria	%	Total Counts	Meet Criteria	%
Flow Criteria											
< 700 vph	±100 vph	> 85 % of links	44	44	100%	39	34	87%			
700 - 2,700 vph	±15%	> 85 % of links	2	2	100%	4	4	100%			
> 2,700 vph	±400 vph	> 85 % of links	0	0	0%	0	0	0%			
GEH Criteria											
GEH Statistic for individual links < 5		> 85 % of links	46	46	100%	43	38	88%			

- 11.3.5 The Interpeak results show that both the calibration and validation counts have at least 85% of links with a GEH value of less than 5. However, for cars the validation percentage is at 79% of links with a GEH below 5. This is not considered an issue as in terms of flow validation the car user class is at 88% of links within 100 pcus of the observed count.

Table 11.7 - Inter Peak hour car only calibration and validation results

CRITERIA AND MEASURE			ACCEPTABILITY GUIDELINE			CALIBRATION			VALIDATION		
						Total Counts	Meet Criteria	%	Total Counts	Meet Criteria	%
Flow Criteria											
< 700 vph	±100 vph	> 85 % of links	44	44	100%	42	37	88%			
700 - 2,700 vph	±15%	> 85 % of links	2	2	100%	1	0	0%			
> 2,700 vph	±400 vph	> 85 % of links	0	0	0%	0	0	0%			
GEH Criteria											
GEH Statistic for individual links < 5		> 85 % of links	46	46	100%	43	34	79%			

11.3.6

The PM results below show that all of the calibration links have a GEH of less than 5. The car only also shows a good level of calibration and can be seen in Table 11.9. In terms of validation, 77% of links have a GEH below 5 which is marginally outside of WebTAG criteria.

Table 11.8 - PM Peak hour all user classes calibration and validation results

CRITERIA AND MEASURE			ACCEPTABILITY GUIDELINE	CALIBRATION			VALIDATION		
				Total Counts	Meet Criteria	%	Total Counts	Meet Criteria	%
Flow Criteria									
< 700 vph	±100 vph	> 85 % of links	38	38	100%	37	28	76%	
700 - 2,700 vph	±15%	> 85 % of links	8	8	100%	6	5	83%	
> 2,700 vph	±400 vph	> 85 % of links	0	0	0%	0	0	0%	
GEH Criteria									
GEH Statistic for individual links < 5		> 85 % of links	46	46	100%	43	33	77%	

Table 11.9 - PM peak hour car only calibration and validation results

CRITERIA AND MEASURE			ACCEPTABILITY GUIDELINE	CALIBRATION			VALIDATION		
				Total Counts	Meet Criteria	%	Total Counts	Meet Criteria	%
Flow Criteria									
< 700 vph	±100 vph	> 85 % of links	44	44	100%	40	33	83%	
700 - 2,700 vph	±15%	> 85 % of links	2	2	100%	3	3	100%	
> 2,700 vph	±400 vph	> 85 % of links	0	0	0%	0	0	0%	
GEH Criteria									
GEH Statistic for individual links < 5		> 85 % of links	46	44	96%	43	36	84%	

- 11.3.7 The matrix estimation process has increased the calibration counts to 100% of links having a GEH value of less than 5 for all three peaks. Despite the percentage of validation counts with a GEH below 5 not reaching 85% across all three peaks, the model is considered to be matched well to the observed data as taking the calibration and validation counts combined, the percentage of links with a GEH is at 92% for the AM peak, 94% for the interpeak and 89% for the PM peak
- 11.3.8 Table 11.10, table 11.11 and table 11.12 show the GEH breakdown for AM peak, interpeak and PM peak respectively.

Table 11.10 - AM Peak GEH summary

GEH RANGE	CALIBRATION		VALIDATION		COMBINED	
GEH < 2	40	87%	19	44%	59	66%
GEH < 4	44	96%	34	79%	78	88%
GEH < 6	46	100%	37	86%	83	93%
GEH < 8	46	100%	41	95%	87	98%
GEH < 10	46	100%	41	95%	87	98%
GEH < 5	46	100%	36	84%	82	92%

Table 11.11 - Inter Peak GEH summary

GEH RANGE	CALIBRATION		VALIDATION		COMBINED	
GEH < 2	42	91%	25	58%	67	75%
GEH < 4	46	100%	34	79%	80	90%
GEH < 6	46	100%	39	91%	85	96%
GEH < 8	46	100%	40	93%	86	97%
GEH < 10	46	100%	40	93%	86	97%
GEH < 5	46	100%	38	88%	84	94%

Table 11.12 - PM Peak GEH summary

GEH RANGE	CALIBRATION		VALIDATION		COMBINED	
GEH < 2	36	78%	17	40%	53	60%
GEH < 4	44	96%	30	70%	74	83%
GEH < 6	46	100%	36	84%	82	92%
GEH < 8	46	100%	37	86%	83	93%
GEH < 10	46	100%	39	91%	85	96%
GEH < 5	46	100%	33	77%	79	89%

- 11.3.9 The GEH summaries show that all three peaks have at least 85% of the combined counts have a GEH value of less than 5. These results show that the model is well validated and calibrated.
- 11.3.10 Appendix E contains all link counts used for validation and calibration. Plots are also provided showing the GEH performance of counts near the bridge crossings.

JOURNEY TIME PERFORMANCE

- 11.3.11 Appendix F contains journey time graphs across all three time periods.
- 11.3.12 The journey time routes which can be seen in Figure 5.7 also indicate that the model reflects the observed data.
- 11.3.13 A summary of the modelled journey times compared to the observed is given for the AM peak in table 11.13.

Table 11.13 - AM peak journey time route comparison

ID	NAME	OBSERVED (s)	MODELLED (s)	DIFF	%	PASS?
1	1 - Northbound	341	367	-26	-8%	Yes
2	1 - Southbound	367	338	29	8%	Yes
3	2 - Northbound	521	583	-62	-12%	Yes
4	2 - Southbound	599	538	61	10%	Yes
5	3 - Eastbound	290	308	-17	-6%	Yes
6	3 - Westbound	289	287	2	1%	Yes
7	4 - Northbound	216	183	33	15%	Yes
8	4 - Southbound	222	187	36	16%	Yes
9	5 - Northbound	637	648	-11	-2%	Yes
10	5 - Southbound	553	537	16	3%	Yes
11	6 - Eastbound	689	608	81	12%	Yes
12	6 - Westbound	598	541	57	10%	Yes
13	7 - Eastbound	449	419	30	7%	Yes
14	7 - Westbound	391	344	46	12%	Yes
15	8 - Eastbound	723	698	25	3%	Yes
16	8 - Westbound	712	725	-13	-2%	Yes
17	9 - Northbound	462	354	107	23%	No
18	9 - Southbound	390	332	57	15%	Yes

- 11.3.14 The results show that seventeen of the eighteen journey times pass the criteria set out in Table 3.3. Journey time route 9 in the northbound direction is marginally outside the required criteria. Along the majority of this route the modelled travel time matches well to the observed travel time, the section between the A146 Beccles Road / Cotmer Road signals and western Mutford Bridge is shown to be modelled faster compared to the observed data leading to the journey time route falling outside the 15% band.
- 11.3.15 Table 11.14 shows that sixteen of the eighteen journey times pass the criteria in the inter peak. Both routes outside the criteria are shown in the journey time graphs to generally match well between the modelled travel time and observed travel time for the majority of the route.

Table 11.14 - Inter peak journey time route comparison

ID	NAME	OBSERVED (s)	MODELLED (s)	DIFF	%	PASS?
1	1 - Northbound	345	341	3	1%	Yes
2	1 - Southbound	373	331	42	11%	Yes
3	2 - Northbound	549	506	42	8%	Yes
4	2 - Southbound	669	526	143	21%	No
5	3 - Eastbound	305	340	-35	-11%	Yes
6	3 - Westbound	369	317	52	14%	Yes
7	4 - Northbound	216	185	31	14%	Yes
8	4 - Southbound	210	188	23	11%	Yes
9	5 - Northbound	846	821	25	3%	Yes
10	5 - Southbound	600	540	60	10%	Yes
11	6 - Eastbound	705	718	-13	-2%	Yes
12	6 - Westbound	646	539	107	17%	No
13	7 - Eastbound	396	419	-24	-6%	Yes
14	7 - Westbound	394	351	43	11%	Yes
15	8 - Eastbound	721	707	15	2%	Yes
16	8 - Westbound	745	738	7	1%	Yes
17	9 - Northbound	430	377	52	12%	Yes
18	9 - Southbound	346	329	17	5%	Yes

- 11.3.16 The results of the PM peak journey times can be seen in Table 11.15. Journey time route 1 southbound is shown to be marginally outside the required criteria.
- 11.3.17 Journey time route 3 westbound is outside the criteria, however the modelled journey time along this route is relatively consistent along this route across all three peaks, however there are notable differences in the observed journey time along this route. The section of the route between Peto Way and the western Mutford Bridge is shown to be modelled faster in the model compared to the observed data.
- 11.3.18 Journey time route 9 shows a notable increase the observed journey time in the PM peak, whereas the modelled journey time remains relatively consistent between the three peaks. The PM peak model does capture some of the delay along this route, with this route slowest in the PM peak model. The section of the route on the A12 Bloodmoor Road between the A12 Tower Road and A12 Tom Crisp Way shows a delay in the observed data which is not fully replicated in the model.

Table 11.15 - PM peak journey time route comparison

ID	NAME	OBSERVED (s)	MODELLED (s)	DIFF	%	PASS?
1	1 - Northbound	335	353	-18	-5%	Yes
2	1 - Southbound	449	354	95	21%	No
3	2 - Northbound	525	511	14	3%	Yes
4	2 - Southbound	592	537	55	9%	Yes
5	3 - Eastbound	287	294	-7	-3%	Yes
6	3 - Westbound	440	274	165	38%	No
7	4 - Northbound	202	188	14	7%	Yes
8	4 - Southbound	193	191	3	1%	Yes
9	5 - Northbound	910	772	138	15%	Yes
10	5 - Southbound	508	549	-40	-8%	Yes
11	6 - Eastbound	710	674	36	5%	Yes
12	6 - Westbound	597	618	-21	-4%	Yes
13	7 - Eastbound	392	421	-29	-7%	Yes
14	7 - Westbound	376	349	28	7%	Yes
15	8 - Eastbound	861	730	131	15%	Yes
16	8 - Westbound	825	824	1	0%	Yes
17	9 - Northbound	623	417	206	33%	No
18	9 - Southbound	312	351	-39	-13%	Yes

- 11.3.19 The journey times show that the link speeds and delays are accurately modelled on the key routes for all three time periods.

MANUAL CLASSIFIED COUNTS

- 11.3.20 Table 11.16 to table 11.18 show the performance of the manual classified counts across all three peaks. Overall, the model matches well to the observed turning movements in terms of GEH for individual turns.

Table 11.16 - AM peak manual classified count performance

ID	DESCRIPTION	GEH < 5	GEH < 7.5	GEH < 10
1	London Road/Arbor Lane/A12/Tower Road	76%	80%	88%
2	Tom Crisp Way/Stadbroke Road/Elm Tree Road	69%	94%	97%
3	Somerleyton Road/Oulton Street/Hall Lane/Gorleston Road	81%	88%	88%
4	Yarmouth Road/Gorleston Road	67%	78%	100%
5	Yarmouth Road/Leisure Way/Foxburrow Hill/Bentley Drive	88%	94%	94%
6	Yarmouth Road/Corton Road	56%	78%	89%
7	Millennium Way/Oulton Road/Peto Way	63%	81%	88%
8	Horn Hill/Maconochie Way/A12/Waveney Drive	88%	94%	100%
9	A12/Corton Long Lane/A12/Lowestoft Link Road	56%	75%	88%
10	A12 Waveney Road/Station Square	100%	100%	100%
11	Commercial Road/Station Square	78%	100%	100%
12	A12 Pier Terrace/London Road South/Belvedere Road	89%	89%	89%
13	A12 Belvedere Road/Kirkley Rise	36%	48%	52%
14	Denmark Road/Katwijk Way	67%	89%	89%
15	Katwijk Way/Raglan Street	56%	81%	81%
16	A12 Waveney Road/Suffolk Road	67%	78%	100%
17	A12 Tom Crisp Way/Blackheath Road	88%	100%	100%
18	Saltwater Way/Victoria Road	100%	100%	100%
19	Normanston Drive/Gorleston Road	67%	89%	100%
20	Fir Lane/A117 Normanston Drive/Peto Way	80%	100%	100%
21	A12/Gordon Road/Whapload Road	50%	69%	88%
22	A12/St Peters Street	56%	72%	92%

ID	DESCRIPTION	GEH < 5	GEH < 7.5	GEH < 10
23	A1144/Katwijk Way	22%	44%	78%
24	A146 Beccles Road/Cotmer Road	100%	100%	100%

11.3.21 As detailed in section 5, interpeak turning movements were not carried out at junctions 10 to 24.

Table 11.17 - Interpeak manual classified count performance

ID	DESCRIPTION	GEH < 5	GEH < 7.5	GEH < 10
1	London Road/Arbor Lane/A12/Tower Road	76%	84%	92%
2	Tom Crisp Way/Stadbroke Road/Elm Tree Road	61%	97%	97%
3	Somerleyton Road/Oulton Street/Hall Lane/Gorleston Road	88%	94%	100%
4	Yarmouth Road/Gorleston Road	89%	100%	100%
5	Yarmouth Road/Leisure Way/Foxburrow Hill/Bentley Drive	81%	100%	100%
6	Yarmouth Road/Corton Road	67%	89%	100%
7	Millennium Way/Oulton Road/Peto Way	63%	81%	100%
8	Horn Hill/Maconochie Way/A12/Waveney Drive	81%	94%	100%
9	A12/Corton Long Lane/A12/Lowestoft Link Road	63%	69%	100%

Table 11.18 - PM peak manual classified count performance

ID	DESCRIPTION	GEH < 5	GEH < 7.5	GEH < 10
1	London Road/Arbor Lane/A12/Tower Road	76%	88%	92%
2	Tom Crisp Way/Stadbroke Road/Elm Tree Road	64%	78%	97%
3	Somerleyton Road/Oulton Street/Hall Lane/Gorleston Road	69%	75%	88%
4	Yarmouth Road/Gorleston Road	78%	78%	89%
5	Yarmouth Road/Leisure Way/Foxburrow Hill/Bentley Drive	75%	94%	100%
6	Yarmouth Road/Corton Road	56%	78%	89%
7	Millennium Way/Oulton Road/Peto Way	31%	56%	88%
8	Horn Hill/Maconochie Way/A12/Waveney Drive	81%	81%	94%
9	A12/Corton Long Lane/A12/Lowestoft Link Road	63%	69%	81%
10	A12 Waveney Road/Station Square	67%	89%	100%
11	Commercial Road/Station Square	67%	89%	100%
12	A12 Pier Terrace/London Road South/Belvedere Road	78%	89%	89%
13	A12 Belvedere Road/Kirkley Rise	40%	48%	56%
14	Denmark Road/Katwijk Way	78%	78%	89%
15	Katwijk Way/Raglan Street	69%	81%	100%
16	A12 Waveney Road/Suffolk Road	78%	78%	100%
17	A12 Tom Crisp Way/Blackheath Road	69%	94%	100%
18	Saltwater Way/Victoria Road	33%	78%	89%
19	Normanston Drive/Gorleston Road	56%	78%	89%
20	Fir Lane/A117 Normanston Drive/Peto Way	60%	80%	92%
21	A12/Gordon Road/Whapload Road	50%	69%	88%
22	A12/St Peters Street	64%	88%	88%
23	A1144/Katwijk Way	56%	67%	89%
24	A146 Beccles Road/Cotmer Road	78%	78%	89%

11.4 MODELLED FLOW AND JUNCTION DELAY

- 11.4.1 Appendix G contains plots of the link flow and junction performance in the vicinity of the river crossings

12 SUMMARY OF MODEL DEVELOPMENT, STANDARDS ACHIEVED AND FITNESS FOR PURPOSE

12.1 SUMMARY OF MODEL DEVELOPMENT

- 12.1.1 The previous 2001 Lowestoft Traffic Model was rebuilt with a comprehensive check of the model network and a revised zone system of sufficient detail based on 2011 census geography.
- 12.1.2 An observed prior matrix was derived from ANPR data which formed a cordon around the main study area and major internal locations. A gravity model was then used to form a synthetic matrix based on NTEM version 6.2 trip ends and 2011 census data. The synthetic matrix was used to infill the prior matrix for traffic movements not represented in the ANPR matrix.
- 12.1.3 Adjustments were made to the prior matrix using scaling of calibration counts to match the modelled flow to the observed flow. Matrix estimation was then carried out to produce a final assignment.

12.2 SUMMARY OF STANDARDS ACHIEVED

- 12.2.1 The base year model validation has been developed closely following TAG M3.1 'Highway Assignment Modelling' guidance (January 2014).
- 12.2.2 The model is shown to satisfactorily converge across all three peaks which is important as the model will be taken forward for testing of a major scheme business case. In these instances it is required that models are converged so that the benefits of the scheme are the result of the infrastructure improvements and not changes to model convergence.
- 12.2.3 Screenlines within the model which capture the key strategic movements within the model have been shown to closely match the observed flows to the modelled flows. Across all three peaks, all calibration screenlines are shown to achieve a GEH below 4, with the majority of validation screenlines also achieving this standard.
- 12.2.4 Link validation is shown to be consistently high in terms of both flow and GEH across all three peaks. Combining the observed counts within calibration and validation, 92% of counts in the AM peak, 94% of counts in the interpeak and 89% of counts in the PM peak achieve a GEH of 5 or lower above the minimum threshold of 85%.
- 12.2.5 Journey time performance reaches the required standard of 85% of modelled journey time routes being within 15% of the observed journey time data in the AM peak and interpeak. In the AM peak, 94% of journey time routes achieve this standard, whilst 89% achieve this in the inter-peak. In the PM peak, journey time performance is marginally below the required standard at 83%, however this is not deemed a significant concern.
- 12.2.6 Manual classified turning counts were carried out at major junctions across the network, with the model shown to align well with the observed movements at these junctions.

12.3 ASSESSMENT OF FITNESS FOR PURPOSE

- 12.3.1 The latest 2015 Lowestoft Transport Model is deemed fit for purpose in terms of its ability to replicate existing strategic traffic movements within the Area of Detailed Modelling (ADM). The base year model forms a suitable basis from which forecast year models can be built to create reference case, do minimum and do something scheme testing.
- 12.3.2 The model provides a suitable evidence base to underpin a major scheme business case and determine the benefits of a third crossing in Lowestoft.

Appendix A

MCC TOTALS USED TO FACTOR ANPR DATA

Table A-1 – AM peak MCC totals used for doubly constrained furness of ANPR matrix

	SITE DESCRIPTION	DIR	CAR		LGV		HGV	
			ORIGIN	DEST	ORIGIN	DEST	ORIGIN	DEST
1	A12 London Road	Inbound	608	0	117	0	30	0
		Outbound	0	430	0	150	0	44
2	A146 Beccles Road	Inbound	587	0	85	0	45	0
		Outbound	0	625	0	125	0	56
3	A12 Yarmouth Road	Inbound	746	0	142	0	44	0
		Outbound	0	1178	0	135	0	40
4	Coast Road	Inbound	61	0	7	0	2	0
		Outbound	0	54	0	4	0	3
5	A12 Pier Terrace (Eastern Bascule bridge)	NB	326	998	44	120	18	25
		SB	369	322	113	56	28	10
6	A146 Bridge Road (Western Mutford bridge)	NB	545	235	90	49	17	8
		SB	442	274	105	47	18	18
7	B1375 Gorleston Road	NB	334	234	45	31	3	6
		SB	276	81	65	7	10	1
8	A1117 Millennium Way	NB	218	110	27	13	8	8
		SB	234	189	29	16	13	2
9	A12 Yarmouth Road	NB	302	204	43	24	23	7
		SB	347	234	77	15	21	0
10	A12 Tom Crisp Way	NB	525	192	64	23	12	6
		SB	199	100	59	19	17	11
11	B1532 London Road South	NB	233	205	29	27	9	6
		SB	199	110	47	18	10	4
12	B1074 Blundeston Road	Inbound	164	0	40	0	4	0
		Outbound	0	99	0	15	0	1
13	Flixton Road	Inbound	78	0	11	0	2	0
		Outbound	0	49	0	9	0	3
14	B1531 Waveney Drive	EB	258	85	28	7	9	0
		WB	40	96	9	11	3	2
15	North Quay Retail Park	Entry	36	107	6	9	0	0
		Exit	0	71	0	0	0	0
16	Links Road Car Park	Entry	3	0	0	0	0	0
		Exit	1	3	0	0	0	0

	SITE DESCRIPTION	DIR	CAR		LGV		HGV	
			ORIGIN	DEST	ORIGIN	DEST	ORIGIN	DEST
17	Swimming Pool Road Car Park	Entry	0	1	0	0	0	0
		Exit	0	0	0	0	0	0
18	Shopping Centre Car Park (Battery Green Rd exit)	Entry	0	0	0	0	0	0
		Exit	0	0	1	0	0	0
19	Shopping Centre Car Park (Gordon Road entry)	Entry	0	32	0	0	0	0
		Exit	0	0	0	0	0	0
20	Surrey St Car Park entry	Entry	8	14	0	0	0	0
		Exit	0	0	0	0	0	0
21	Surrey St Car Park exit (onto Clapham Road)	Entry	0	0	0	0	0	0
		Exit	0	3	0	0	0	0
22	Clapham Road Car Park	Entry	3	24	0	0	0	0
		Exit	3	6	0	0	0	0
23	St Johns Rd Car Park	Entry	0	0	0	0	0	0
		Exit	0	2	0	0	0	0
24	Kirkley Rise Car Park (Northern access)	Entry	4	21	0	0	0	0
		Exit	0	9	0	0	0	0
25	Kirkley Rise Car Park (Southern access)	Entry	2	3	0	0	0	0
		Exit	0	0	0	0	0	0
26	Kirkley Cliff Road Car Park	Entry	0	0	0	0	0	0
		Exit	0	0	0	0	0	0
27	Claremont Road Car Park	Entry	0	0	0	0	0	0
		Exit	0	0	0	0	0	0
28	Marine Parade Car Park	Entry	0	5	0	0	0	0
		Exit	0	0	0	0	0	0
29	Asda Car Park	Entry	155	60	12	2	0	0
		Exit	23	132	13	0	0	0

Table A-2 – Inter peak MCC totals used for doubly constrained furness of ANPR matrix

	SITE DESCRIPTION	DIR	CAR		LGV		HGV	
			ORIGIN	DEST	ORIGIN	DEST	ORIGIN	DEST
1	A12 London Road	Inbound	511	0	90	0	33	0
		Outbound	0	526	0	85	0	38
2	A146 Beccles Road	Inbound	457	0	93	0	38	0
		Outbound	0	467	0	86	0	39
3	A12 Yarmouth Road	Inbound	704	0	119	0	37	0
		Outbound	0	711	0	118	0	35
4	Coast Road	Inbound	52	0	5	0	2	0
		Outbound	0	58	0	7	0	3
5	A12 Pier Terrace (Eastern Bascule bridge)	NB	323	547	40	87	12	32
		SB	624	252	86	39	37	14
6	A146 Bridge Road (Western Mutford bridge)	NB	428	390	60	70	22	13
		SB	391	389	61	59	13	18
7	B1375 Gorleston Road	NB	163	207	29	33	1	15
		SB	292	106	52	10	13	1
8	A1117 Millennium Way	NB	220	161	30	15	6	8
		SB	217	120	35	11	11	3
9	A12 Yarmouth Road	NB	292	183	45	26	15	8
		SB	344	116	52	16	25	3
10	A12 Tom Crisp Way	NB	297	137	41	31	13	5
		SB	179	254	31	33	11	15
11	B1532 London Road South	NB	143	138	25	26	13	5
		SB	180	149	31	15	10	8
12	B1074 Blundeston Road	Inbound	90	0	20	0	3	0
		Outbound	0	80	0	18	0	3
13	Flixton Road	Inbound	57	0	9	0	1	0
		Outbound	0	41	0	7	0	1
14	B1531 Waveney Drive	EB	169	44	22	5	4	4
		WB	53	163	8	21	4	5
15	North Quay Retail Park	Entry	147	230	9	19	1	0
		Exit	0	373	0	0	0	0
16	Links Road Car Park	Entry	7	3	0	0	0	0
		Exit	4	5	0	1	0	0

	SITE DESCRIPTION	DIR	CAR		LGV		HGV	
			ORIGIN	DEST	ORIGIN	DEST	ORIGIN	DEST
17	Swimming Pool Road Car Park	Entry	0	7	0	0	0	0
		Exit	5	0	0	0	0	0
18	Shopping Centre Car Park (Battery Green Rd exit)	Entry	0	0	0	0	0	0
		Exit	48	0	1	0	0	0
19	Shopping Centre Car Park (Gordon Road entry)	Entry	6	39	0	1	0	0
		Exit	0	0	0	0	0	0
20	Surrey St Car Park entry	Entry	34	26	1	0	0	0
		Exit	0	0	0	0	0	0
21	Surrey St Car Park exit (onto Clapham Road)	Entry	0	0	0	0	0	0
		Exit	33	33	1	1	0	0
22	Clapham Road Car Park	Entry	25	78	0	0	0	0
		Exit	82	24	0	3	0	0
23	St Johns Rd Car Park	Entry	0	4	2	0	0	0
		Exit	2	1	0	1	0	0
24	Kirkley Rise Car Park (Northern access)	Entry	6	4	0	0	0	0
		Exit	11	0	0	0	0	0
25	Kirkley Rise Car Park (Southern access)	Entry	2	1	0	0	0	0
		Exit	0	0	0	0	0	0
26	Kirkley Cliff Road Car Park	Entry	0	0	0	0	0	0
		Exit	0	0	0	0	0	0
27	Claremont Road Car Park	Entry	1	11	0	0	0	0
		Exit	6	5	2	0	0	0
28	Marine Parade Car Park	Entry	4	34	0	3	0	0
		Exit	29	7	0	0	0	1
29	Asda Car Park	Entry	265	72	15	1	1	1
		Exit	81	259	3	13	1	0

Table A-3 – PM peak MCC totals used for doubly constrained furness of ANPR matrix

	SITE DESCRIPTION	DIR	CAR		LGV		HGV	
			ORIGIN	DEST	ORIGIN	DEST	ORIGIN	DEST
1	A12 London Road	Inbound	611	0	104	0	14	0
		Outbound	0	668	0	103	0	15
2	A146 Beccles Road	Inbound	871	0	136	0	19	0
		Outbound	0	673	0	53	0	17
3	A12 Yarmouth Road	Inbound	1388	0	173	0	18	0
		Outbound	0	909	0	91	0	19
4	Coast Road	Inbound	84	0	7	0	3	0
		Outbound	0	52	0	5	0	3
5	A12 Pier Terrace (Eastern Bascule bridge)	NB	358	608	32	78	5	23
		SB	1086	333	107	40	15	10
6	A146 Bridge Road (Western Mutford bridge)	NB	419	538	52	88	6	11
		SB	331	672	40	79	4	7
7	B1375 Gorleston Road	NB	115	277	17	49	1	3
		SB	455	188	65	35	6	1
8	A1117 Millennium Way	NB	247	221	12	25	2	5
		SB	227	247	34	30	4	1
9	A12 Yarmouth Road	NB	429	265	44	26	7	8
		SB	397	167	46	11	17	3
10	A12 Tom Crisp Way	NB	232	217	31	40	5	4
		SB	297	471	19	44	3	6
11	B1532 London Road South	NB	187	239	14	25	7	4
		SB	222	229	19	23	8	3
12	B1074 Blundeston Road	Inbound	167	0	32	0	1	0
		Outbound	0	111	0	15	0	0
13	Flixton Road	Inbound	59	0	13	0	1	0
		Outbound	0	61	0	6	0	0
14	B1531 Waveney Drive	EB	164	50	16	6	4	2
		WB	117	290	6	33	1	5
15	North Quay Retail Park	Entry	99	175	7	17	0	0
		Exit	0	309	0	0	0	0
16	Links Road Car Park	Entry	5	4	0	0	0	0
		Exit	0	8	0	0	0	0

	SITE DESCRIPTION	DIR	CAR		LGV		HGV	
			ORIGIN	DEST	ORIGIN	DEST	ORIGIN	DEST
17	Swimming Pool Road Car Park	Entry	0	5	0	0	0	0
		Exit	15	0	0	0	0	0
18	Shopping Centre Car Park (Battery Green Rd exit)	Entry	0	0	0	0	0	0
		Exit	25	0	1	0	0	0
19	Shopping Centre Car Park (Gordon Road entry)	Entry	0	1	0	0	0	0
		Exit	0	0	0	0	0	0
20	Surrey St Car Park entry	Entry	3	0	0	0	0	0
		Exit	0	0	0	0	0	0
21	Surrey St Car Park exit (onto Clapham Road)	Entry	0	0	0	0	0	0
		Exit	11	4	0	0	0	0
22	Clapham Road Car Park	Entry	7	15	0	0	0	0
		Exit	35	12	0	3	0	0
23	St Johns Rd Car Park	Entry	8	0	0	0	0	0
		Exit	3	1	0	0	0	0
24	Kirkley Rise Car Park (Northern access)	Entry	0	2	0	0	0	0
		Exit	10	0	0	0	0	0
25	Kirkley Rise Car Park (Southern access)	Entry	0	0	0	0	0	0
		Exit	1	1	0	0	0	0
26	Kirkley Cliff Road Car Park	Entry	0	0	0	0	0	0
		Exit	1	0	0	0	0	0
27	Claremont Road Car Park	Entry	5	10	0	2	0	0
		Exit	9	1	0	0	0	0
28	Marine Parade Car Park	Entry	1	16	0	0	0	0
		Exit	22	1	4	0	0	0
29	Asda Car Park	Entry	184	104	11	8	0	0
		Exit	61	254	6	15	0	0

Appendix B

GRAVITY MODEL TLD AND MATRIX CHANGES

AM peak gravity model

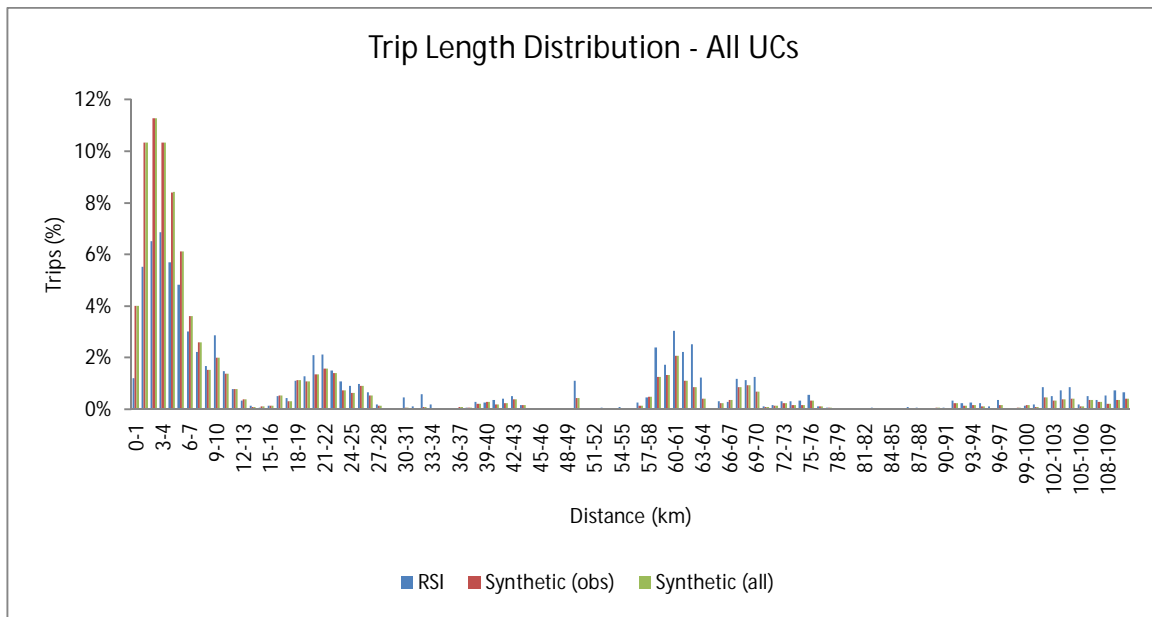


Figure B.1 – AM peak - Overall trip length distribution (UC1 to UC3)

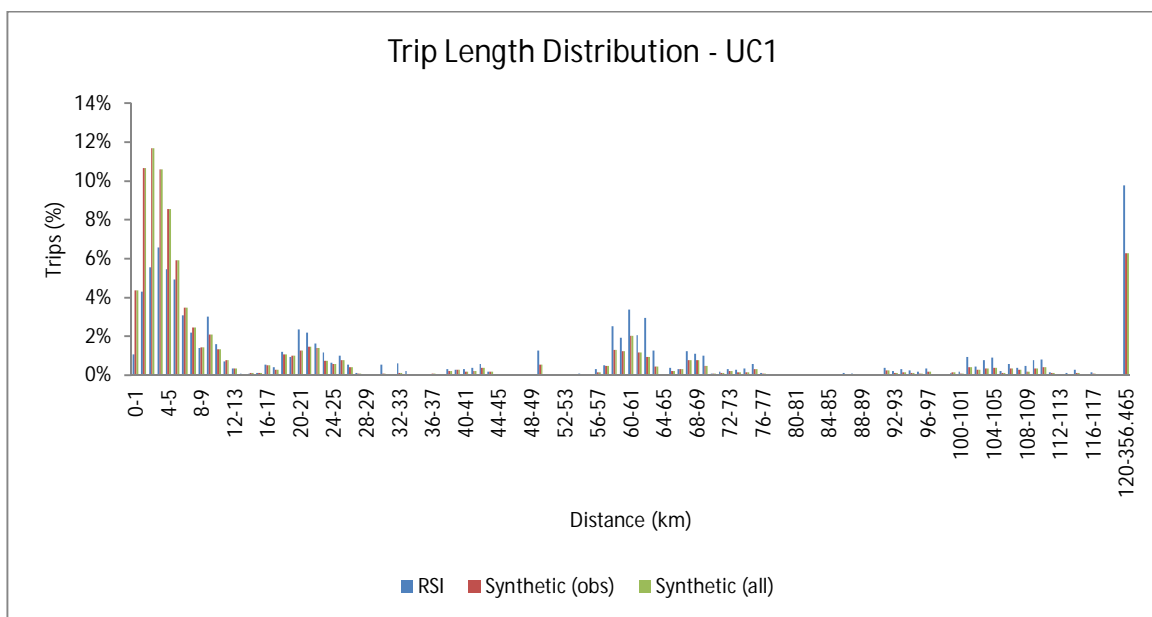


Figure B.2 – AM peak – UC1 trip length distribution

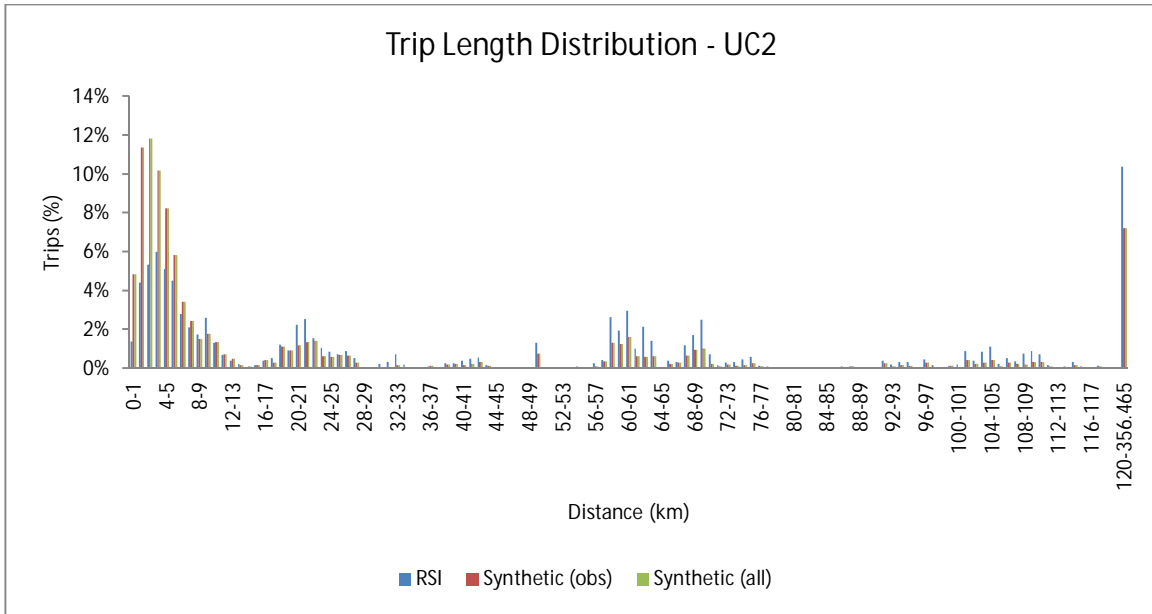


Figure B.3 – AM peak – UC2 trip length distribution

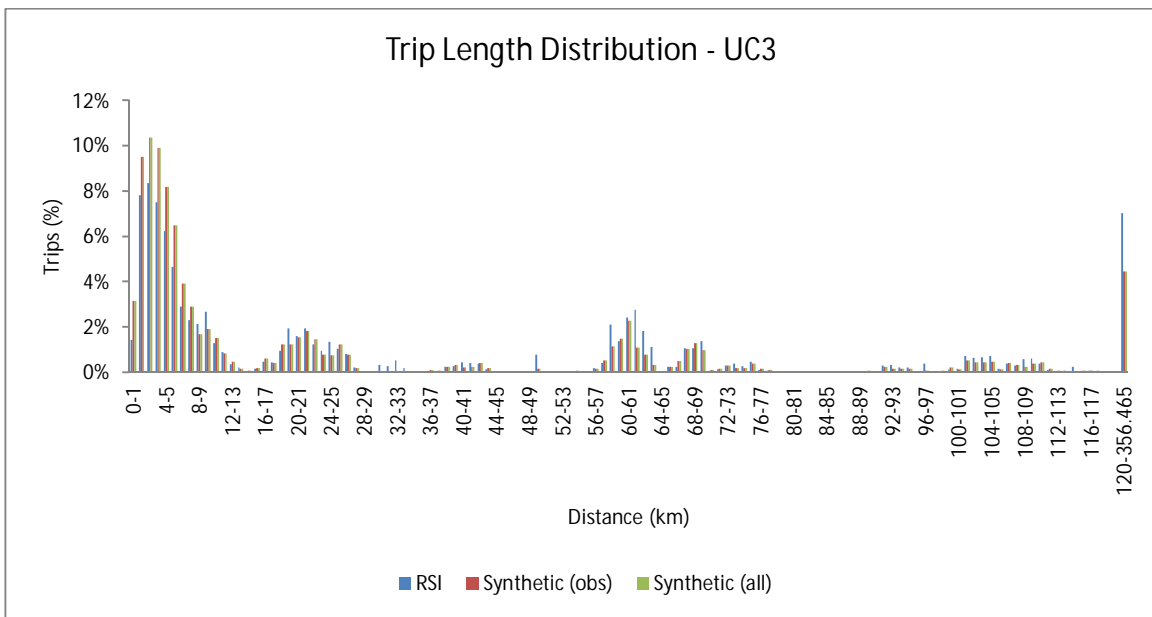


Figure B.4 – AM peak – UC3 trip length distribution

SATURN Gravity Model - Sectors - UC1

NB - Intra-zonals are removed

RSI Matrix - Total Trips

	1	2	3	4	5	Total
1	0	0	0	31	16	47
2	0	0	0	326	145	471
3	0	0	0	301	153	454
4	42	188	260	361	239	1090
5	35	172	236	622	400	1465
Total	77	360	496	1641	953	3527

RSI Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	0%	1%
2	0%	0%	0%	9%	4%	13%
3	0%	0%	0%	9%	4%	13%
4	1%	5%	7%	10%	7%	31%
5	1%	5%	7%	18%	11%	42%
Total	2%	10%	14%	47%	27%	100%

Synthetic (Observed) Matrix - Total Trips

	1	2	3	4	5	Total
1	2	13	16	17	10	58
2	28	69	101	243	154	595
3	27	77	43	273	148	568
4	32	202	317	1483	530	2564
5	36	226	307	957	1040	2566
Total	125	587	784	2973	1882	6351

Synthetic (Observed) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	1%	2%	4%	2%	9%
3	0%	1%	1%	4%	2%	9%
4	1%	3%	5%	23%	8%	40%
5	1%	4%	5%	15%	16%	40%
Total	2%	9%	12%	47%	30%	100%

Synthetic (Full) Matrix - Total Trips

	1	2	3	4	5	Total
1	2	13	16	17	10	58
2	28	69	101	244	154	596
3	27	77	43	273	148	568
4	32	202	317	1484	530	2565
5	36	226	307	958	1040	2567
Total	125	587	784	2976	1882	6354

Synthetic (Full) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	1%	2%	4%	2%	9%
3	0%	1%	1%	4%	2%	9%
4	1%	3%	5%	23%	8%	40%
5	1%	4%	5%	15%	16%	40%
Total	2%	9%	12%	47%	30%	100%

SATURN Gravity Model - Sectors - UC2

NB - Intra-zonals are removed

RSI Matrix - Total Trips

	1	2	3	4	5	Total
1	0	0	0	4	2	6
2	0	0	0	40	18	58
3	0	0	0	36	19	55
4	6	27	36	44	35	148
5	4	19	26	64	41	154
Total	10	46	62	188	115	421

RSI Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	0%	1%
2	0%	0%	0%	10%	4%	14%
3	0%	0%	0%	9%	5%	13%
4	1%	6%	9%	10%	8%	35%
5	1%	5%	6%	15%	10%	37%
Total	2%	11%	15%	45%	27%	100%

Synthetic (Observed) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	2	2	2	1	7
2	4	11	16	26	17	74
3	4	12	7	29	17	69
4	3	22	36	184	66	311
5	3	23	32	102	116	276
Total	14	70	93	343	217	737

Synthetic (Observed) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	1%	1%	2%	4%	2%	10%
3	1%	2%	1%	4%	2%	9%
4	0%	3%	5%	25%	9%	42%
5	0%	3%	4%	14%	16%	37%
Total	2%	9%	13%	47%	29%	100%

Synthetic (Full) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	2	2	2	1	7
2	4	11	16	26	17	74
3	4	12	7	29	17	69
4	3	22	36	185	66	312
5	3	23	32	102	116	276
Total	14	70	93	344	217	738

Synthetic (Full) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	1%	1%	2%	4%	2%	10%
3	1%	2%	1%	4%	2%	9%
4	0%	3%	5%	25%	9%	42%
5	0%	3%	4%	14%	16%	37%
Total	2%	9%	13%	47%	29%	100%

SATURN Gravity Model - Sectors - UC3

NB - Intra-zonals are removed

RSI Matrix - Total Trips

	1	2	3	4	5	Total
1	0	0	0	12	12	24
2	0	0	0	134	130	264
3	0	0	0	132	147	279
4	17	87	121	177	297	699
5	14	75	114	313	352	868
Total	31	162	235	768	938	2134

RSI Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	1%	1%
2	0%	0%	0%	6%	6%	12%
3	0%	0%	0%	6%	7%	13%
4	1%	4%	6%	8%	14%	33%
5	1%	4%	5%	15%	16%	41%
Total	1%	8%	11%	36%	44%	100%

Synthetic (Observed) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	2	2	15	12	31
2	4	9	14	171	144	342
3	4	11	7	200	150	372
4	20	112	179	642	381	1334
5	18	102	145	412	552	1229
Total	46	236	347	1440	1239	3308

Synthetic (Observed) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	0%	0%	5%	4%	10%
3	0%	0%	0%	6%	5%	11%
4	1%	3%	5%	19%	12%	40%
5	1%	3%	4%	12%	17%	37%
Total	1%	7%	10%	44%	37%	100%

Synthetic (Full) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	2	2	15	12	31
2	4	9	14	171	144	342
3	4	11	7	200	150	372
4	20	112	179	642	382	1335
5	18	102	145	412	552	1229
Total	46	236	347	1440	1240	3309

Synthetic (Full) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	0%	0%	5%	4%	10%
3	0%	0%	0%	6%	5%	11%
4	1%	3%	5%	19%	12%	40%
5	1%	3%	4%	12%	17%	37%
Total	1%	7%	10%	44%	37%	100%

Inter peak gravity model

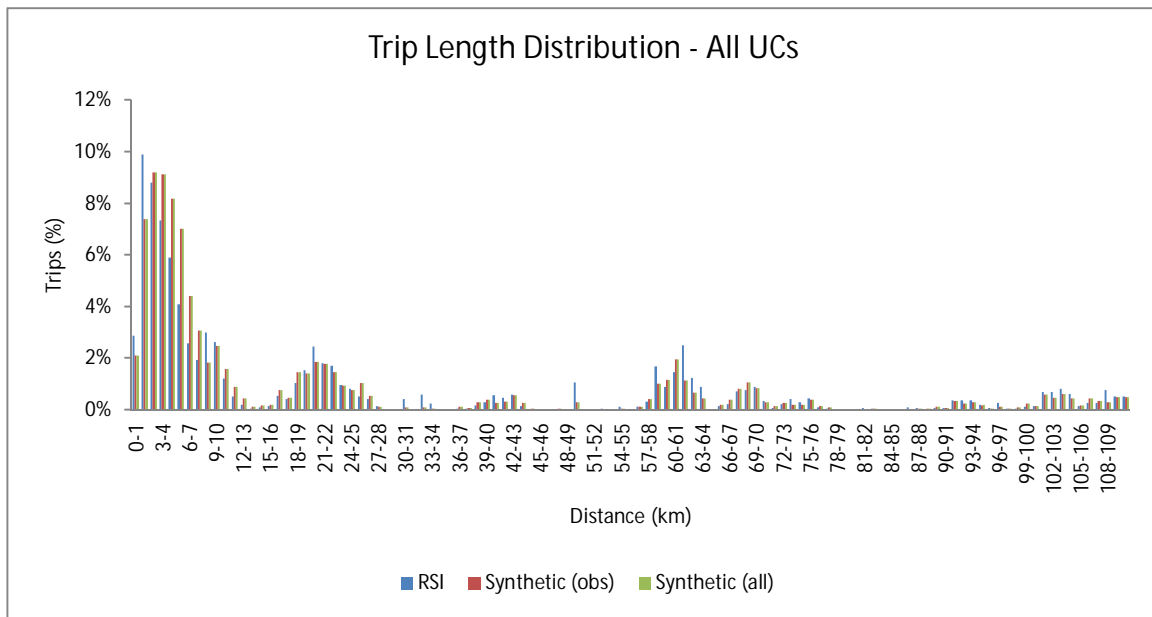


Figure B.5 – Interpeak - Overall trip length distribution (UC1 to UC3)

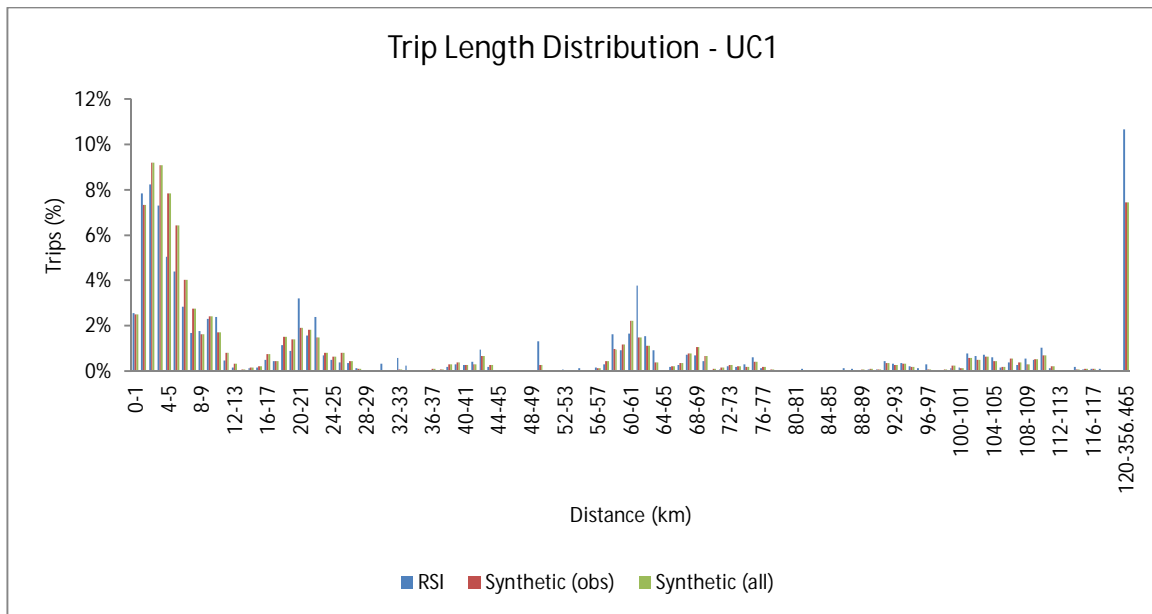


Figure B.6 – Interpeak – UC1 trip length distribution

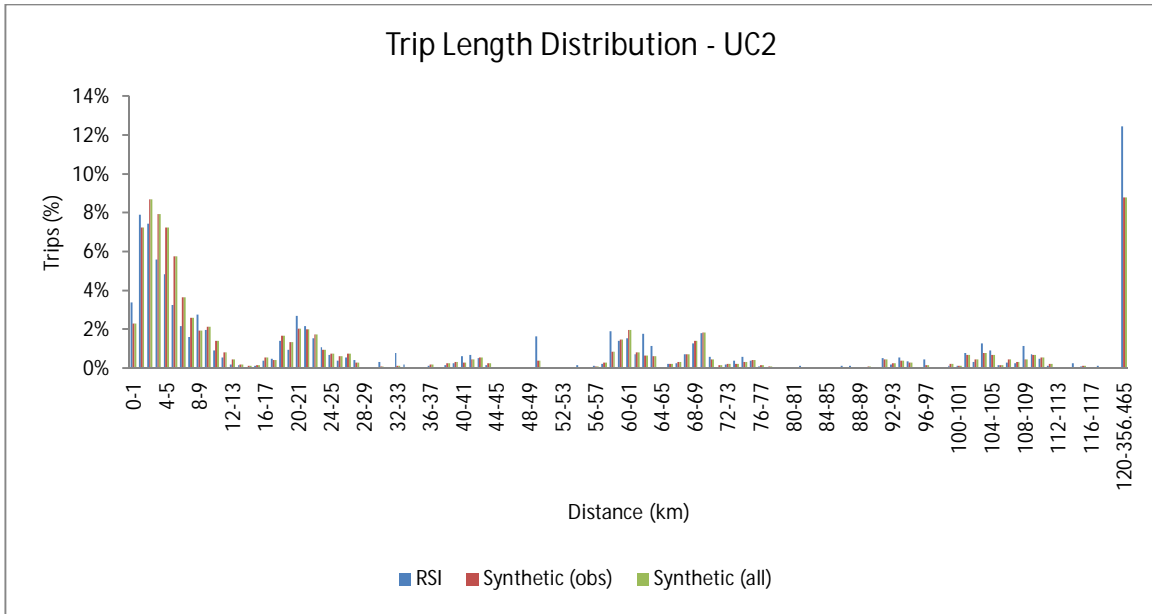


Figure B.7 – Interpeak – UC2 trip length distribution

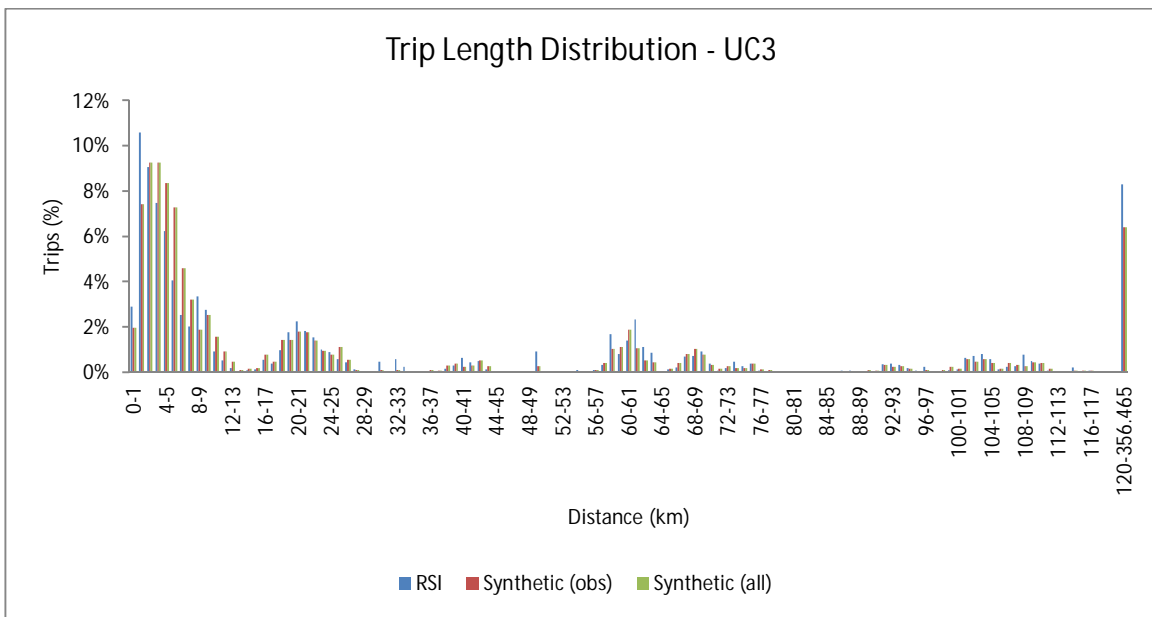


Figure B.8 – Interpeak – UC3 trip length distribution

SATURN Gravity Model - Sectors - UC1

NB - Intra-zonals are removed

RSI Matrix - Total Trips

	1	2	3	4	5	Total
1	0	0	0	18	7	25
2	0	0	0	117	39	156
3	0	0	0	126	37	163
4	12	68	50	228	79	437
5	12	59	38	224	126	459
Total	24	127	88	713	288	1240

RSI Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	1%	2%
2	0%	0%	0%	9%	3%	13%
3	0%	0%	0%	10%	3%	13%
4	1%	5%	4%	18%	6%	35%
5	1%	5%	3%	18%	10%	37%
Total	2%	10%	7%	58%	23%	100%

Synthetic (Observed) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	3	2	17	10	32
2	3	10	9	108	67	197
3	3	12	5	116	68	204
4	18	99	76	327	172	692
5	18	100	72	281	235	706
Total	42	224	164	849	552	1831

Synthetic (Observed) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	1%	2%
2	0%	1%	0%	6%	4%	11%
3	0%	1%	0%	6%	4%	11%
4	1%	5%	4%	18%	9%	38%
5	1%	5%	4%	15%	13%	39%
Total	2%	12%	9%	46%	30%	100%

Synthetic (Full) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	3	2	17	10	32
2	3	10	9	108	67	197
3	3	12	5	116	68	204
4	18	99	76	328	172	693
5	18	100	72	281	235	706
Total	42	224	164	850	552	1832

Synthetic (Full) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	1%	2%
2	0%	1%	0%	6%	4%	11%
3	0%	1%	0%	6%	4%	11%
4	1%	5%	4%	18%	9%	38%
5	1%	5%	4%	15%	13%	39%
Total	2%	12%	9%	46%	30%	100%

SATURN Gravity Model - Sectors - UC2

NB - Intra-zonals are removed

RSI Matrix - Total Trips

	1	2	3	4	5	Total
1	0	0	0	5	2	7
2	0	0	0	29	15	44
3	0	0	0	29	15	44
4	8	43	29	76	51	207
5	4	22	16	44	39	125
Total	12	65	45	183	122	427

RSI Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	0%	2%
2	0%	0%	0%	7%	4%	10%
3	0%	0%	0%	7%	4%	10%
4	2%	10%	7%	18%	12%	48%
5	1%	5%	4%	10%	9%	29%
Total	3%	15%	11%	43%	29%	100%

Synthetic (Observed) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	1	1	6	3	11
2	1	5	5	36	23	70
3	1	6	3	39	23	72
4	8	47	36	117	70	278
5	6	36	27	81	65	215
Total	16	95	72	279	184	646

Synthetic (Observed) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	0%	2%
2	0%	1%	1%	6%	4%	11%
3	0%	1%	0%	6%	4%	11%
4	1%	7%	6%	18%	11%	43%
5	1%	6%	4%	13%	10%	33%
Total	2%	15%	11%	43%	28%	100%

Synthetic (Full) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	1	1	6	3	11
2	1	5	5	36	23	70
3	1	6	3	39	23	72
4	8	47	36	117	70	278
5	6	36	27	81	65	215
Total	16	95	72	279	184	646

Synthetic (Full) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	0%	2%
2	0%	1%	1%	6%	4%	11%
3	0%	1%	0%	6%	4%	11%
4	1%	7%	6%	18%	11%	43%
5	1%	6%	4%	13%	10%	33%
Total	2%	15%	11%	43%	28%	100%

SATURN Gravity Model - Sectors - UC3

NB - Intra-zonals are removed

RSI Matrix - Total Trips

	1	2	3	4	5	Total
1	0	0	0	34	34	68
2	0	0	0	234	237	471
3	0	0	0	269	241	510
4	49	293	241	702	782	2067
5	39	219	142	494	817	1711
Total	88	512	383	1733	2111	4827

RSI Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	1%	1%
2	0%	0%	0%	5%	5%	10%
3	0%	0%	0%	6%	5%	11%
4	1%	6%	5%	15%	16%	43%
5	1%	5%	3%	10%	17%	35%
Total	2%	11%	8%	36%	44%	100%

Synthetic (Observed) Matrix - Total Trips

	1	2	3	4	5	Total
1	1	8	7	45	38	99
2	10	35	34	312	268	659
3	10	47	21	367	297	742
4	54	326	273	1063	860	2576
5	49	297	236	864	967	2413
Total	124	713	571	2651	2430	6489

Synthetic (Observed) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	1%	2%
2	0%	1%	1%	5%	4%	10%
3	0%	1%	0%	6%	5%	11%
4	1%	5%	4%	16%	13%	40%
5	1%	5%	4%	13%	15%	37%
Total	2%	11%	9%	41%	37%	100%

Synthetic (Full) Matrix - Total Trips

	1	2	3	4	5	Total
1	1	8	7	45	38	99
2	10	35	34	312	268	659
3	10	47	21	367	297	742
4	54	326	273	1064	860	2577
5	49	297	236	864	967	2413
Total	124	713	571	2652	2430	6490

Synthetic (Full) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	1%	2%
2	0%	1%	1%	5%	4%	10%
3	0%	1%	0%	6%	5%	11%
4	1%	5%	4%	16%	13%	40%
5	1%	5%	4%	13%	15%	37%
Total	2%	11%	9%	41%	37%	100%

PM peak gravity model

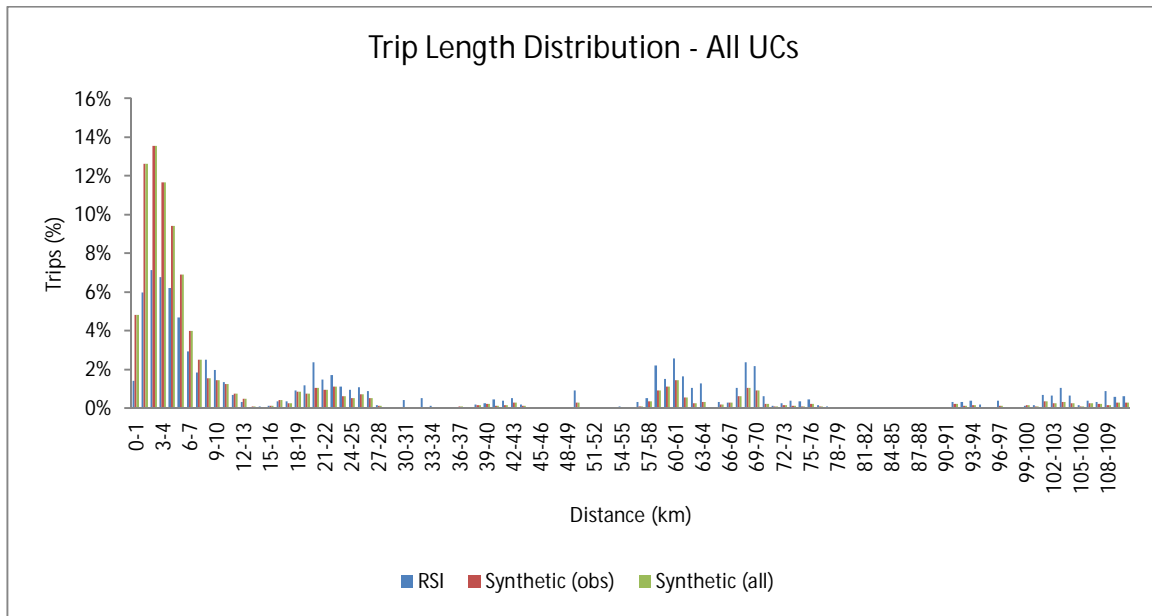


Figure B.9 – PM peak - Overall trip length distribution (UC1 to UC3)

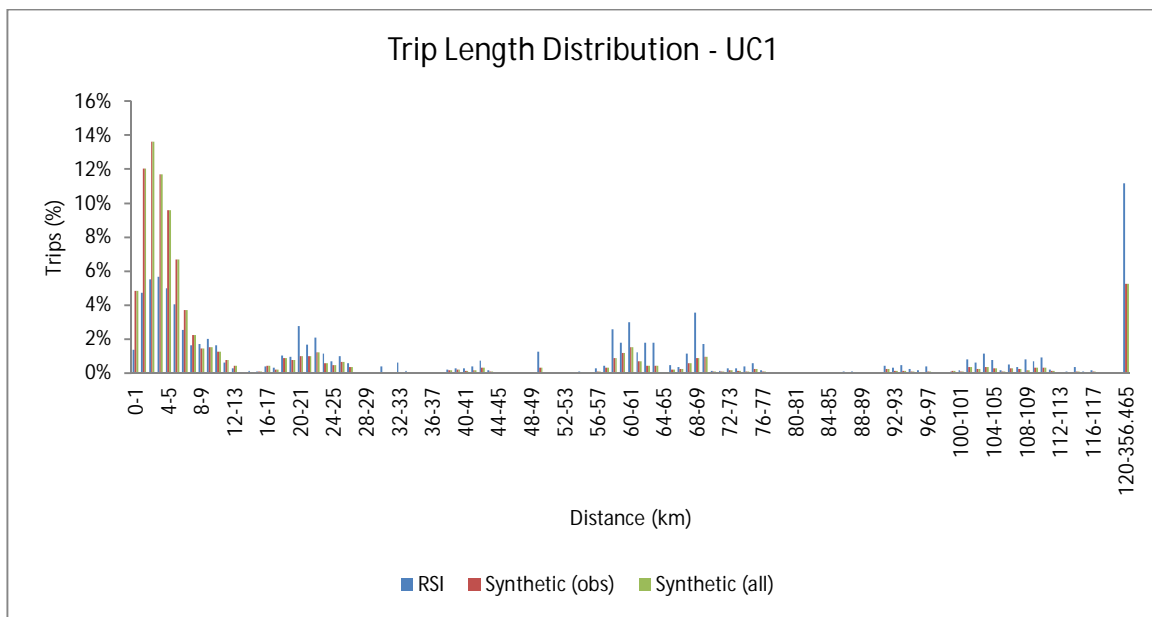


Figure B.10 – PM peak – UC1 trip length distribution

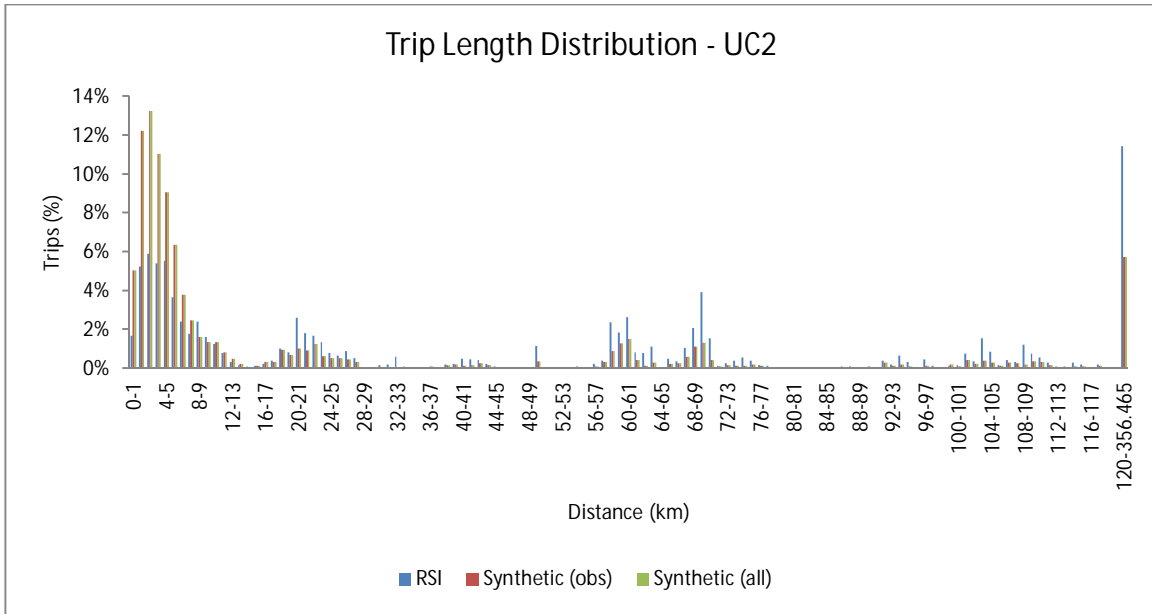


Figure B.11 – PM peak – UC2 trip length distribution

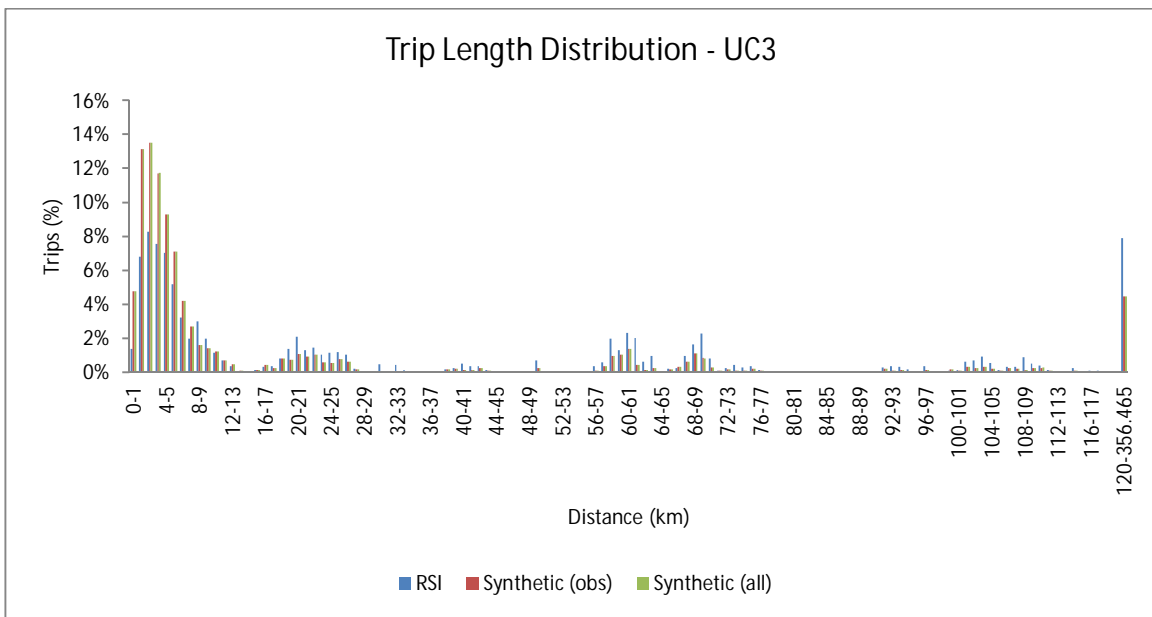


Figure B.12 – PM peak – UC3 trip length distribution

SATURN Gravity Model - Sectors - UC1

NB - Intra-zonals are removed

RSI Matrix - Total Trips

	1	2	3	4	5	Total
1	0	0	0	38	14	52
2	0	0	0	255	96	351
3	0	0	0	341	176	517
4	34	196	180	320	282	1012
5	27	135	133	352	240	887
Total	61	331	313	1306	808	2819

RSI Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	0%	2%
2	0%	0%	0%	9%	3%	12%
3	0%	0%	0%	12%	6%	18%
4	1%	7%	6%	11%	10%	36%
5	1%	5%	5%	12%	9%	31%
Total	2%	12%	11%	46%	29%	100%

Synthetic (Observed) Matrix - Total Trips

	1	2	3	4	5	Total
1	2	14	12	21	13	62
2	16	47	49	192	121	425
3	22	78	32	311	171	614
4	29	201	219	1786	617	2852
5	30	217	207	1050	1183	2687
Total	99	557	519	3360	2105	6640

Synthetic (Observed) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	1%	1%	3%	2%	6%
3	0%	1%	0%	5%	3%	9%
4	0%	3%	3%	27%	9%	43%
5	0%	3%	3%	16%	18%	40%
Total	1%	8%	8%	51%	32%	100%

Synthetic (Full) Matrix - Total Trips

	1	2	3	4	5	Total
1	2	14	12	21	13	62
2	16	47	49	192	121	425
3	22	78	32	311	171	614
4	29	201	219	1787	618	2854
5	30	217	207	1051	1183	2688
Total	99	557	519	3362	2106	6643

Synthetic (Full) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	1%	1%	3%	2%	6%
3	0%	1%	0%	5%	3%	9%
4	0%	3%	3%	27%	9%	43%
5	0%	3%	3%	16%	18%	40%
Total	1%	8%	8%	51%	32%	100%

SATURN Gravity Model - Sectors - UC2

NB - Intra-zonals are removed

RSI Matrix - Total Trips

	1	2	3	4	5	Total
1	0	0	0	5	2	7
2	0	0	0	32	14	46
3	0	0	0	43	25	68
4	8	46	41	59	64	218
5	4	20	21	41	35	121
Total	12	66	62	180	140	460

RSI Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	0%	2%
2	0%	0%	0%	7%	3%	10%
3	0%	0%	0%	9%	5%	15%
4	2%	10%	9%	13%	14%	47%
5	1%	4%	5%	9%	8%	26%
Total	3%	14%	13%	39%	30%	100%

Synthetic (Observed) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	2	2	3	2	9
2	3	8	8	26	17	62
3	4	13	5	43	24	89
4	5	37	40	279	101	462
5	5	37	35	148	172	397
Total	17	97	90	499	316	1019

Synthetic (Observed) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	1%	1%	3%	2%	6%
3	0%	1%	0%	4%	2%	9%
4	0%	4%	4%	27%	10%	45%
5	0%	4%	3%	15%	17%	39%
Total	2%	10%	9%	49%	31%	100%

Synthetic (Full) Matrix - Total Trips

	1	2	3	4	5	Total
1	0	2	2	3	2	9
2	3	8	8	26	17	62
3	4	13	5	43	24	89
4	5	37	40	280	101	463
5	5	37	35	148	172	397
Total	17	97	90	500	316	1020

Synthetic (Full) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	1%	1%	3%	2%	6%
3	0%	1%	0%	4%	2%	9%
4	0%	4%	4%	27%	10%	45%
5	0%	4%	3%	15%	17%	39%
Total	2%	10%	9%	49%	31%	100%

SATURN Gravity Model - Sectors - UC3

NB - Intra-zonals are removed

RSI Matrix - Total Trips

	1	2	3	4	5	Total
1	0	0	0	31	30	61
2	0	0	0	228	223	451
3	0	0	0	320	394	714
4	42	279	282	497	987	2087
5	34	189	189	406	635	1453
Total	76	468	471	1482	2269	4766

RSI Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	1%	1%	1%
2	0%	0%	0%	5%	5%	9%
3	0%	0%	0%	7%	8%	15%
4	1%	6%	6%	10%	21%	44%
5	1%	4%	4%	9%	13%	30%
Total	2%	10%	10%	31%	48%	100%

Synthetic (Observed) Matrix - Total Trips

	1	2	3	4	5	Total
1	2	15	14	24	22	77
2	17	55	60	238	222	592
3	24	92	42	411	325	894
4	30	235	276	2312	1164	4017
5	27	224	229	1165	1933	3578
Total	100	621	621	4150	3666	9158

Synthetic (Observed) Matrix - % of Total

	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	1%	1%	3%	2%	6%
3	0%	1%	0%	4%	4%	10%
4	0%	3%	3%	25%	13%	44%
5	0%	2%	3%	13%	21%	39%
Total	1%	7%	7%	45%	40%	100%

Synthetic (Full) Matrix - Total Trips

	1	2	3	4	5	Total
1	2	15	14	24	22	77
2	17	55	60	238	222	592
3	24	92	42	411	325	894
4	30	235	276	2314	1164	4019
5	27	224	229	1166	1933	3579
Total	100	621	621	4153	3666	9161

Synthetic (Full) Matrix - % of Total

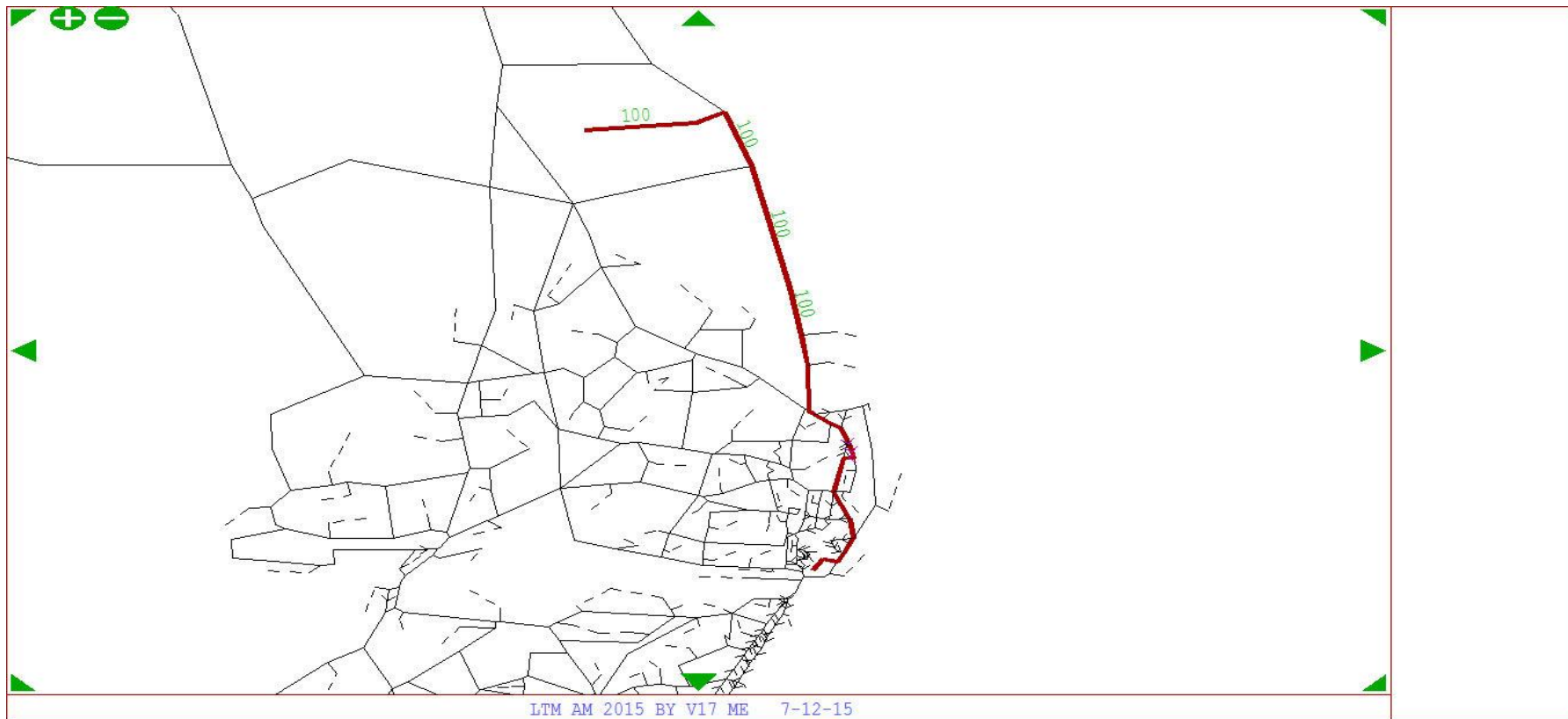
	1	2	3	4	5	Total
1	0%	0%	0%	0%	0%	1%
2	0%	1%	1%	3%	2%	6%
3	0%	1%	0%	4%	4%	10%
4	0%	3%	3%	25%	13%	44%
5	0%	2%	2%	13%	21%	39%
Total	1%	7%	7%	45%	40%	100%

Appendix C

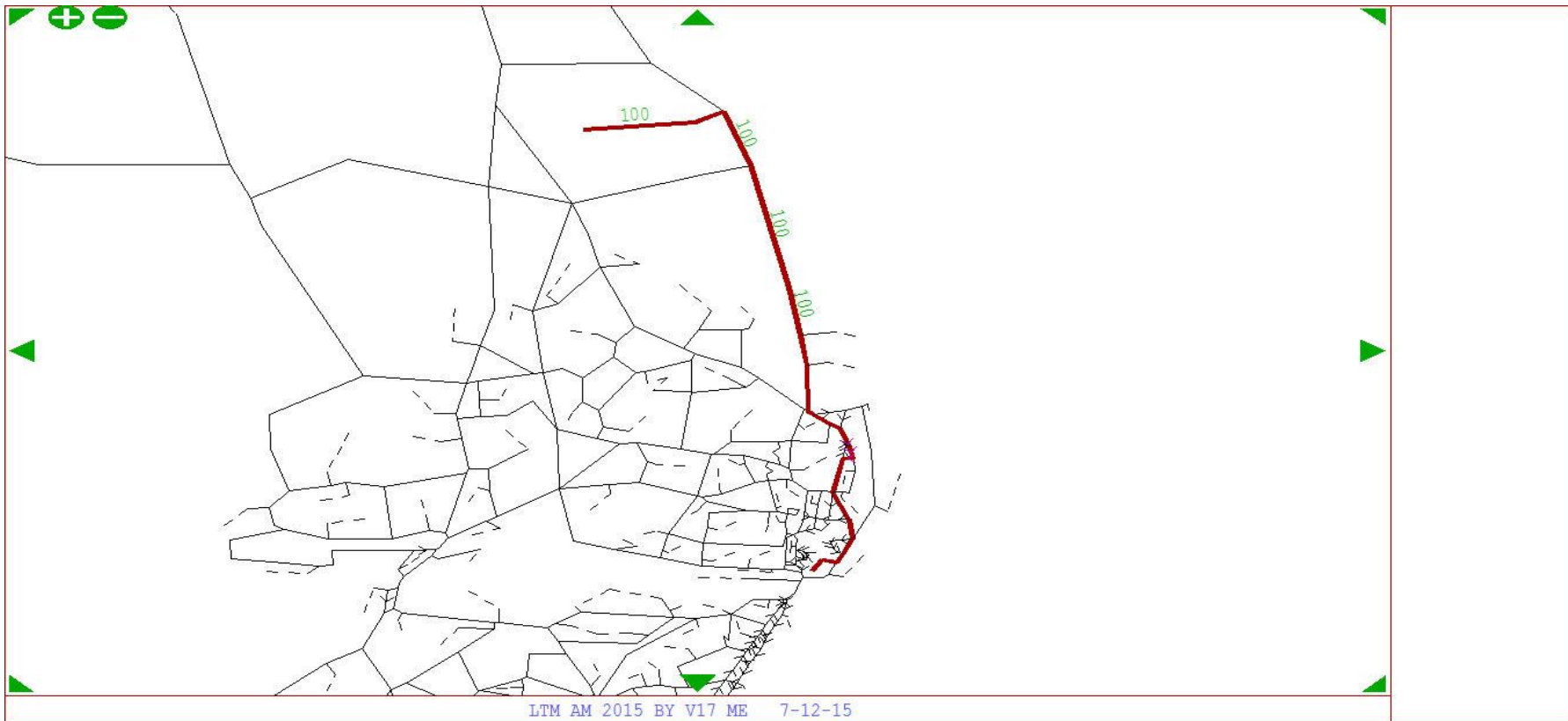
ORIGIN-DESTINATION TREES

Lowestoft: OD Tree Plots - AM Peak

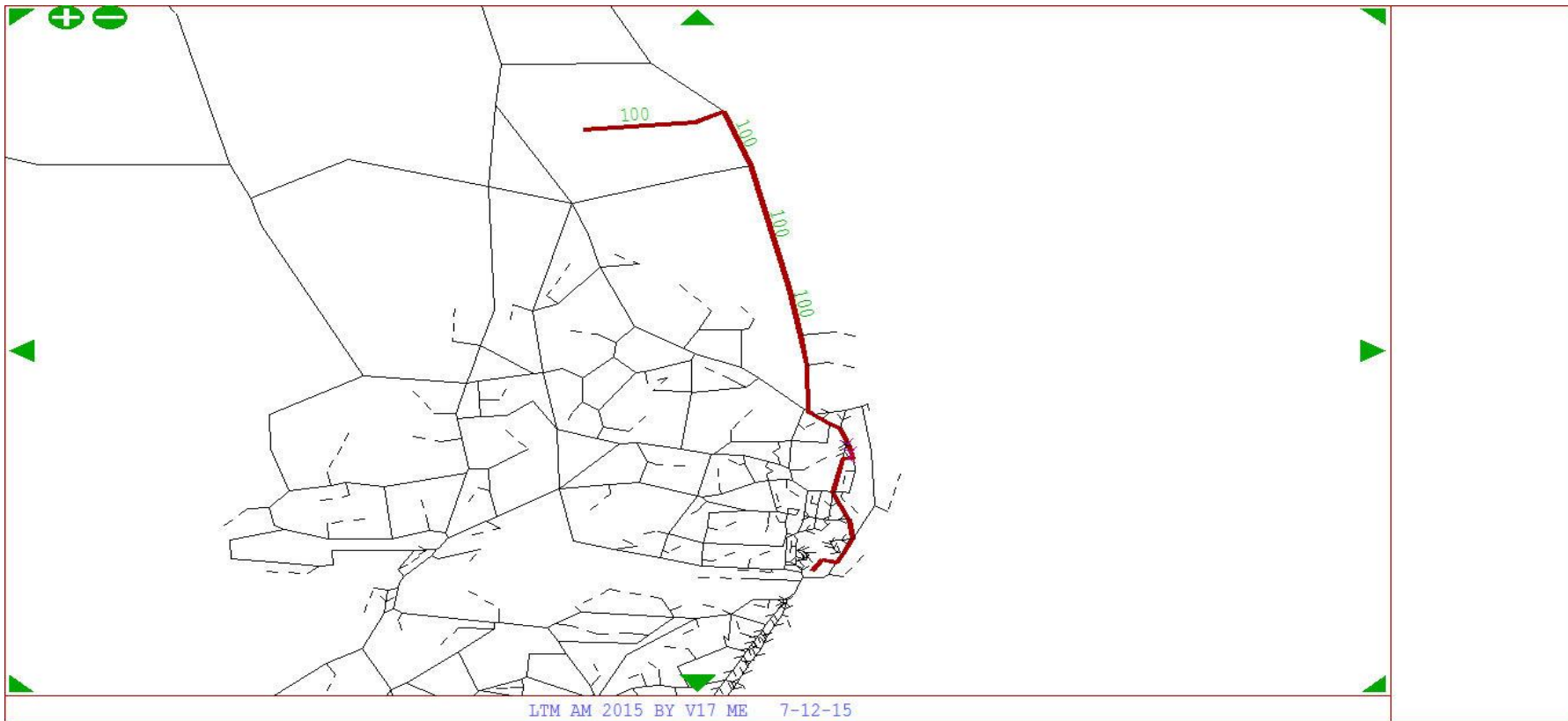
From Zone 102 To Zone 119 - User Class 1



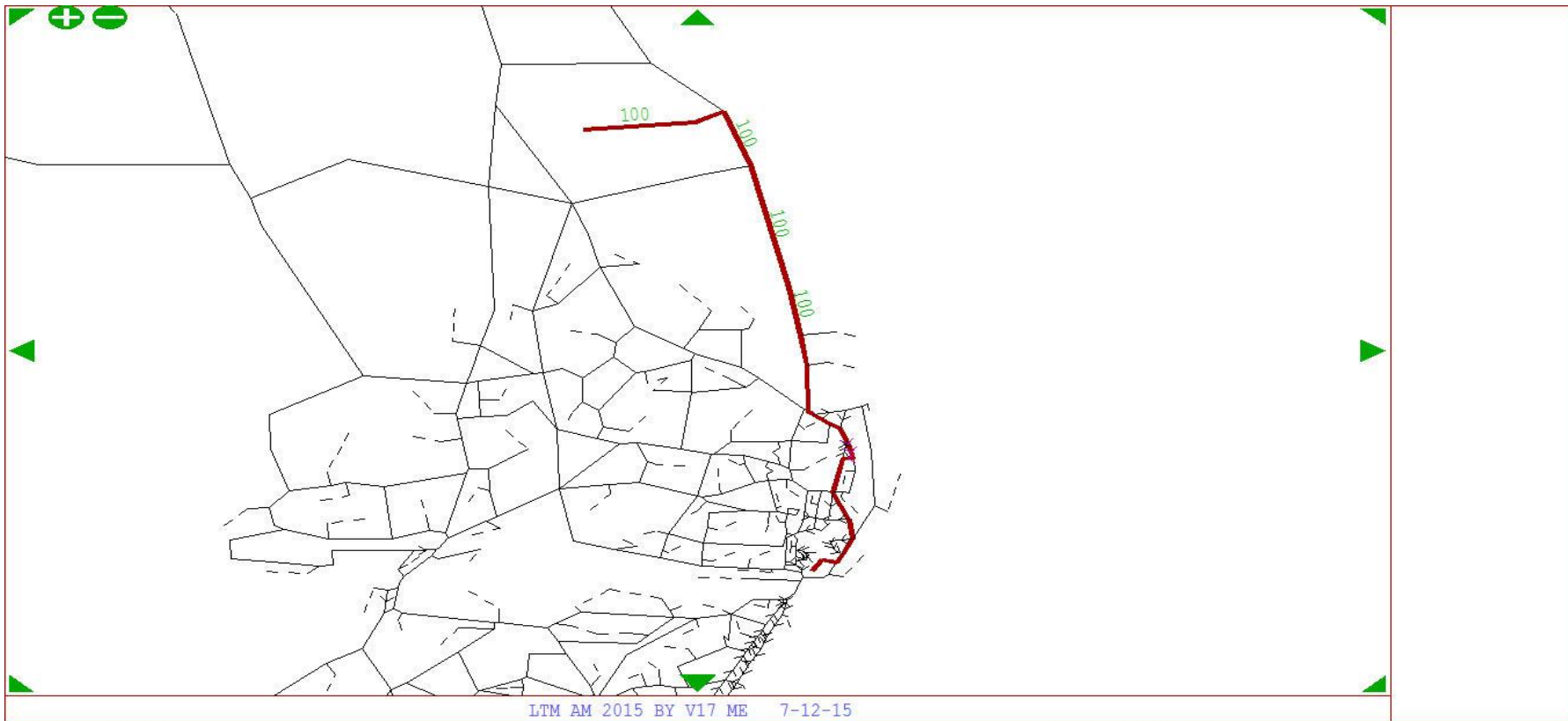
From Zone 102 To Zone 119 - User Class 2



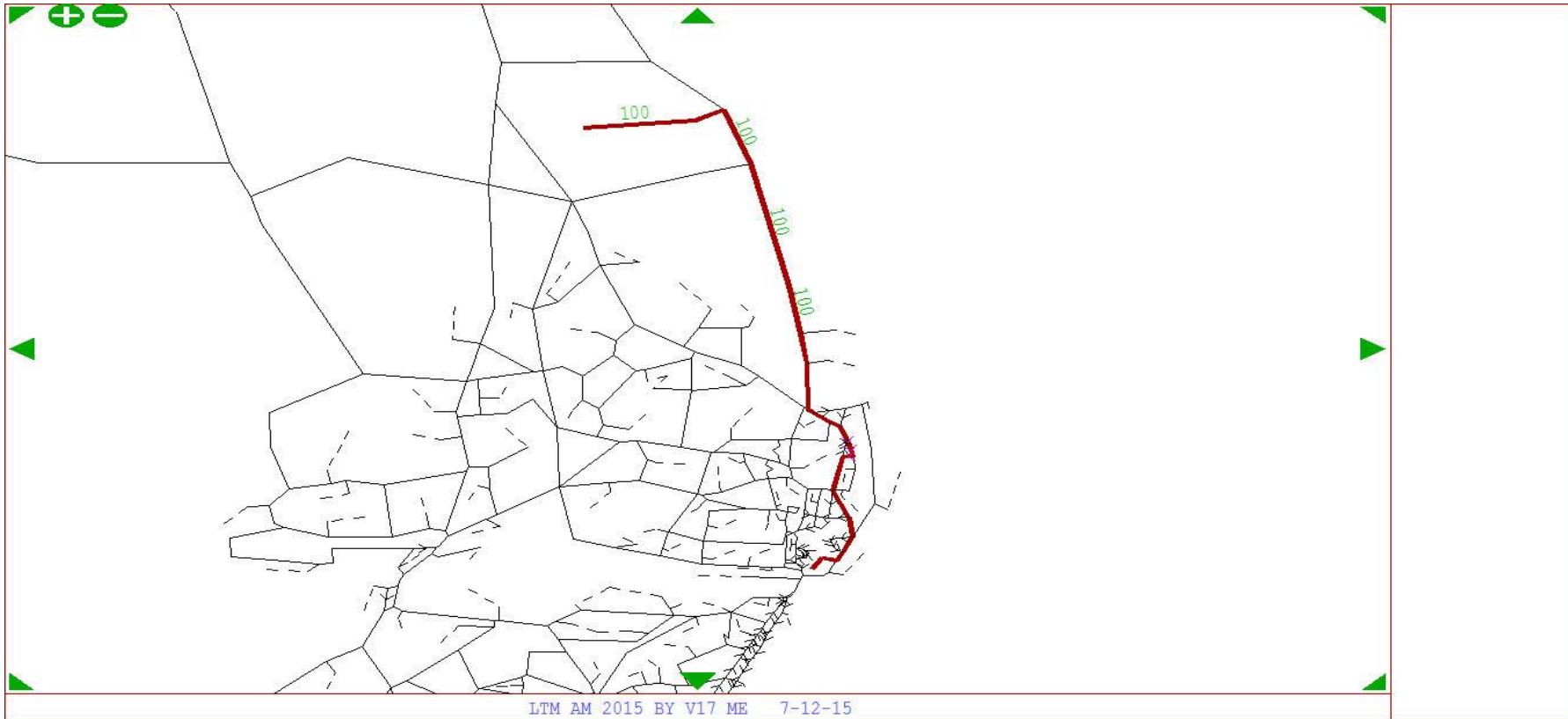
From Zone 102 To Zone 119 - User Class 3



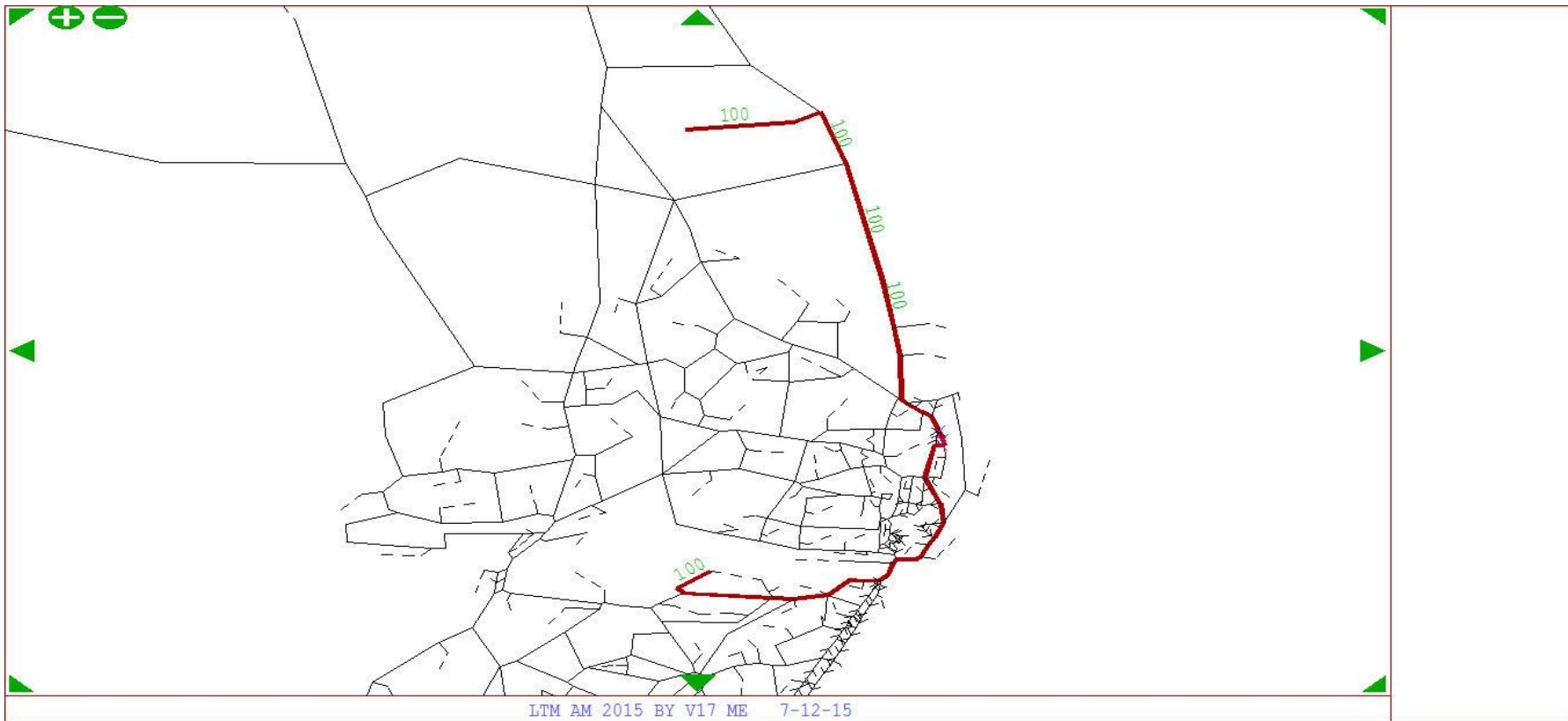
From Zone 102 To Zone 119 - User Class 4



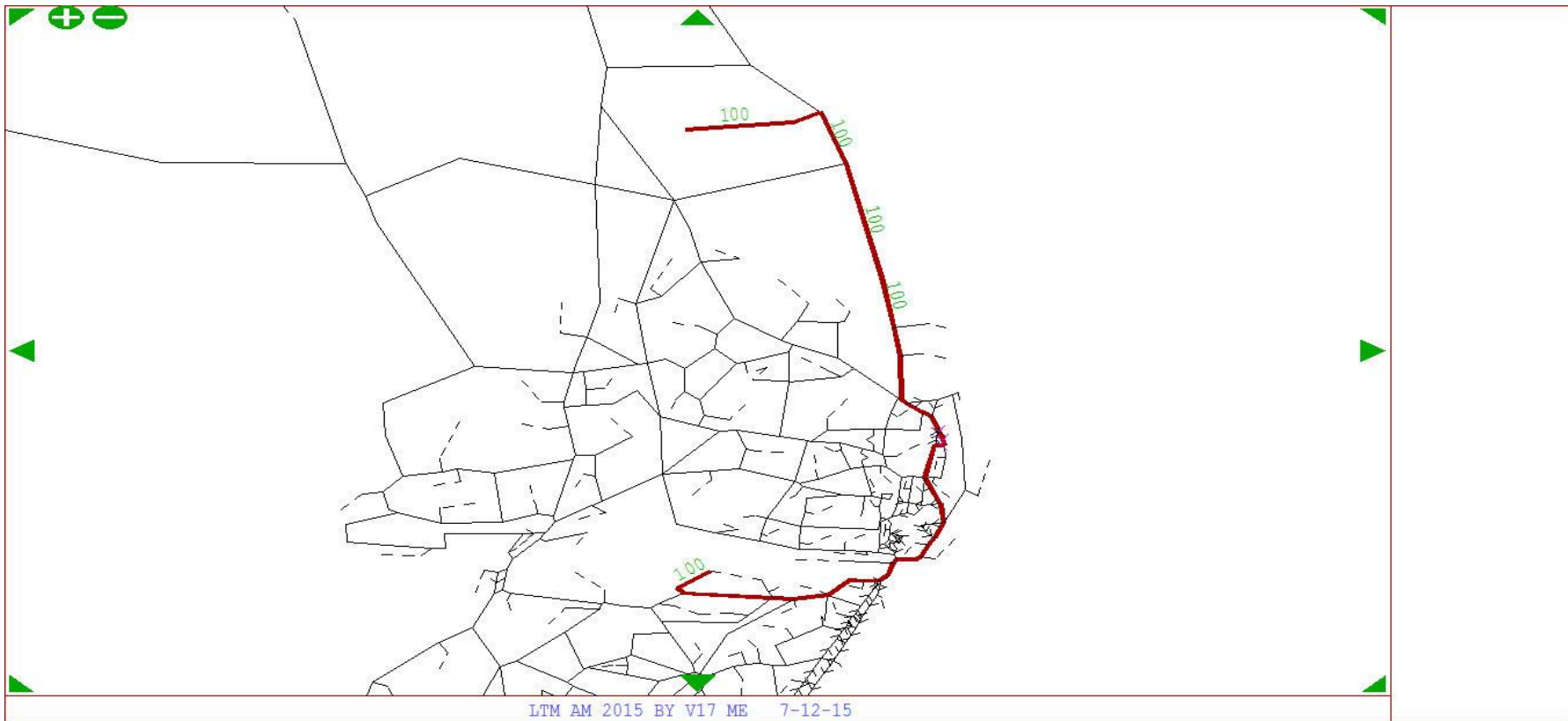
From Zone 102 To Zone 119 - User Class 5



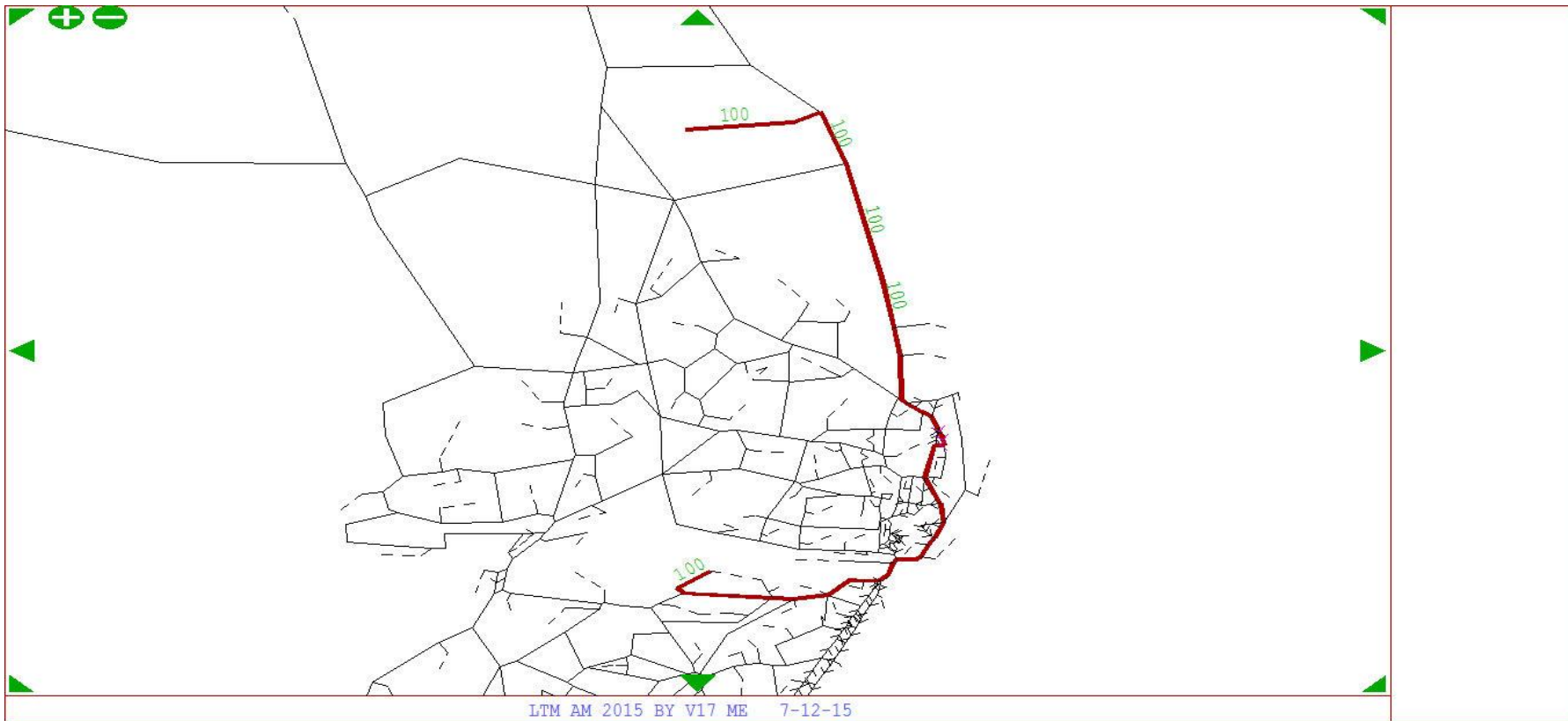
From Zone 102 To Zone 122 - User Class 1



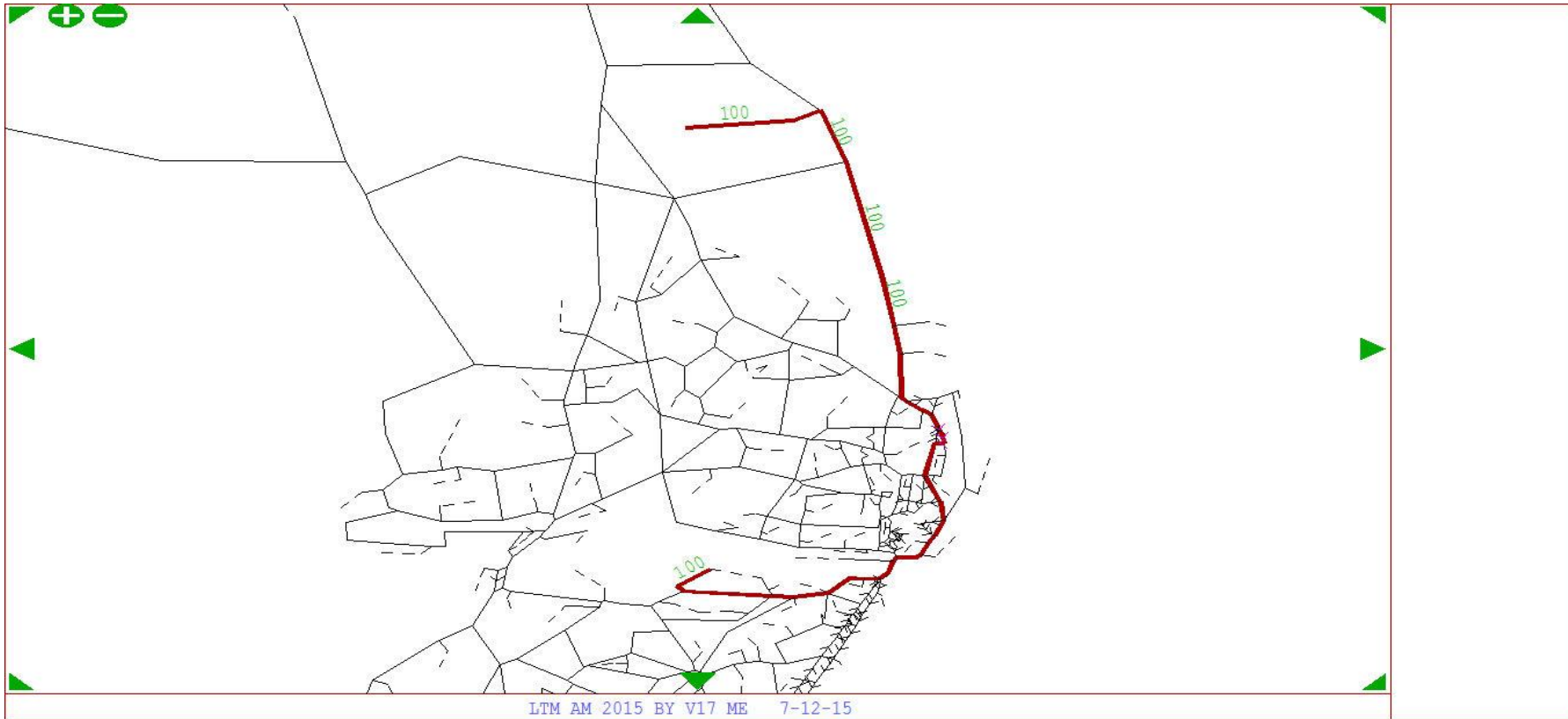
From Zone 102 To Zone 122 - User Class 2



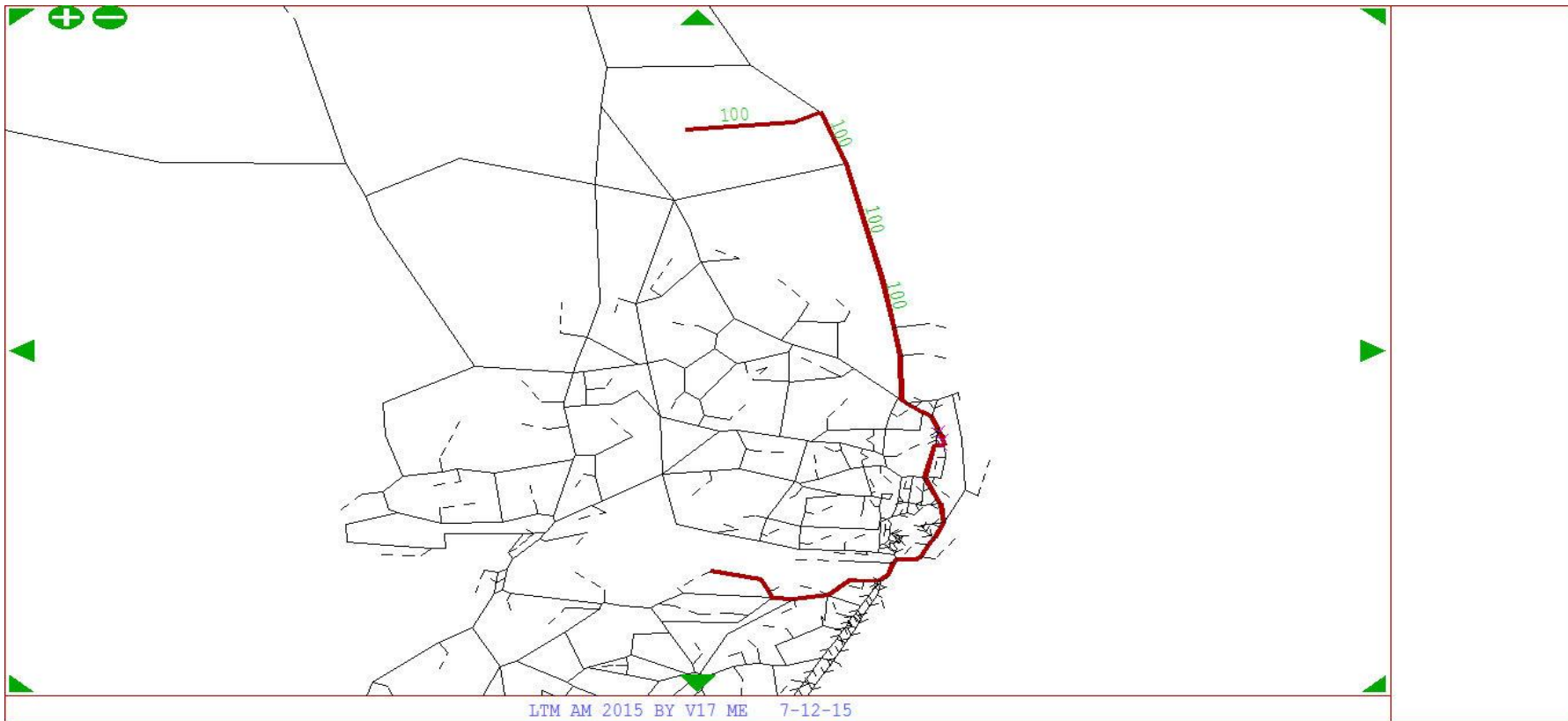
From Zone 102 To Zone 122 - User Class 3



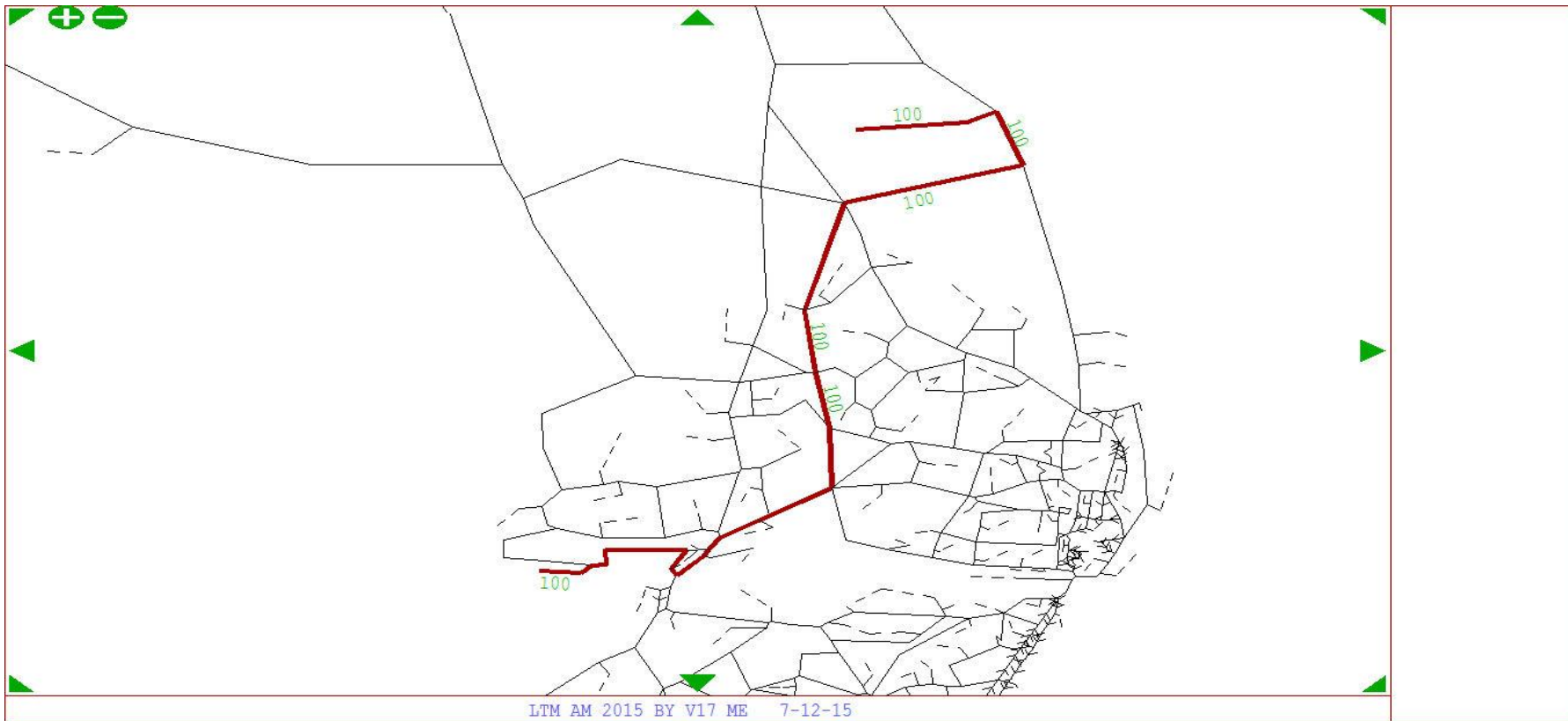
From Zone 102 To Zone 122 - User Class 4



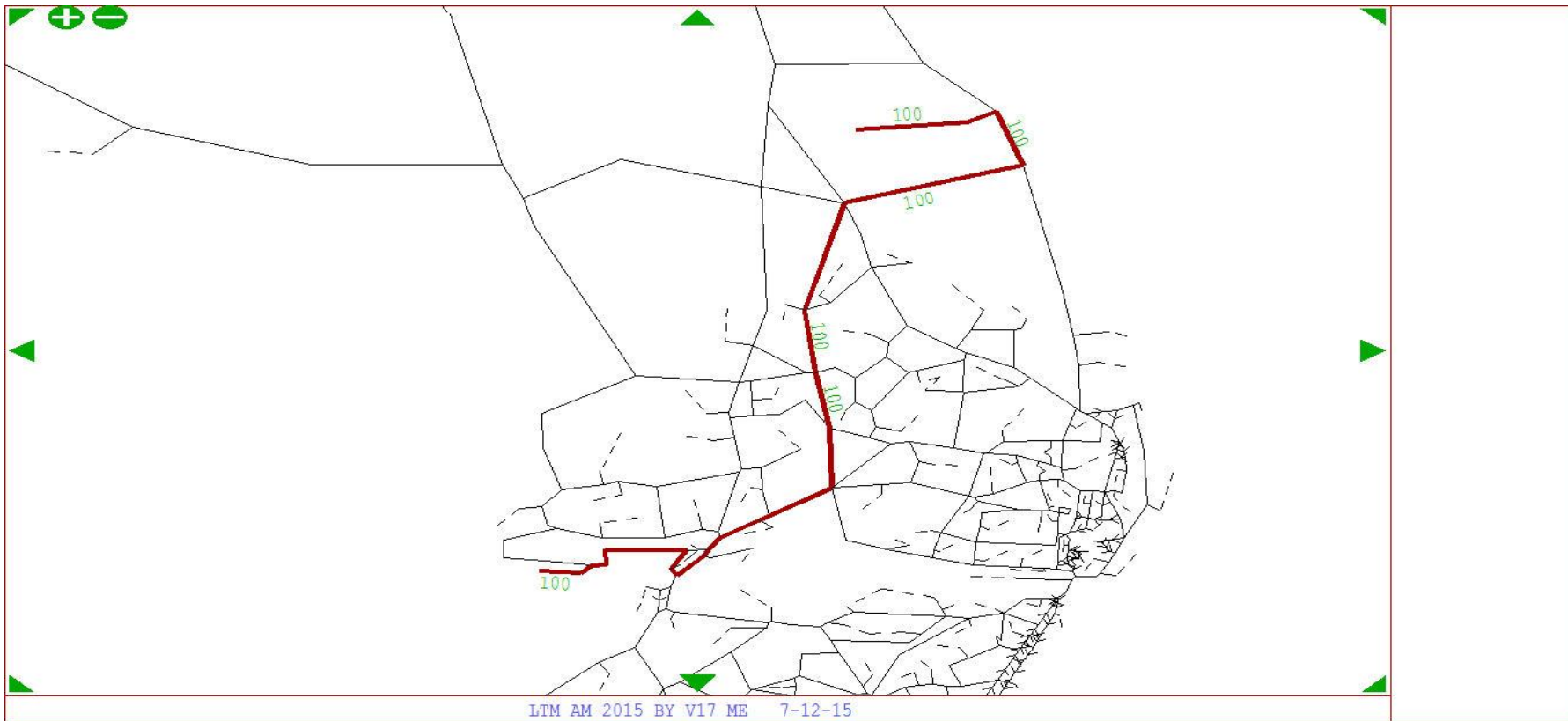
From Zone 102 To Zone 122 - User Class 5



From Zone 102 To Zone 130 - User Class 1

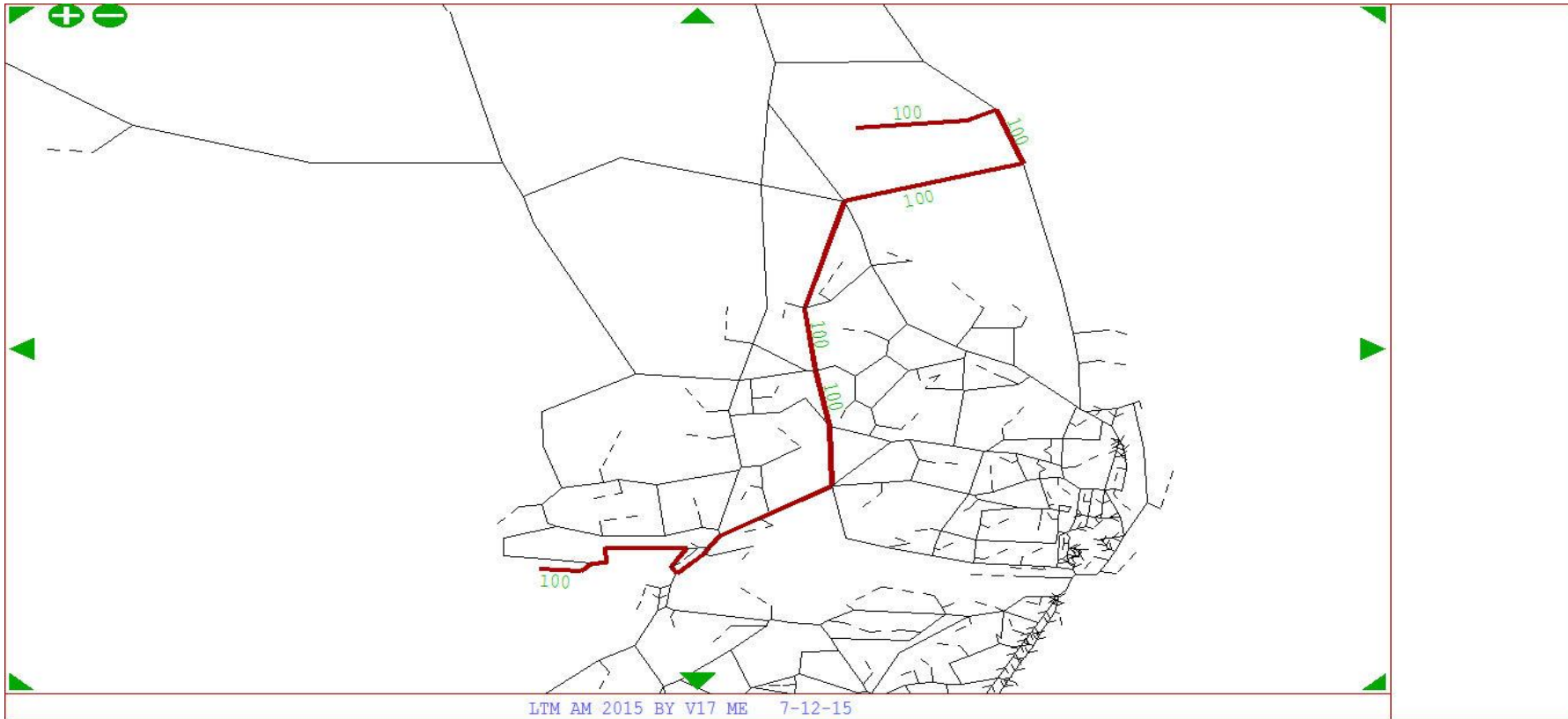


From Zone 102 To Zone 130 - User Class 2



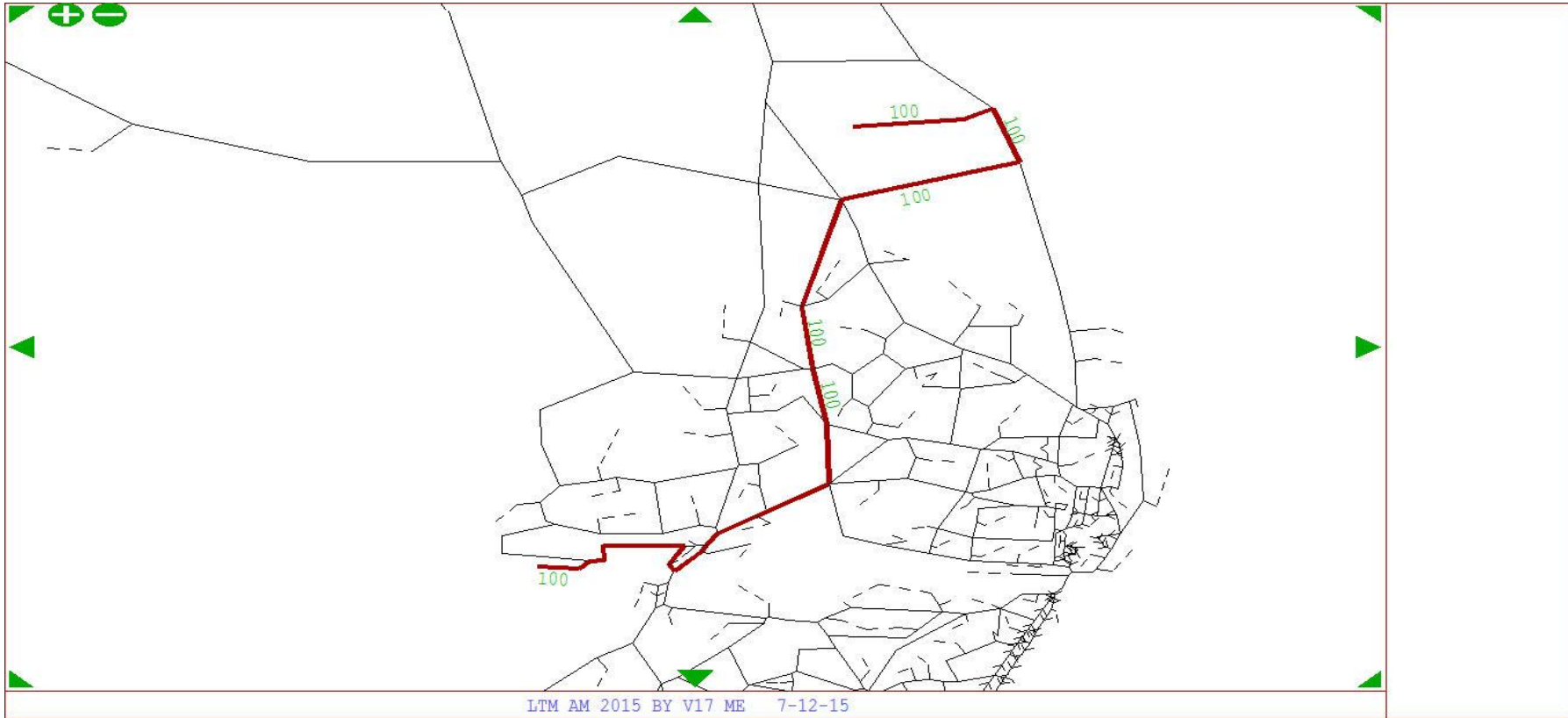
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From Zone 102 To Zone 130 - User Class 3

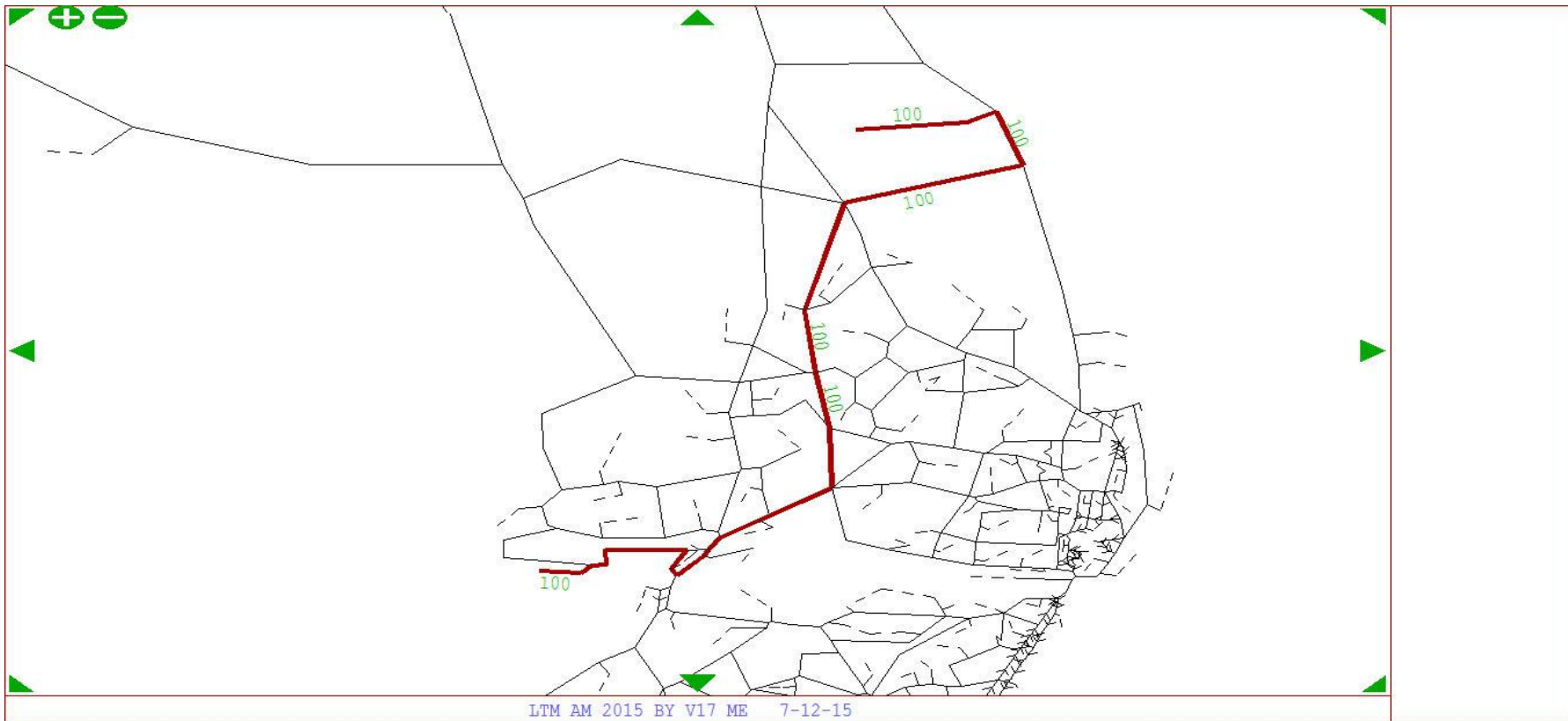


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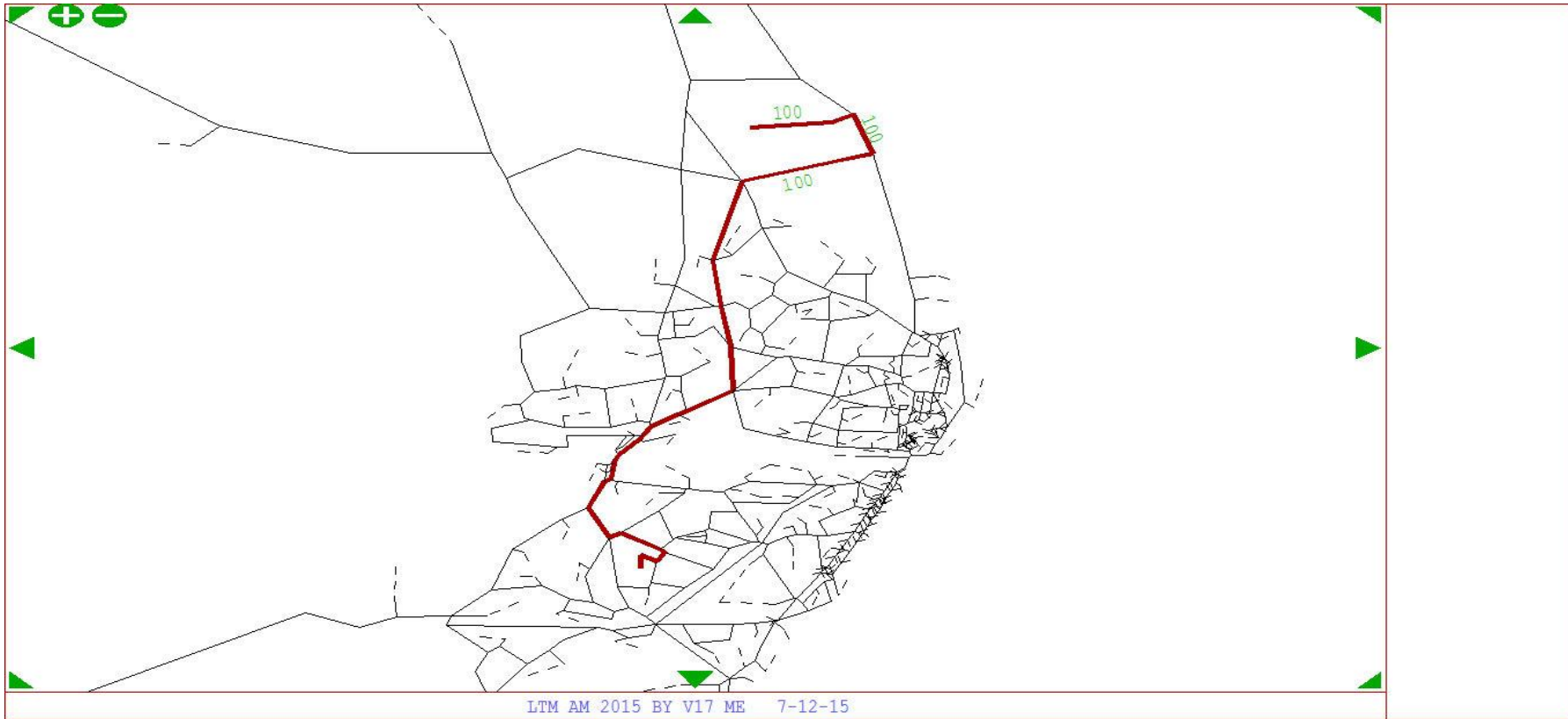
From Zone 102 To Zone 130 - User Class 4



From Zone 102 To Zone 130 - User Class 5

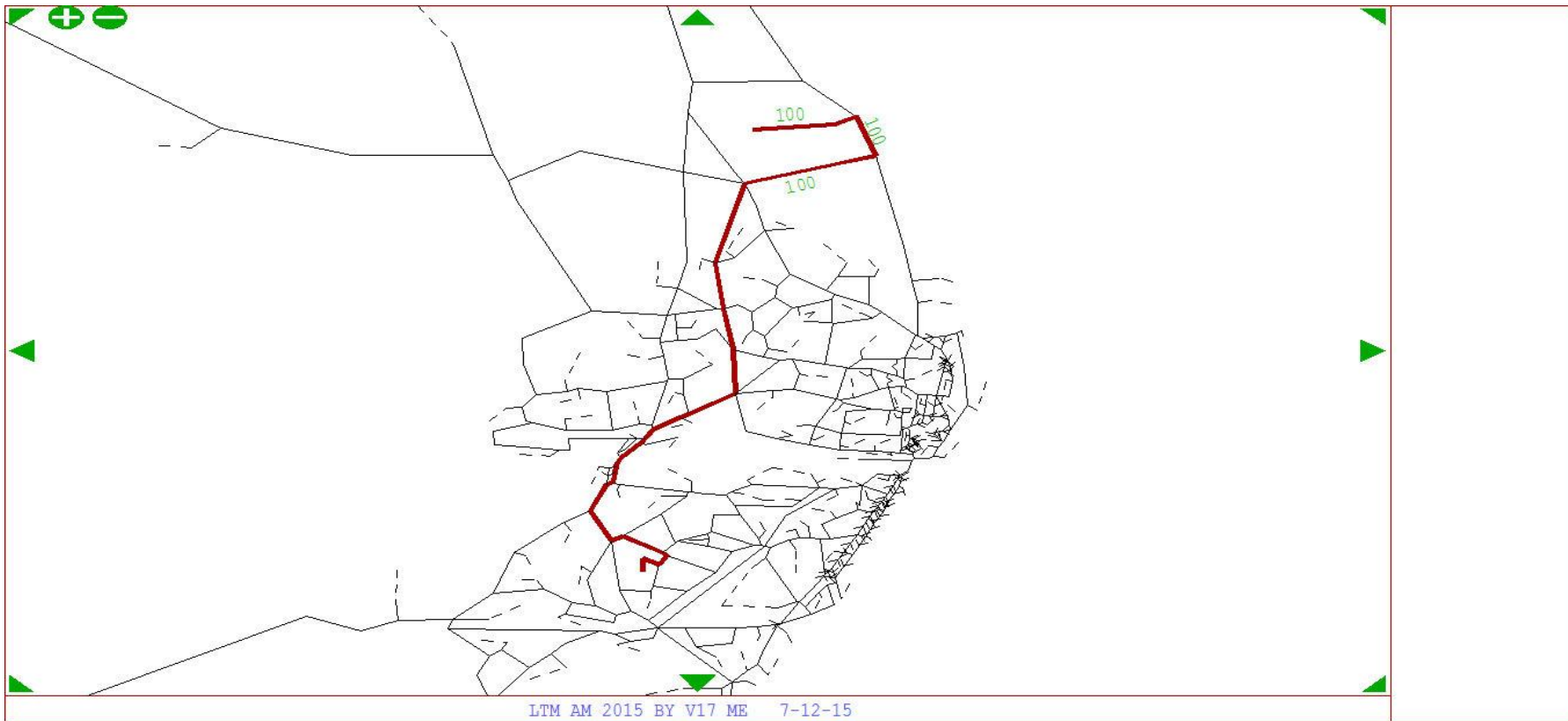


From Zone 102 To Zone 131 - User Class 1



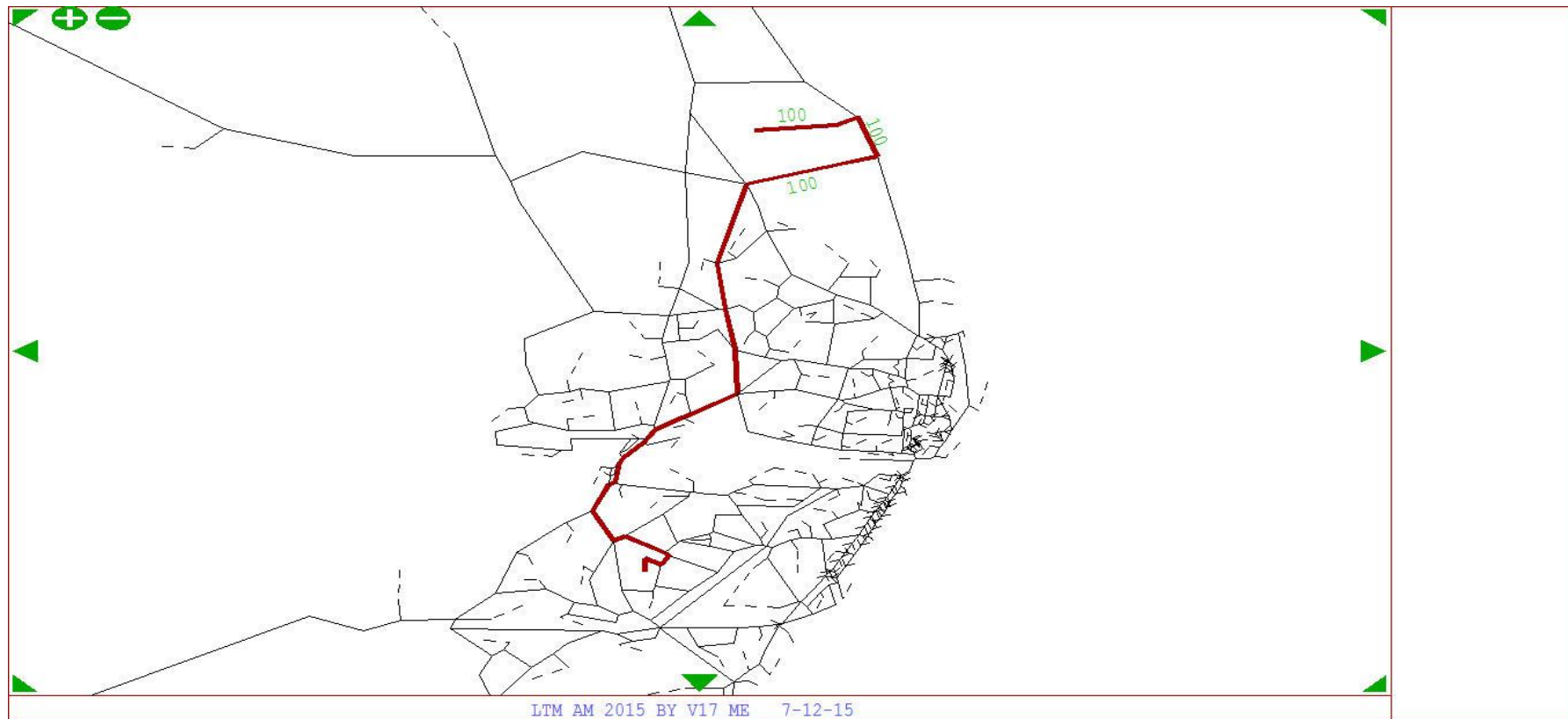
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From Zone 102 To Zone 131 - User Class 2



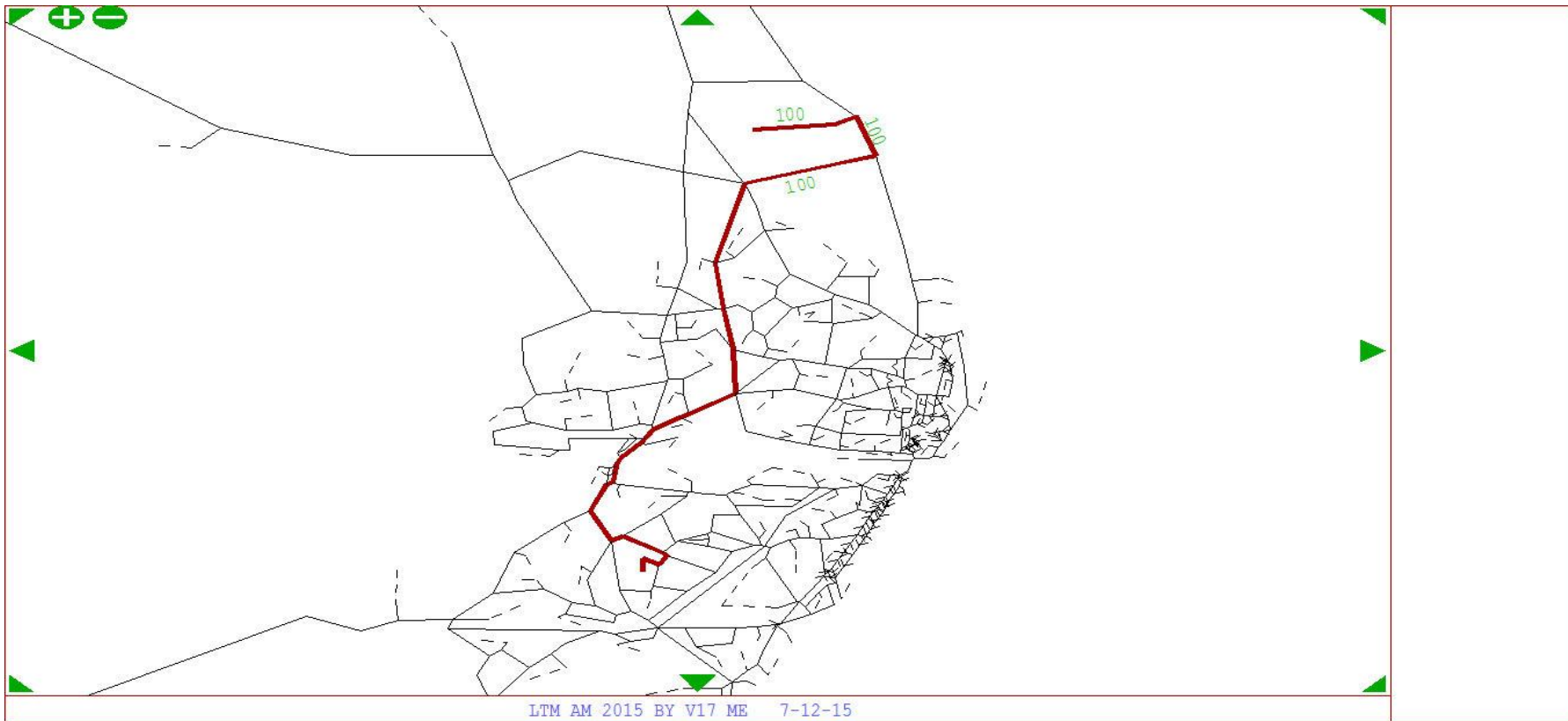
LTM AM 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 131 - User Class 3

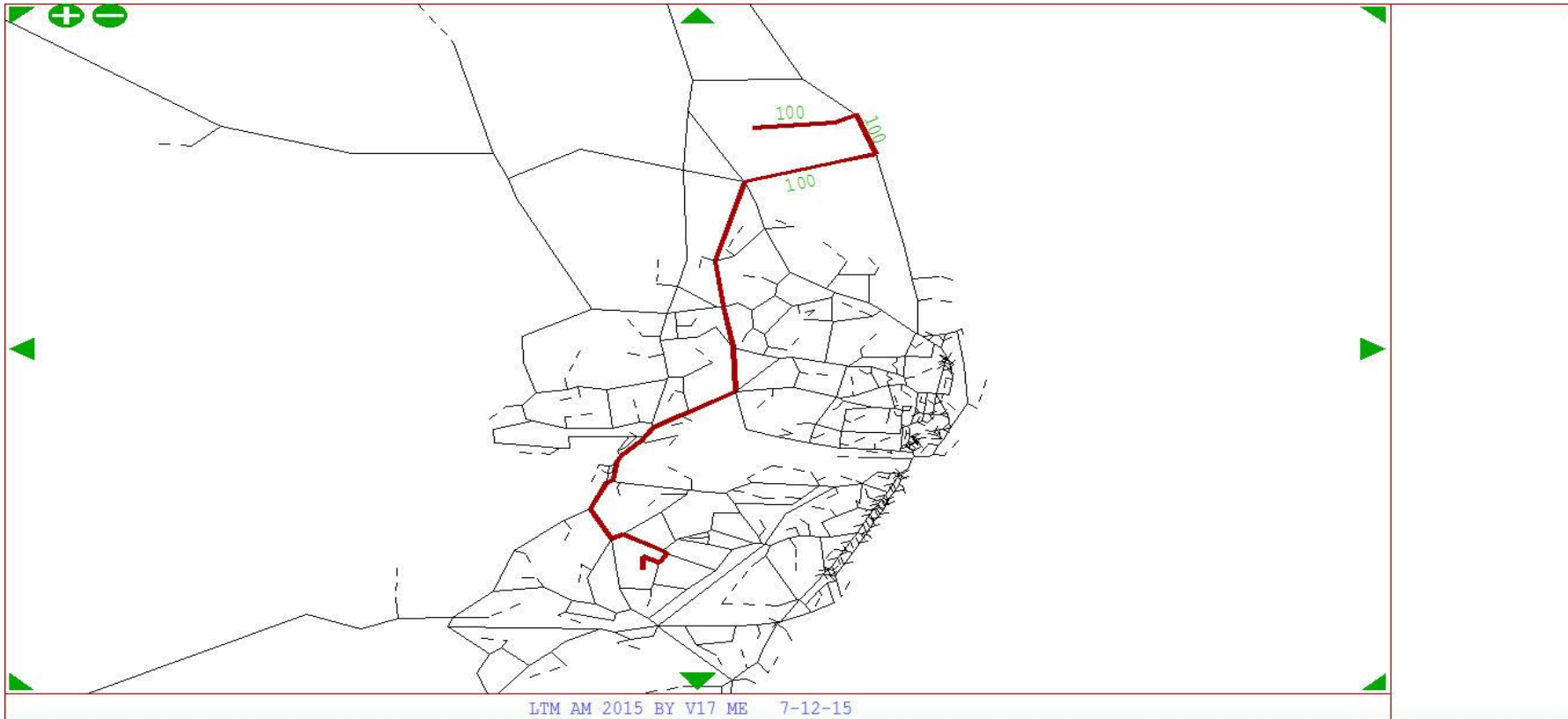


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From Zone 102 To Zone 131 - User Class 4

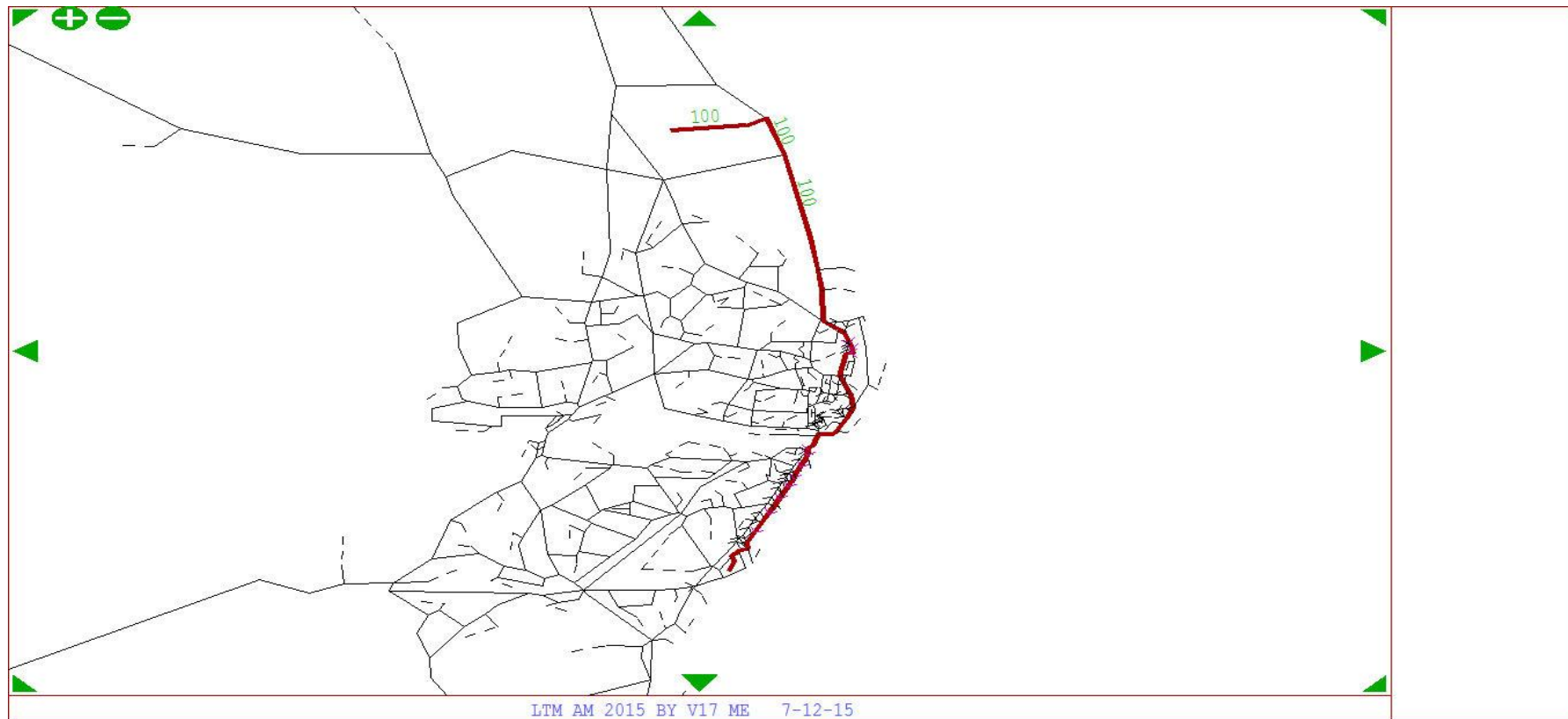


From Zone 102 To Zone 131 - User Class 5

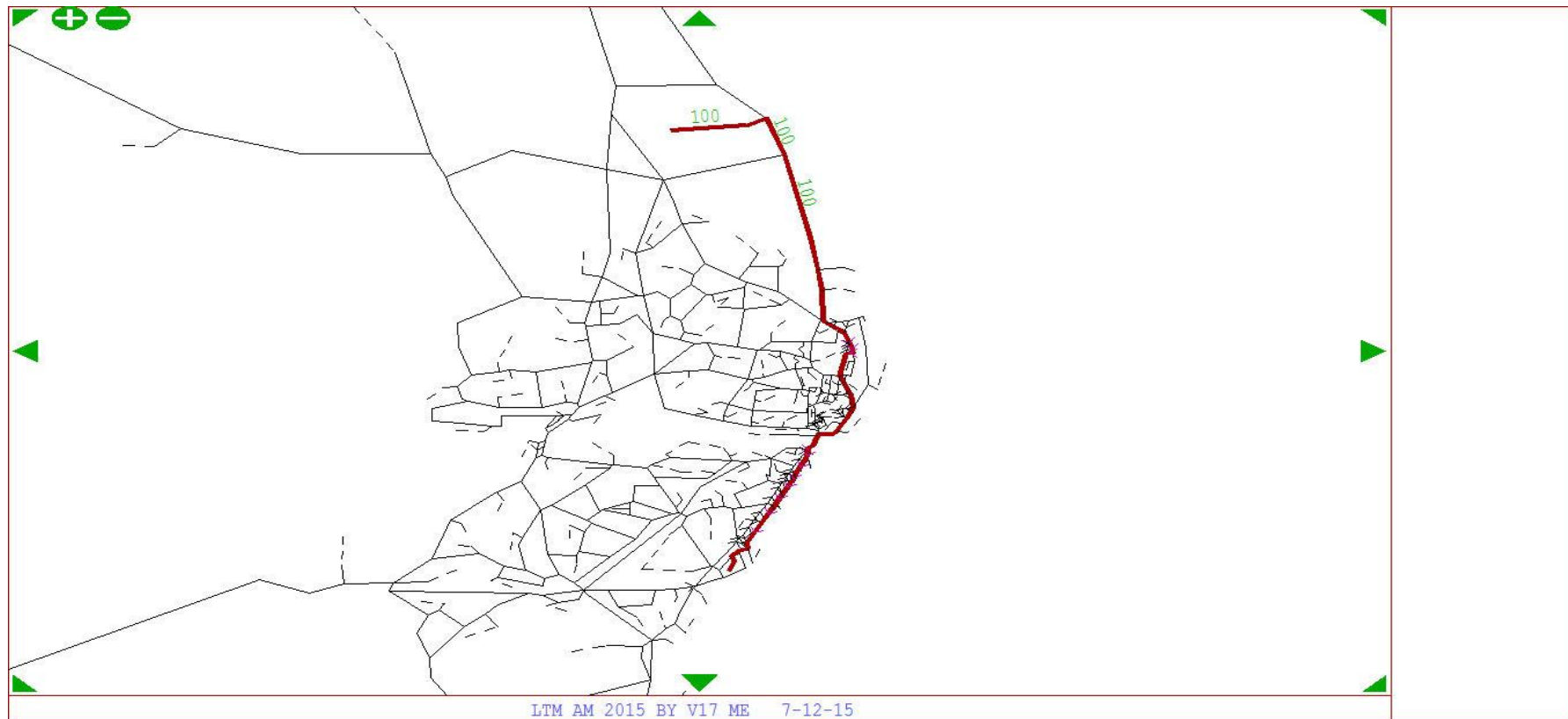


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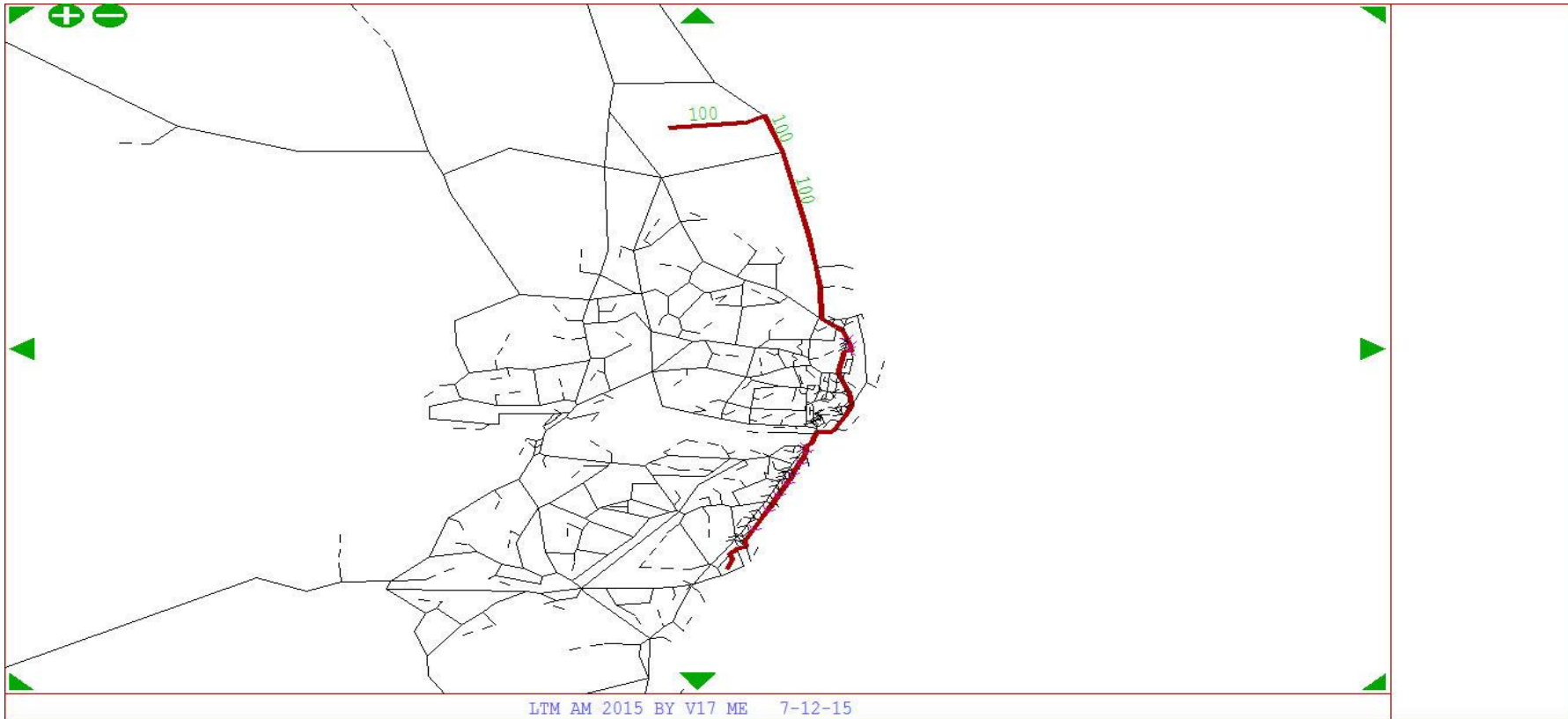
From Zone 102 To Zone 136 - User Class 1



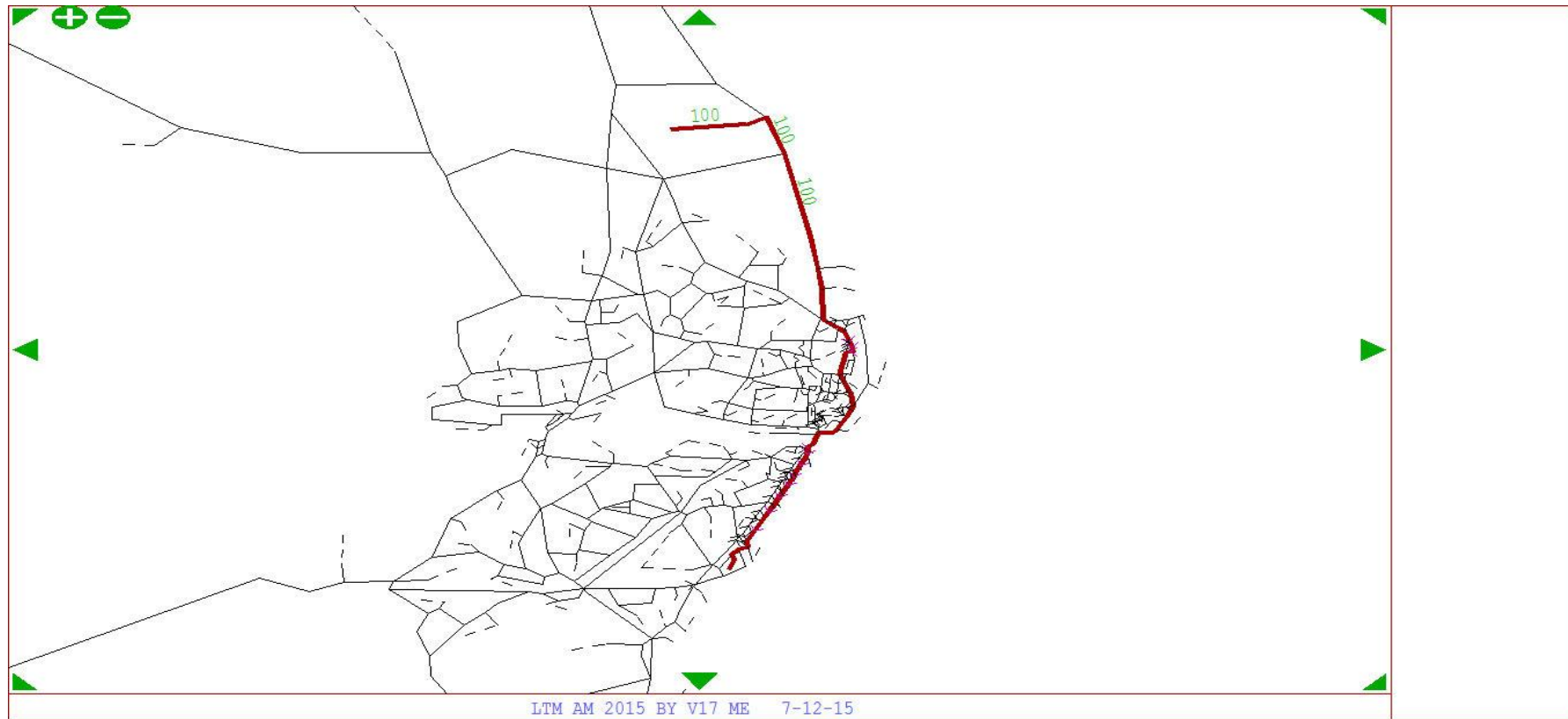
From Zone 102 To Zone 136 - User Class 2



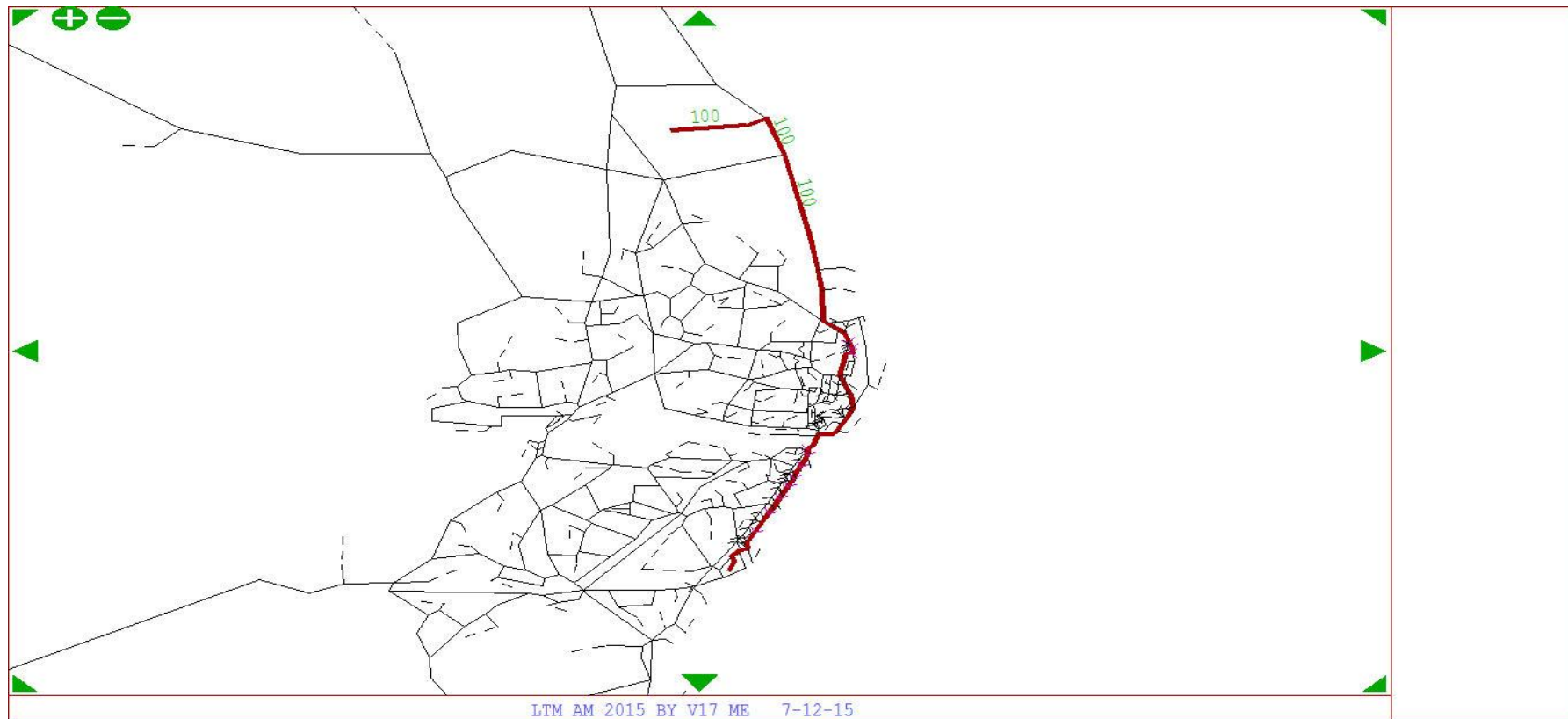
From Zone 102 To Zone 136 - User Class 3



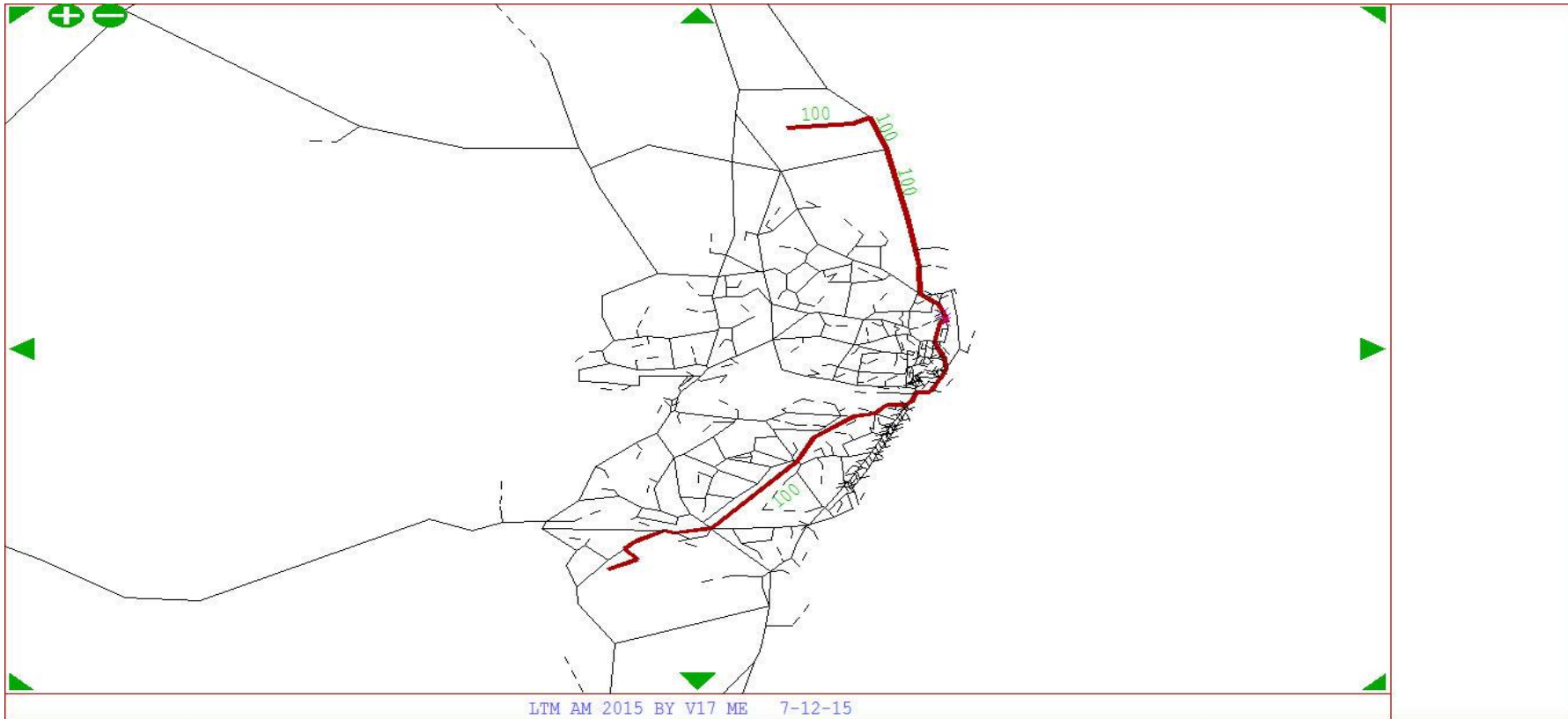
From Zone 102 To Zone 136 - User Class 4



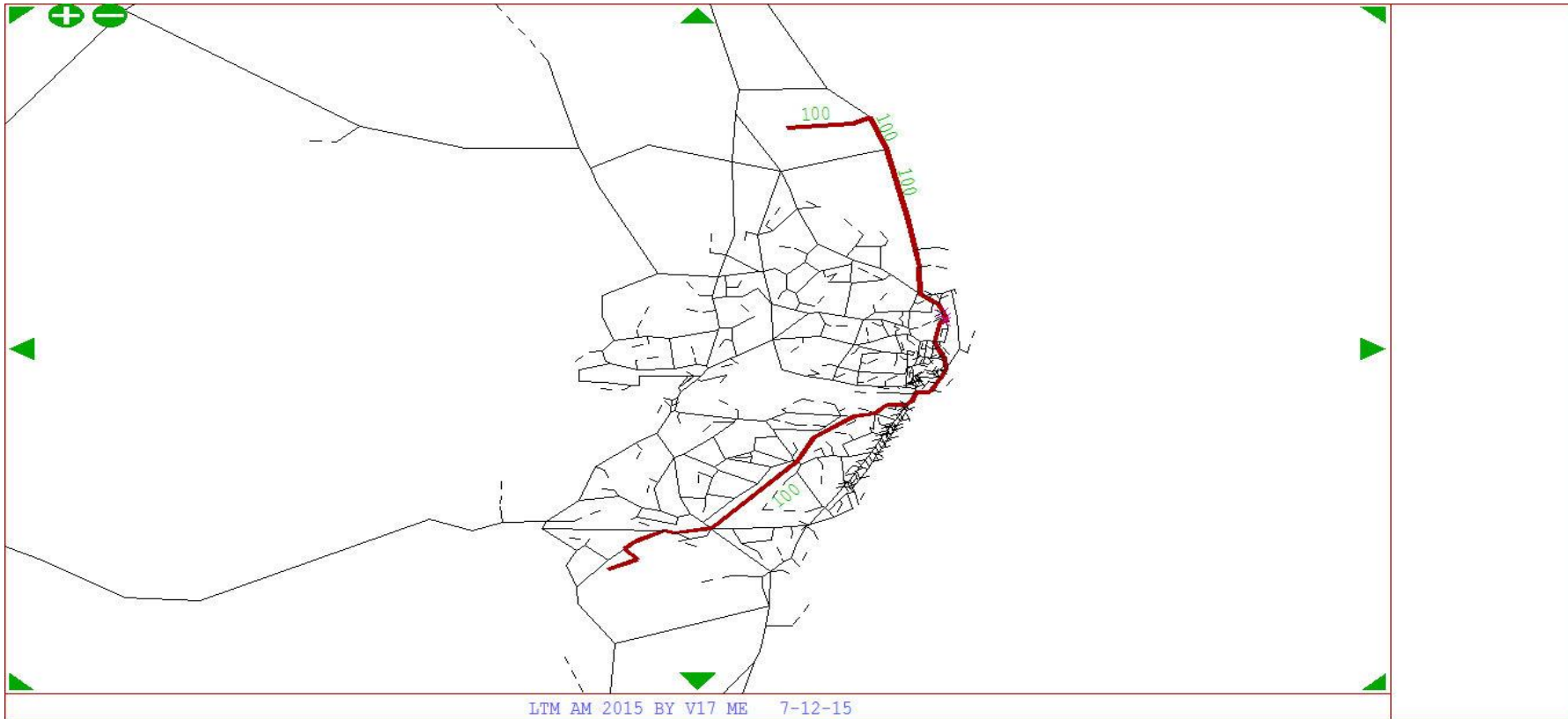
From Zone 102 To Zone 136 - User Class 5



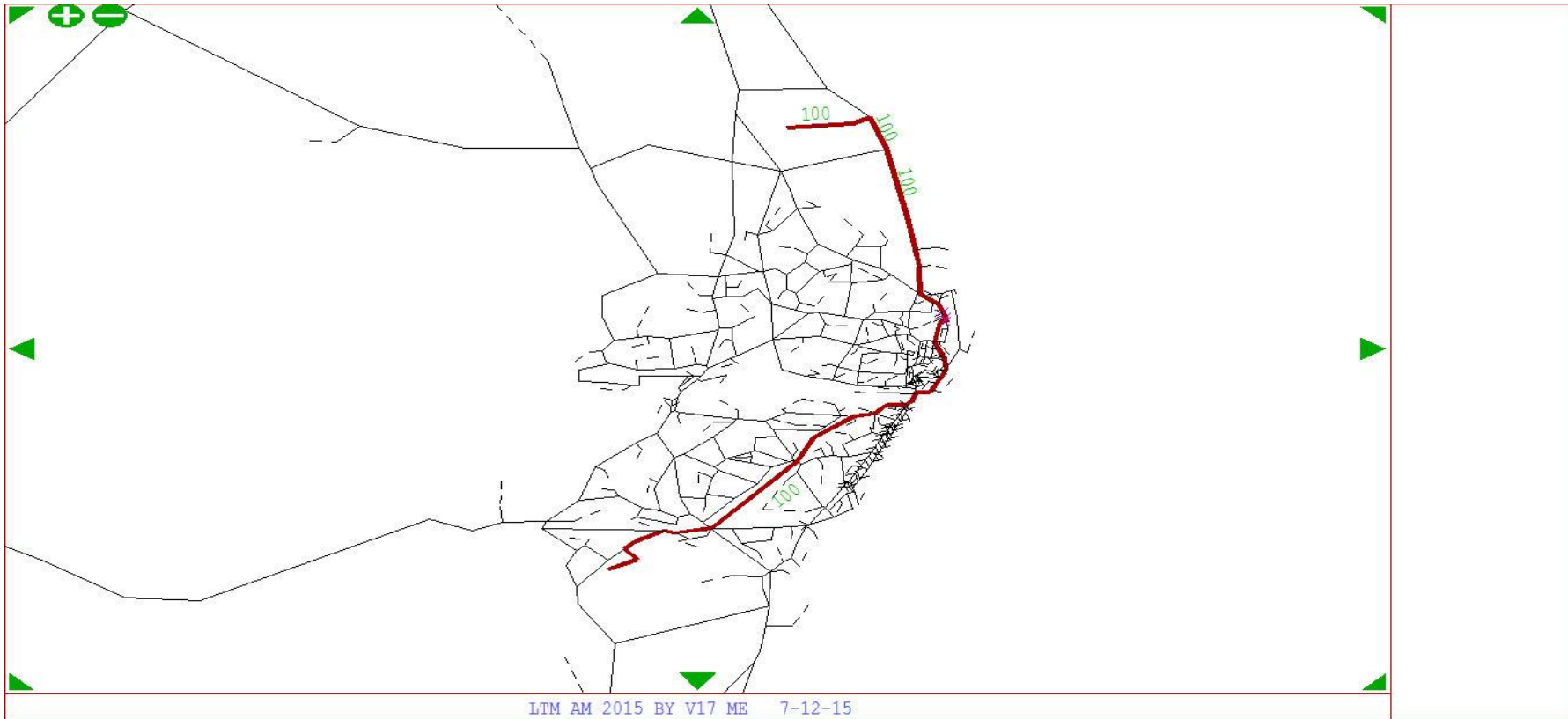
From Zone 102 To Zone 143 - User Class 1



From Zone 102 To Zone 143 - User Class 2

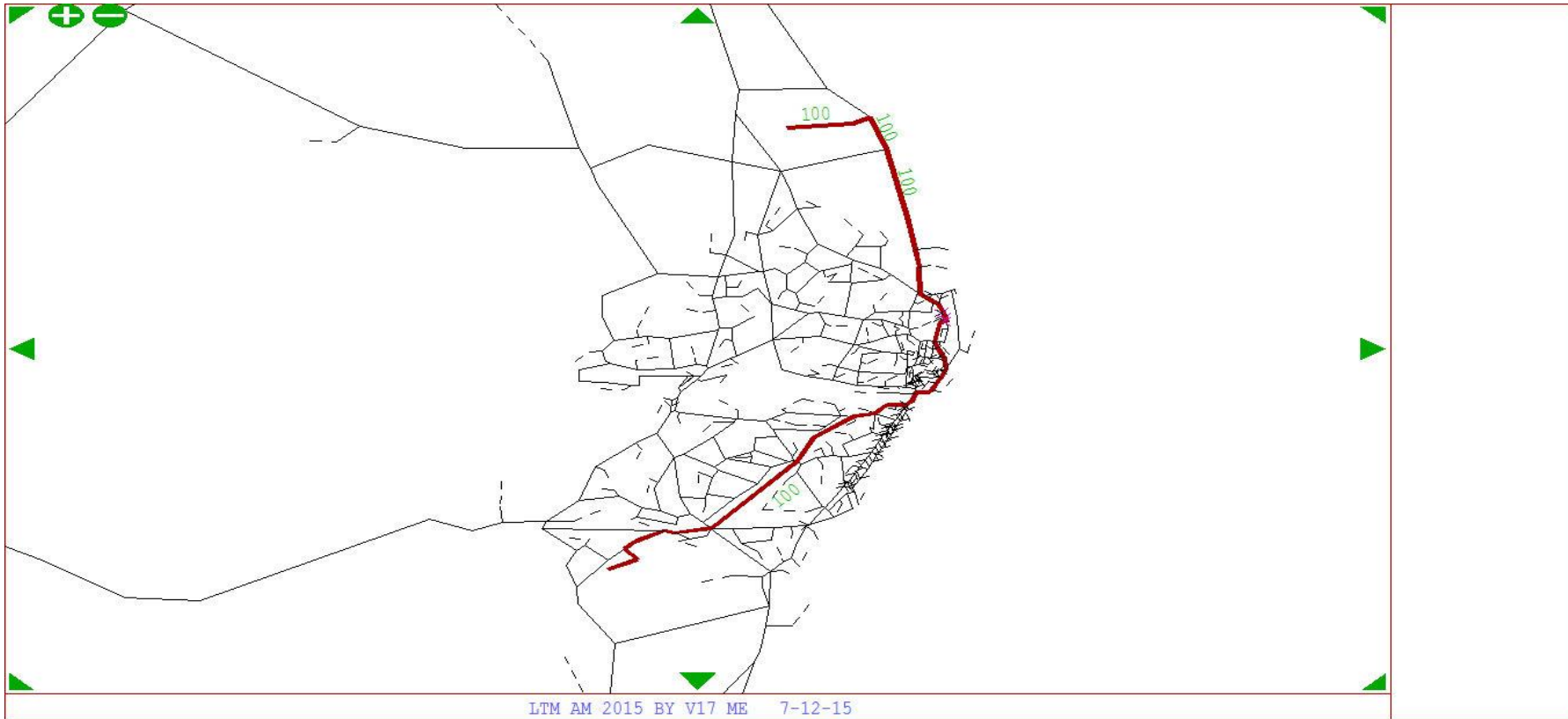


From Zone 102 To Zone 143 - User Class 3

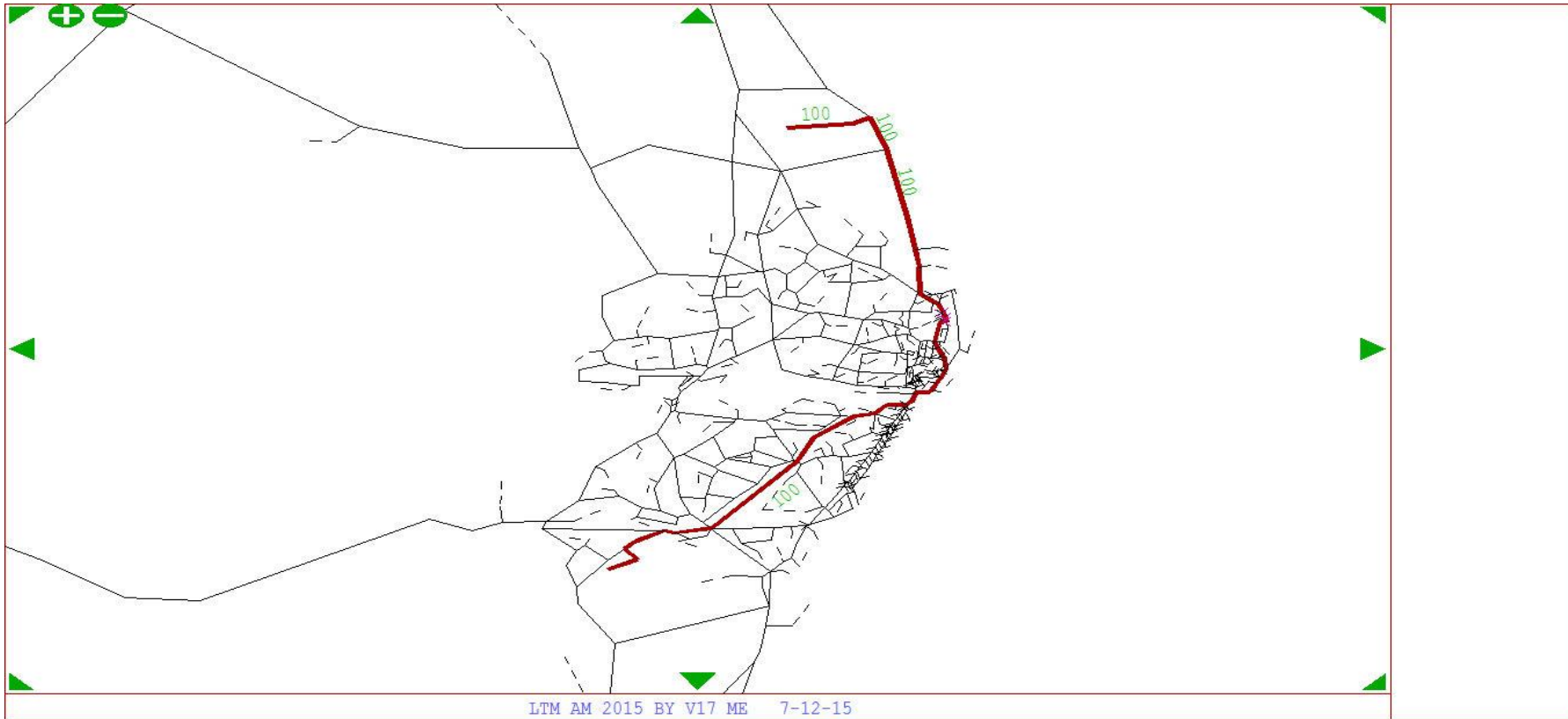


LTM AM 2015 BY V17 ME 7-12-15

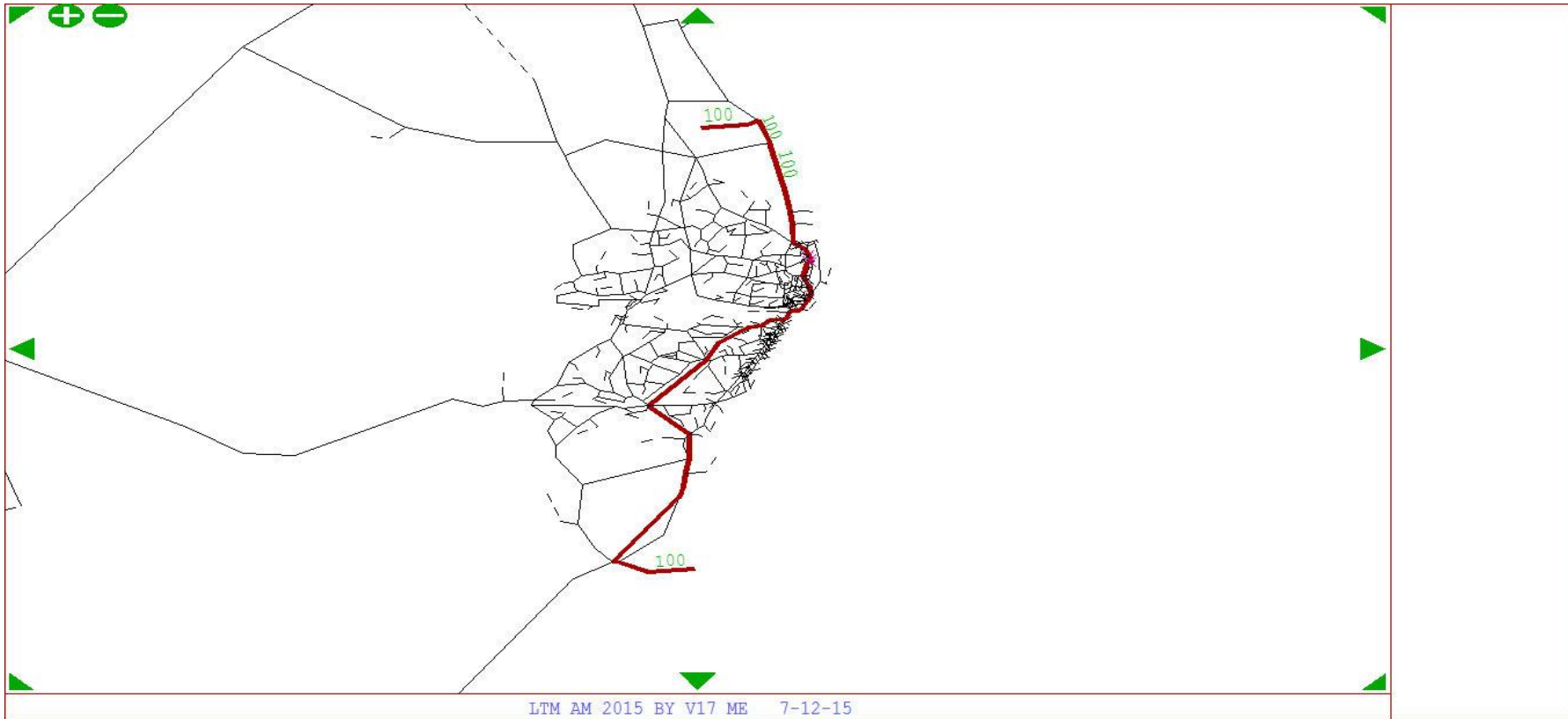
From Zone 102 To Zone 143 - User Class 4



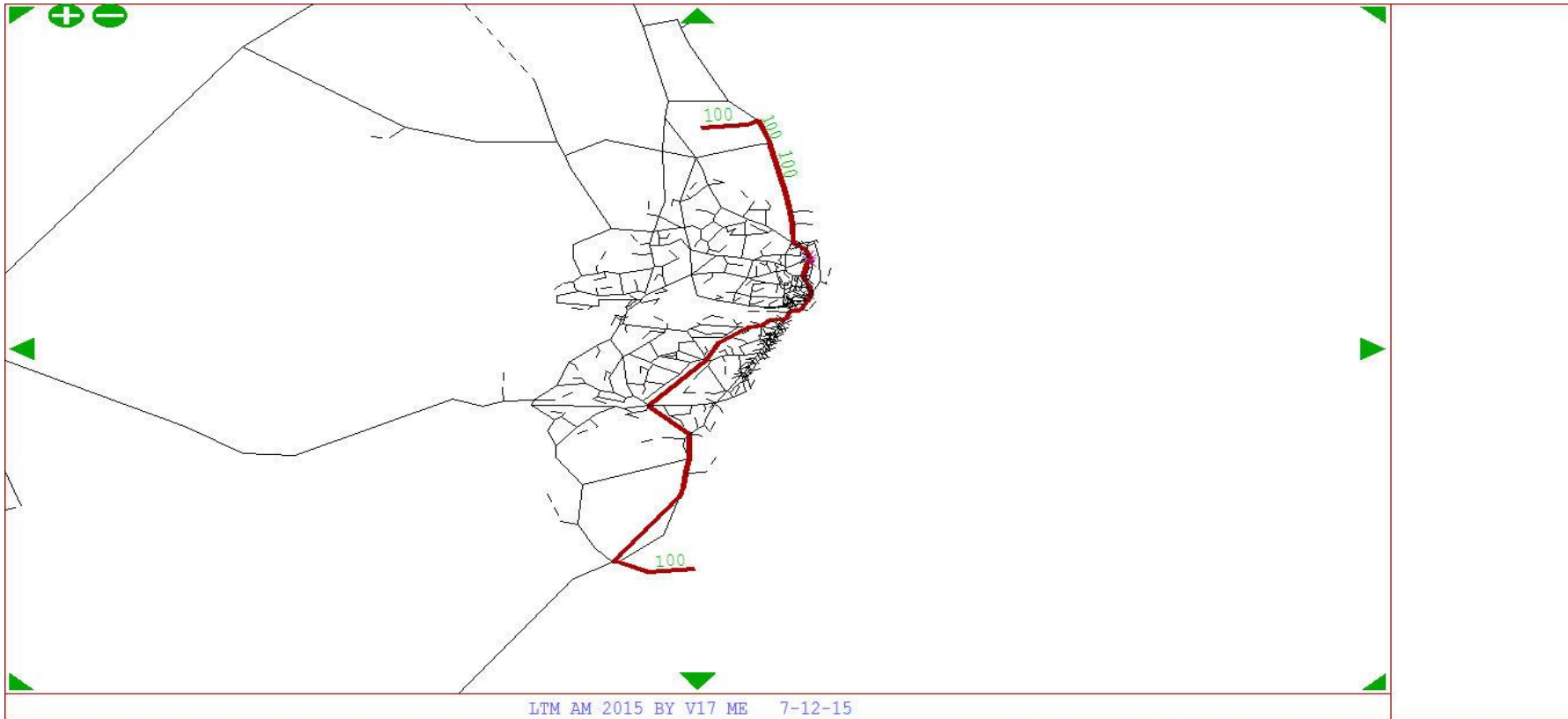
From Zone 102 To Zone 143 - User Class 5



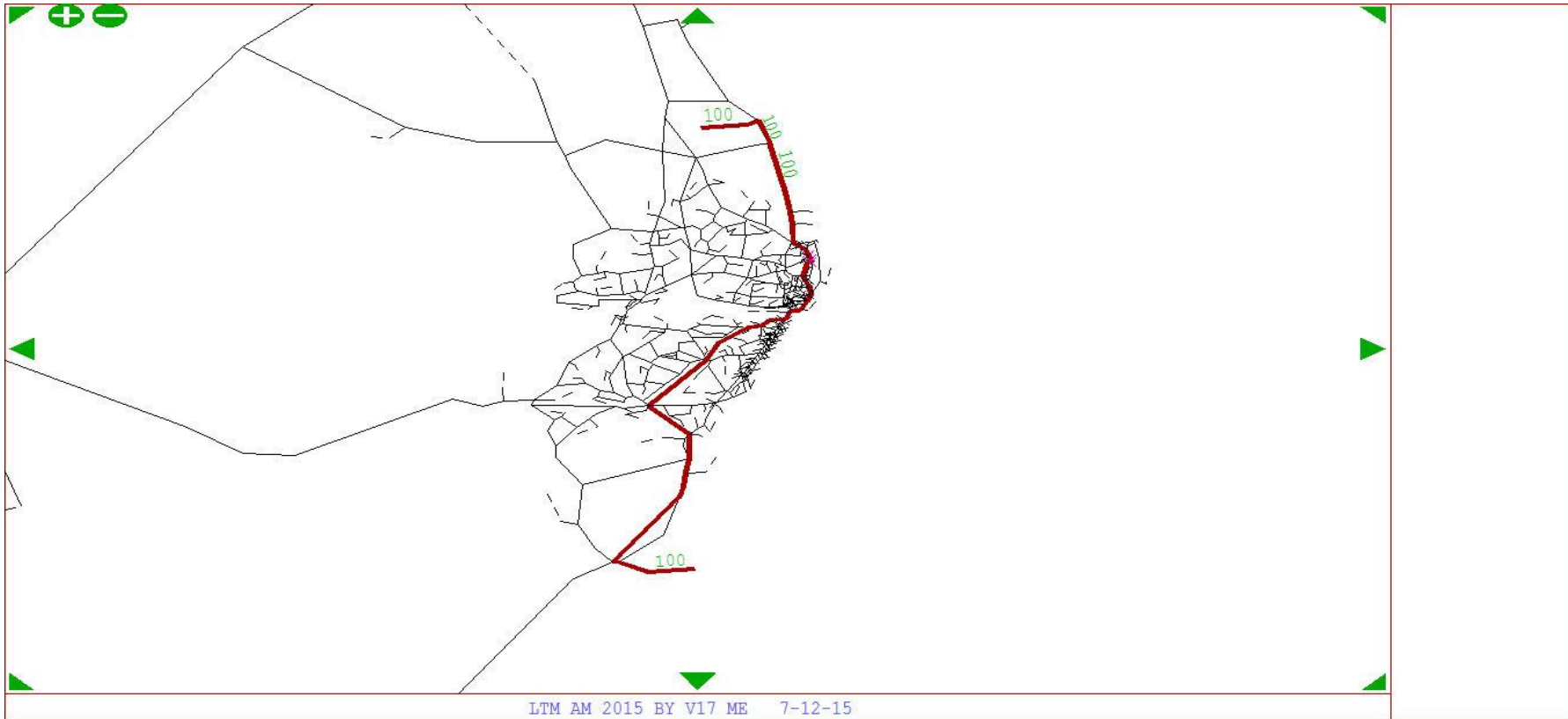
From Zone 102 To Zone 149 - User Class 1



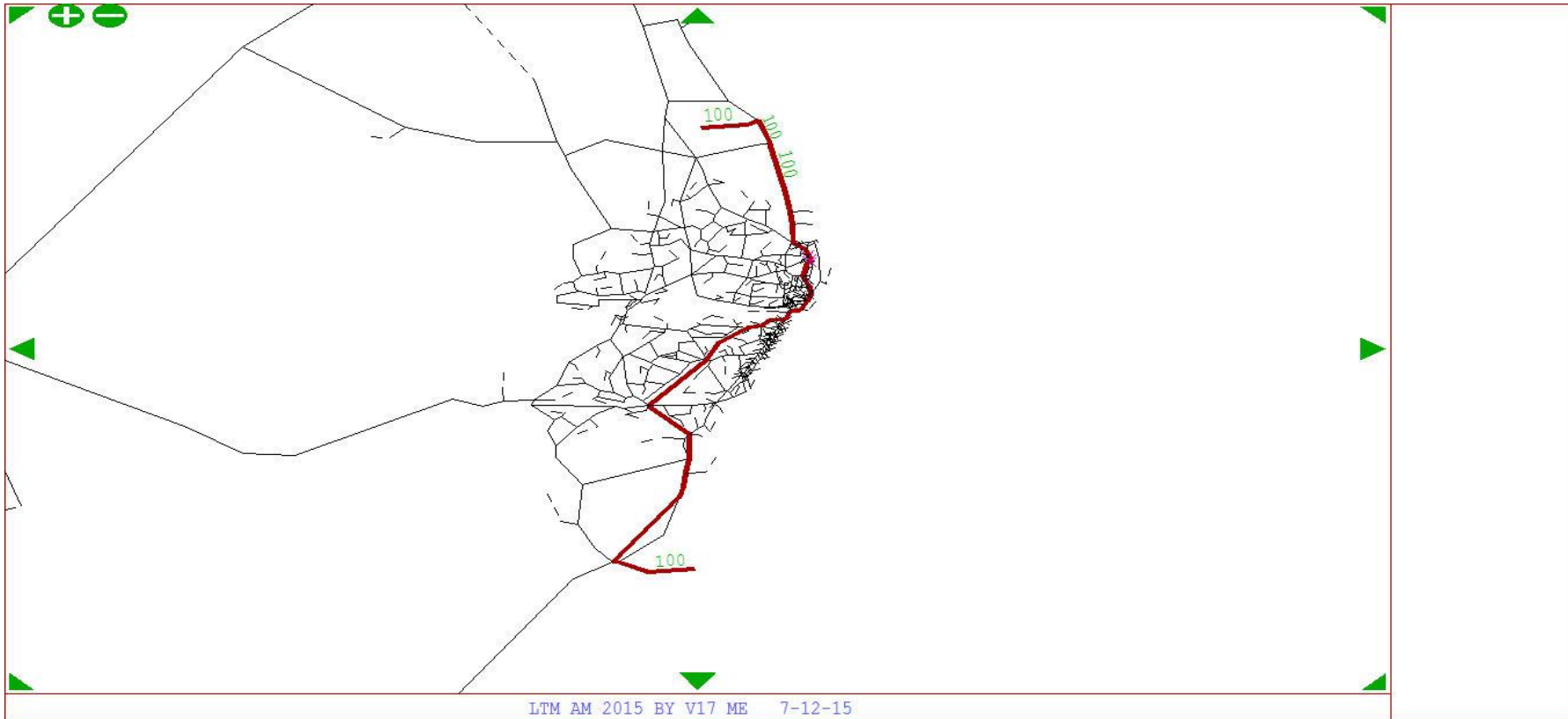
From Zone 102 To Zone 149 - User Class 2



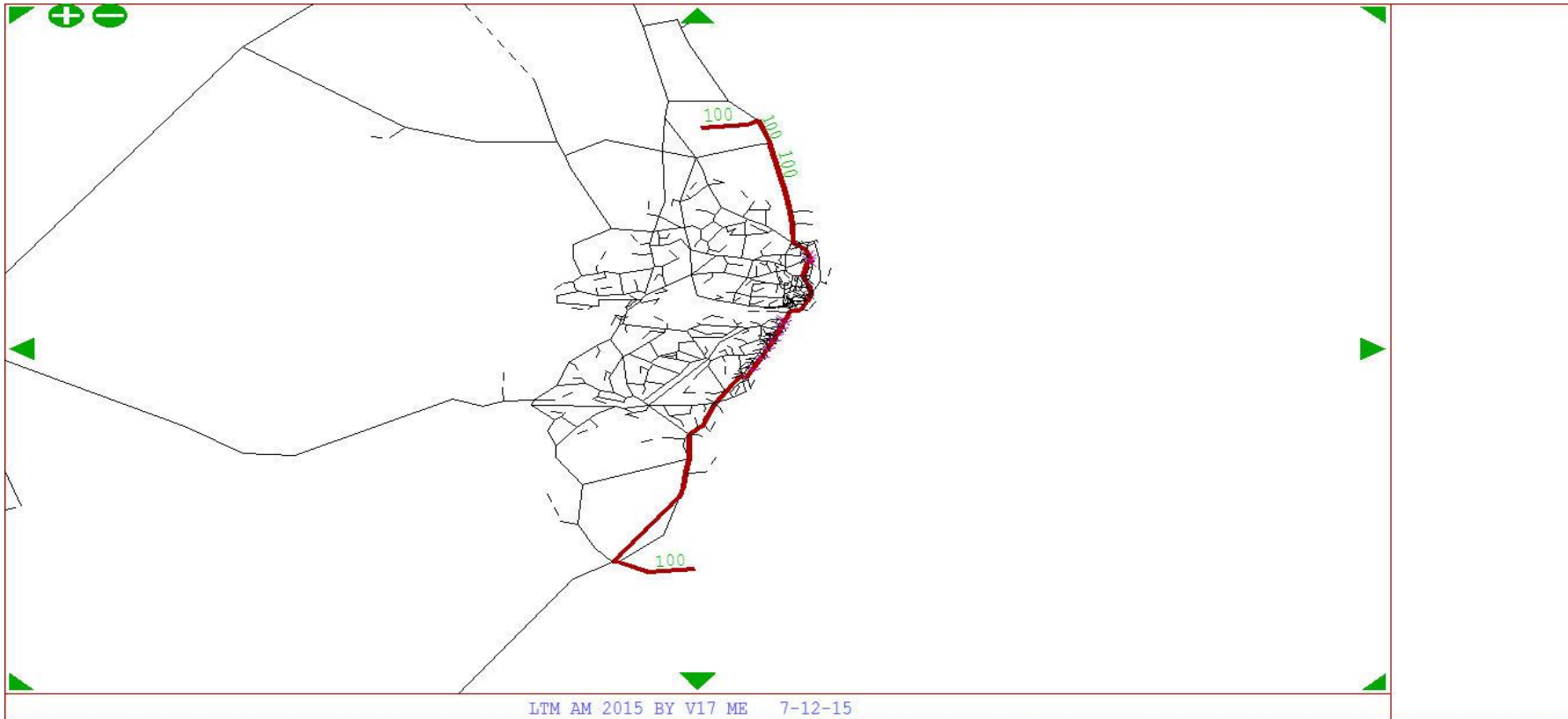
From Zone 102 To Zone 149 - User Class 3



From Zone 102 To Zone 149 - User Class 4

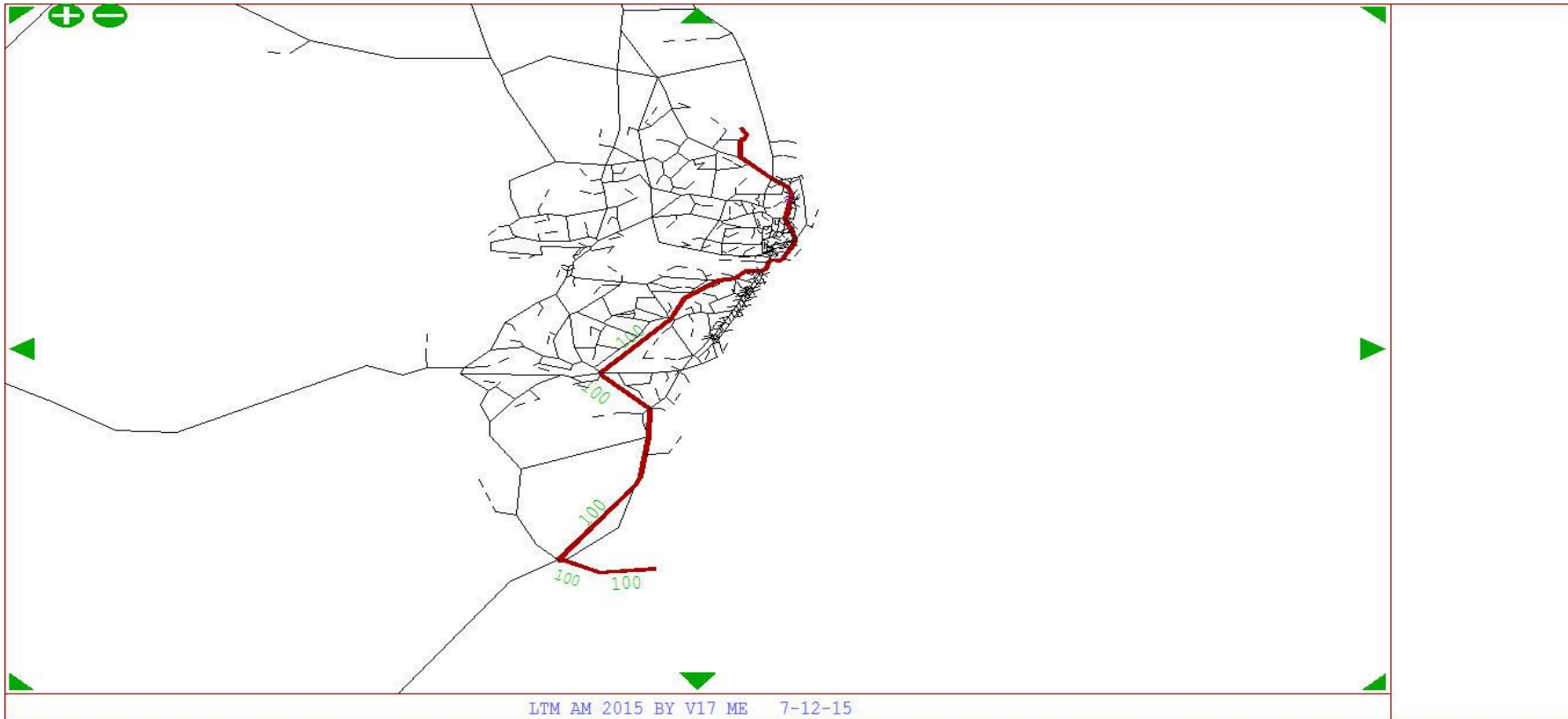


From Zone 102 To Zone 149 - User Class 5



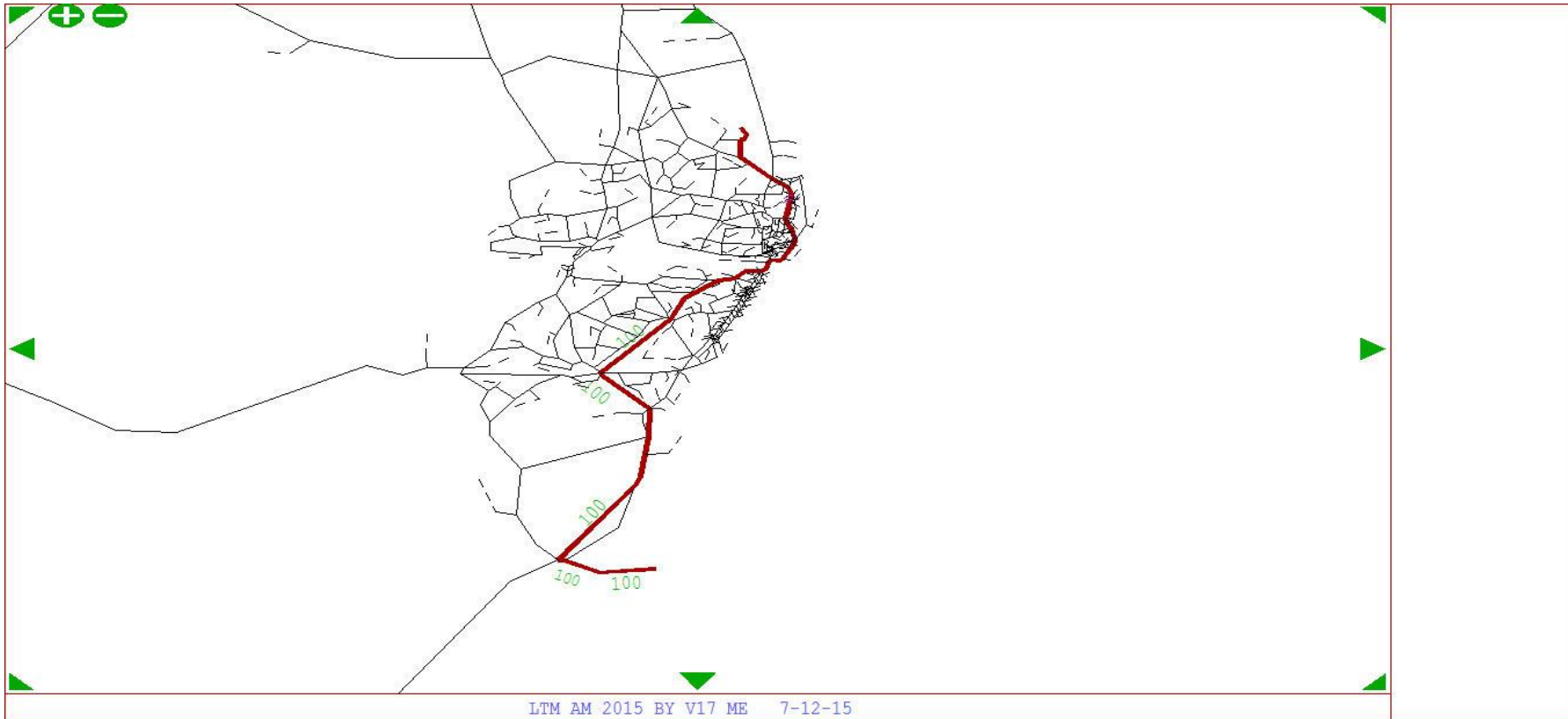
LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 101 - User Class 1

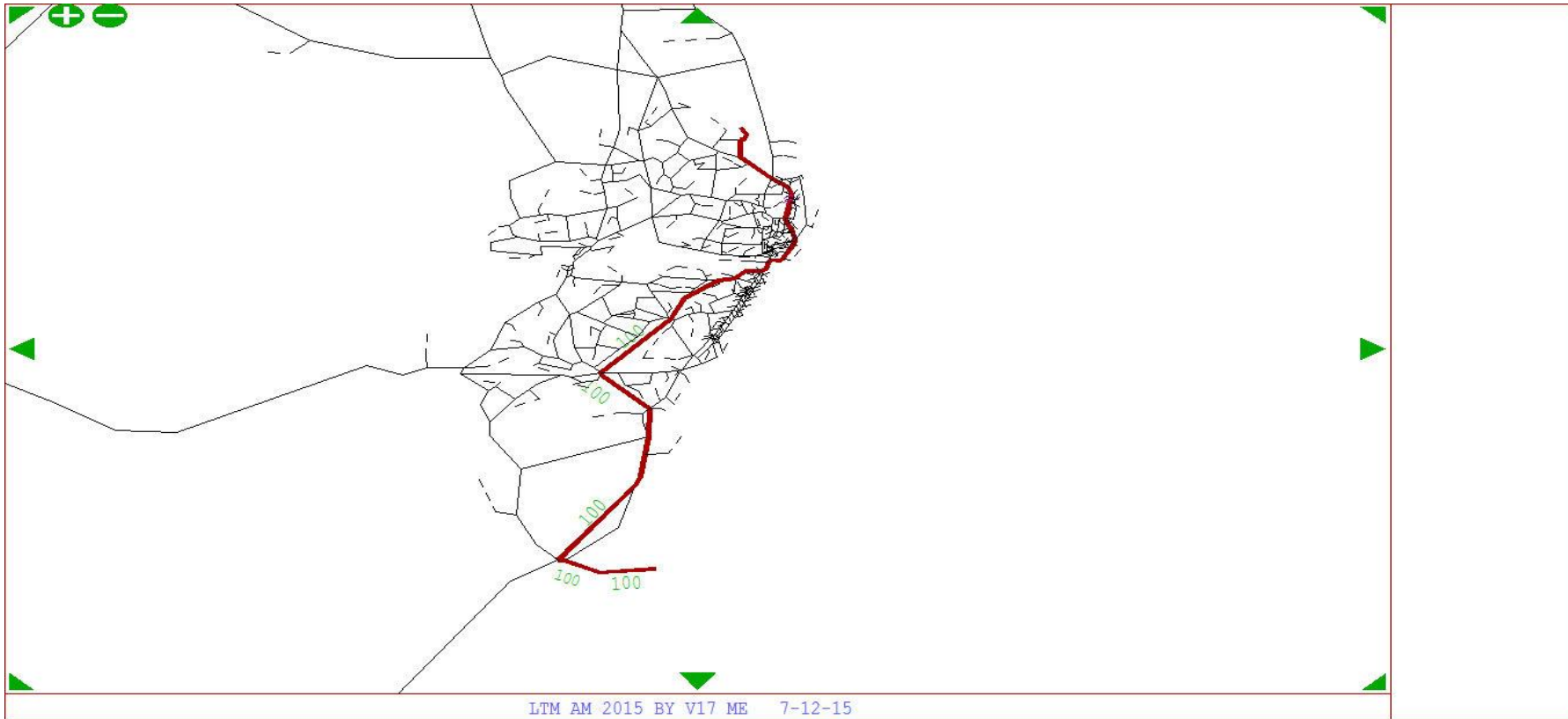


LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 101 - User Class 2

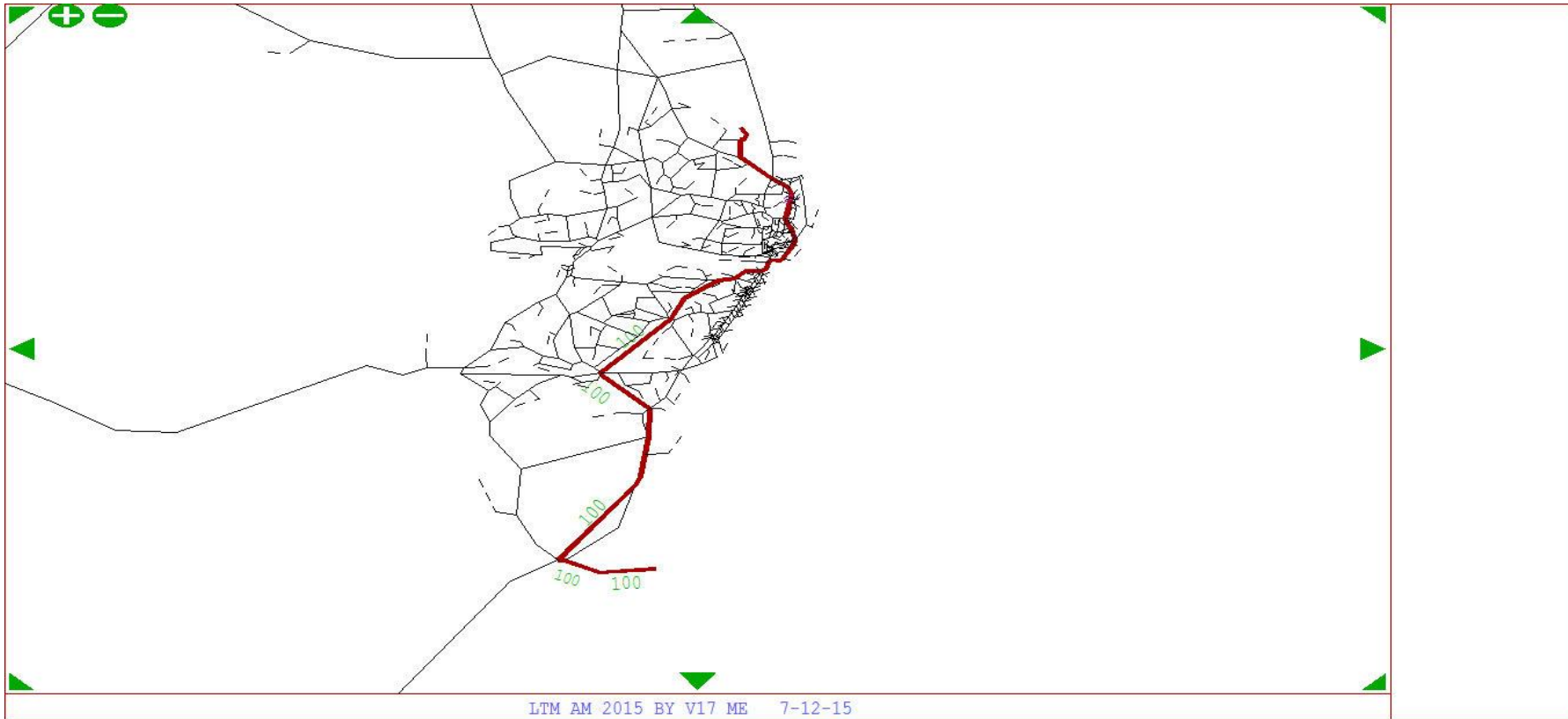


From Zone 149 To Zone 101 - User Class 3



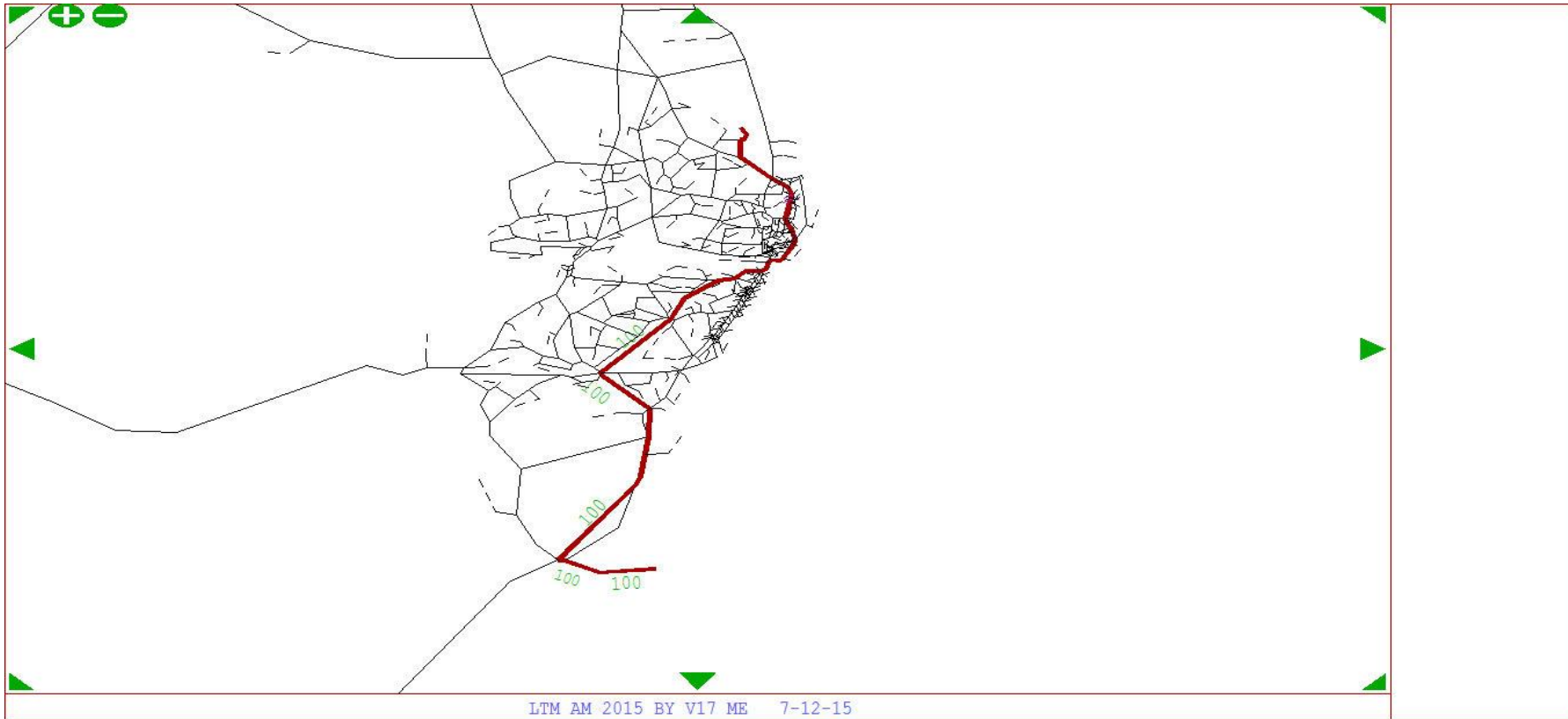
LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 101 - User Class 4

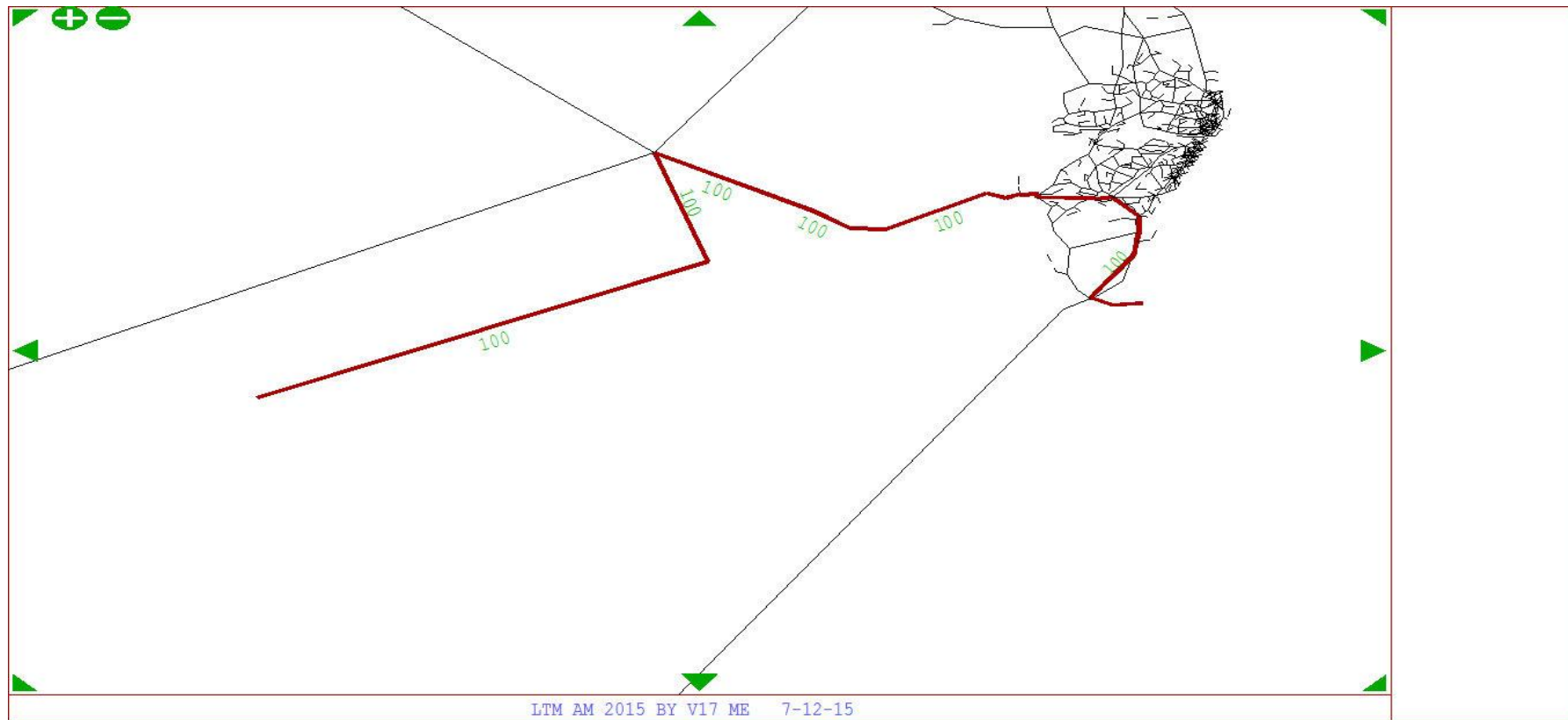


LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 101 - User Class 5

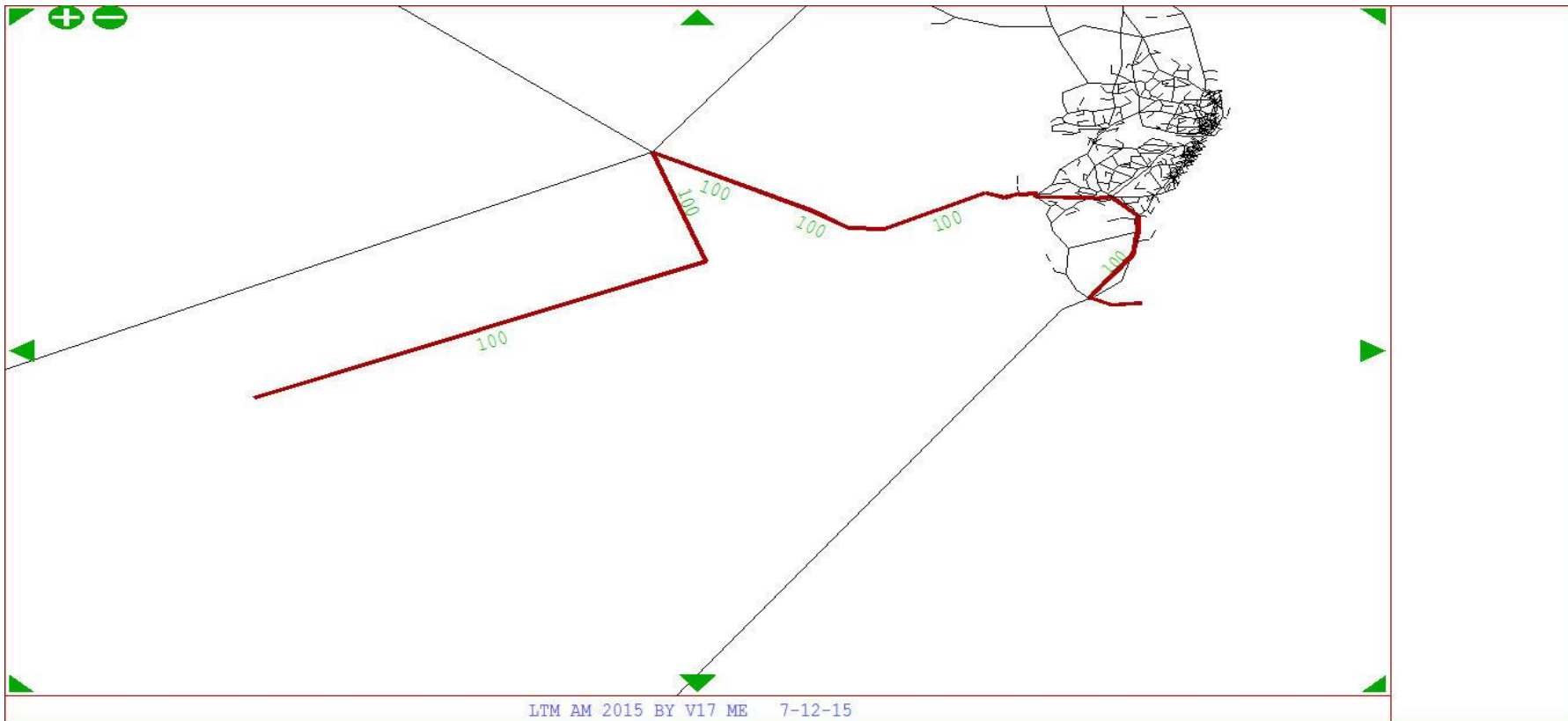


From Zone 149 To Zone 104 - User Class 1



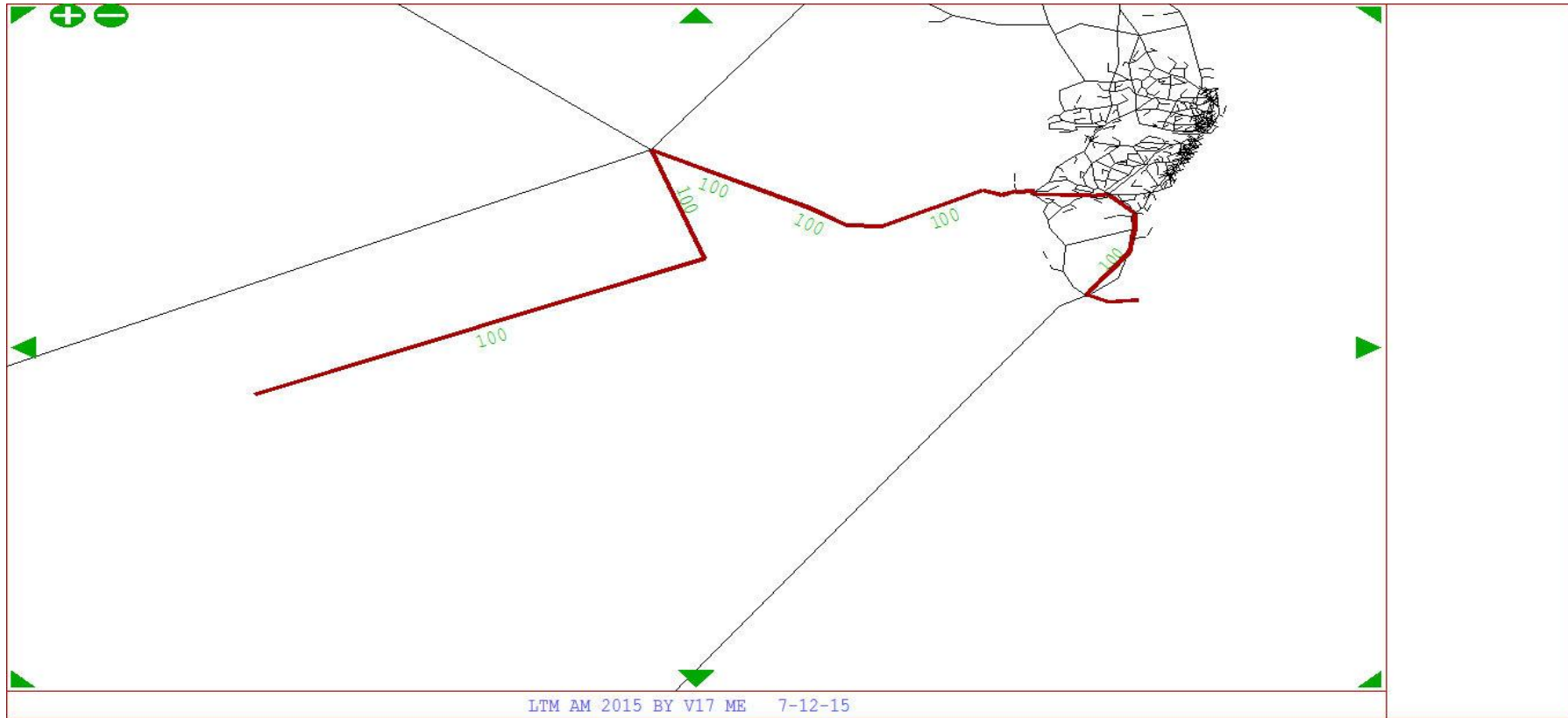
LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 104 - User Class 2

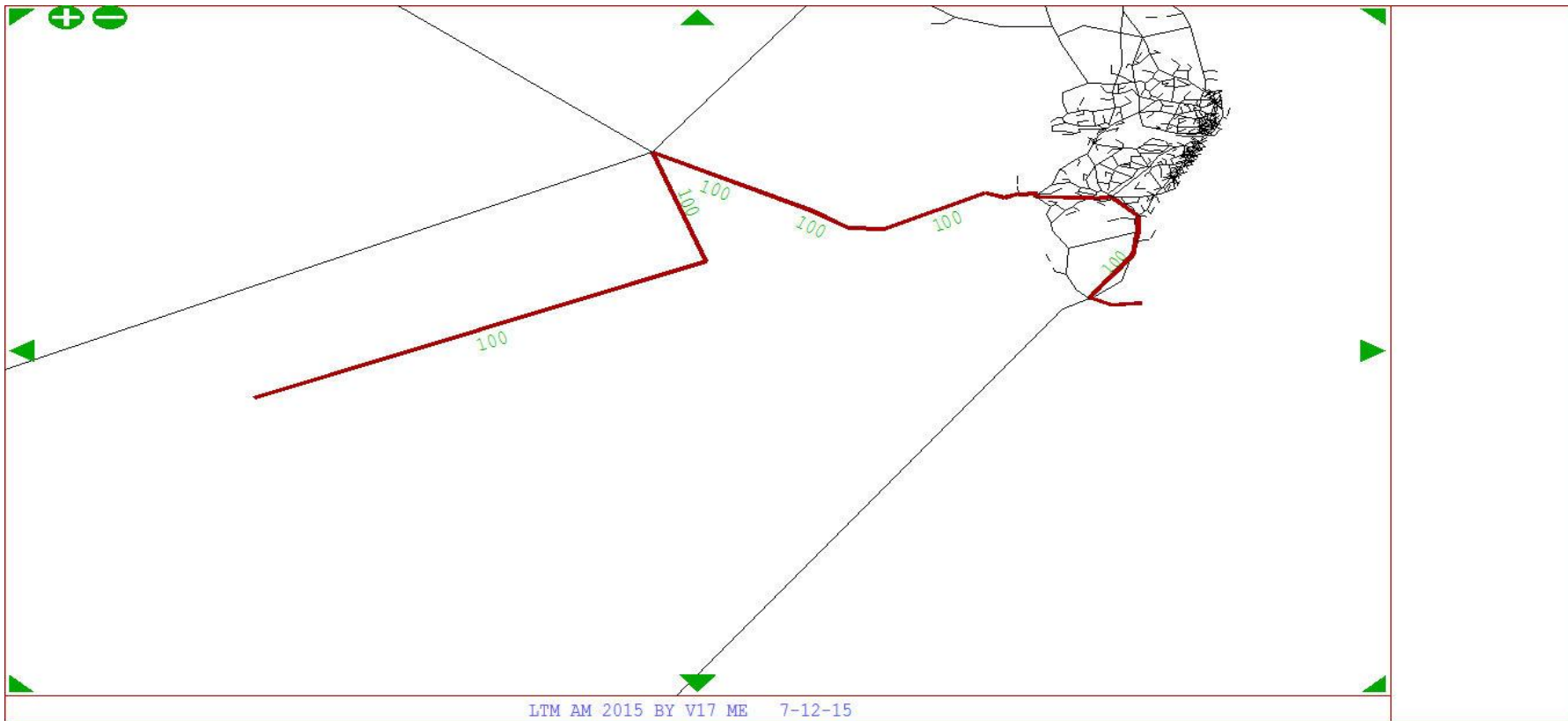


LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 104 - User Class 3

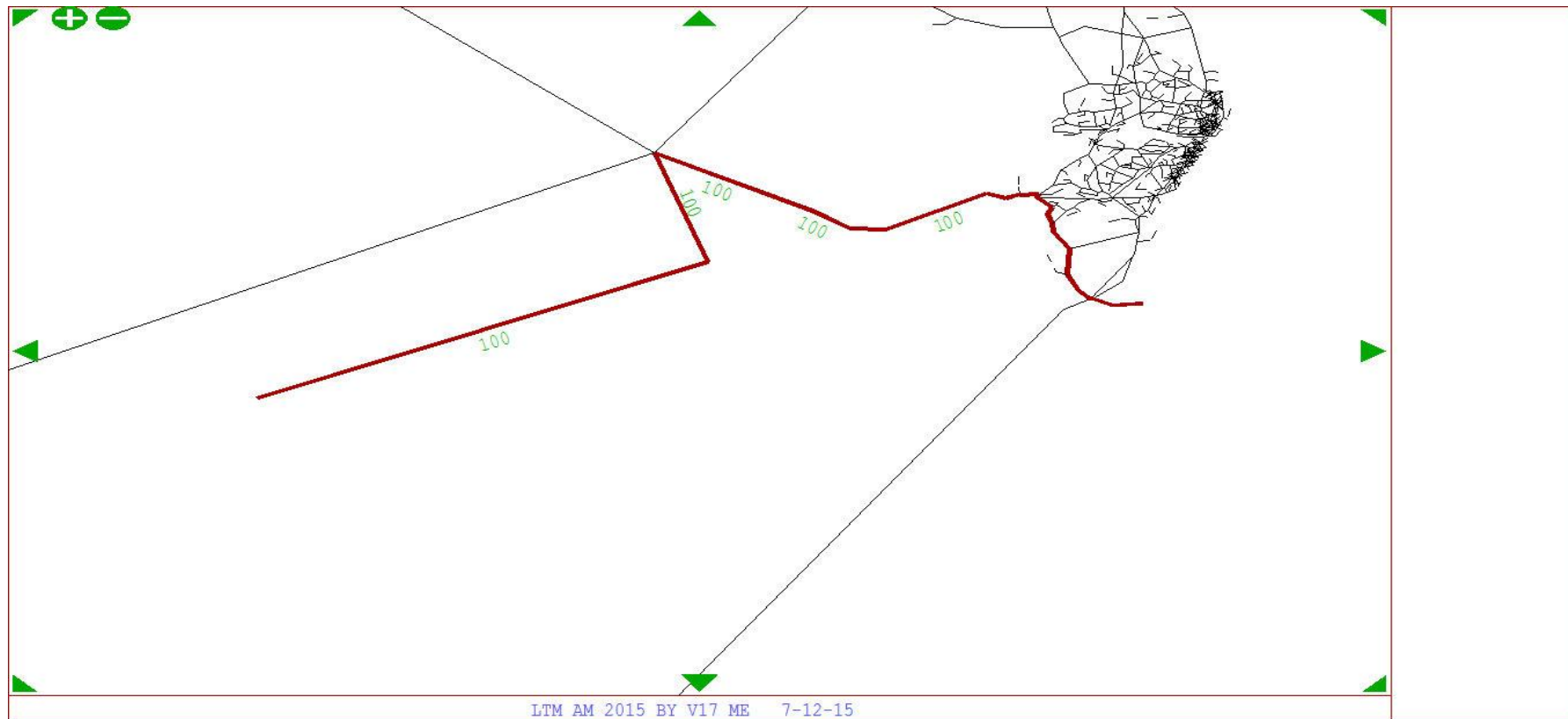


From Zone 149 To Zone 104 - User Class 4



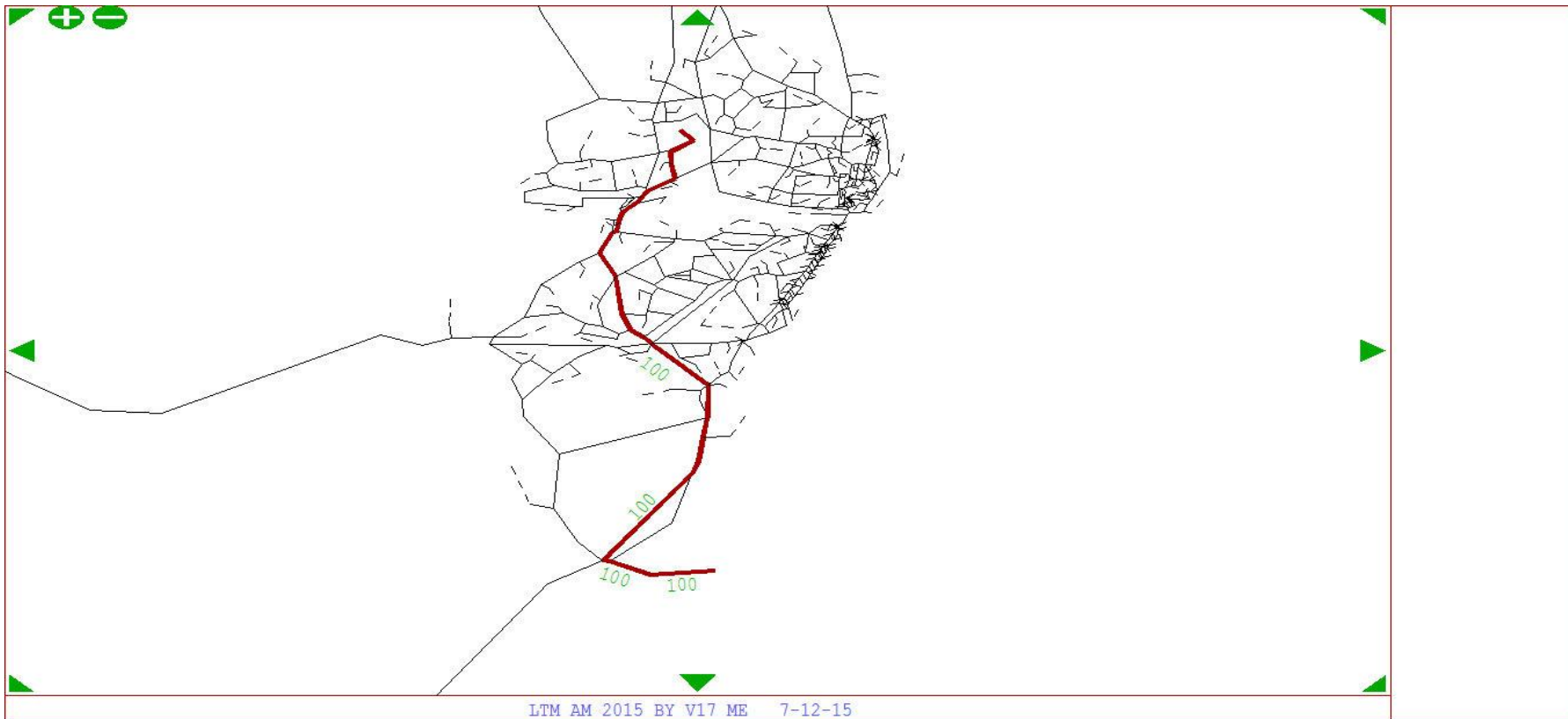
LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 104 - User Class 5

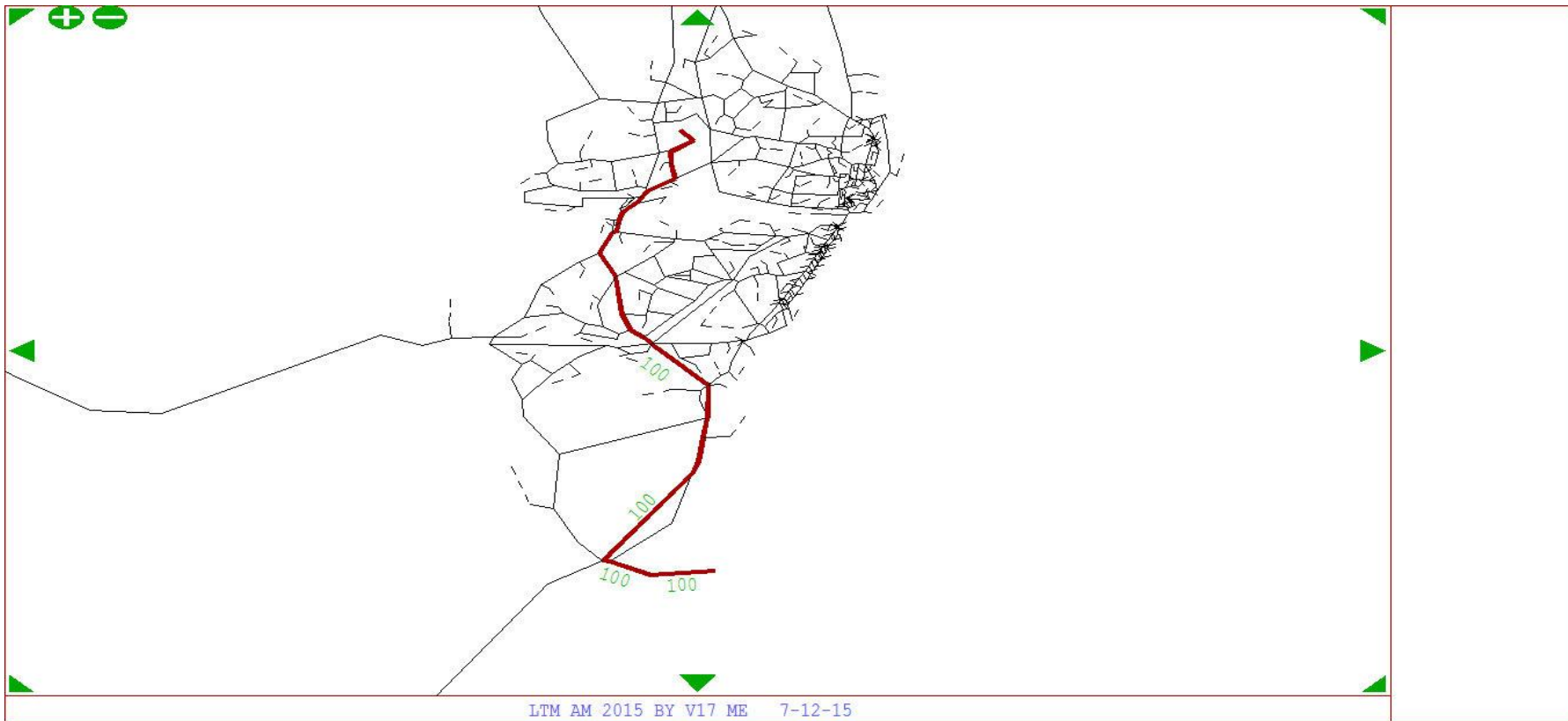


LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 113 - User Class 1

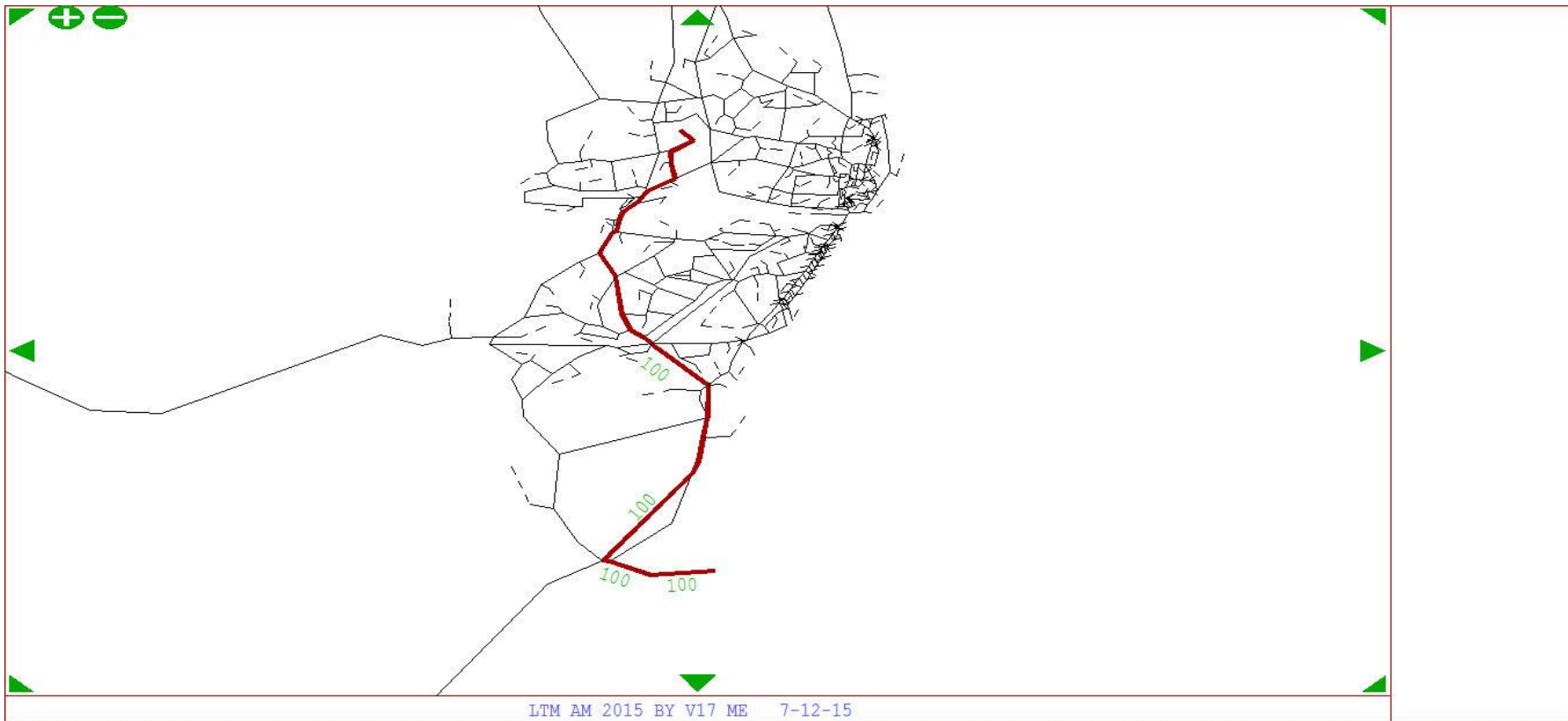


From Zone 149 To Zone 113 - User Class 2

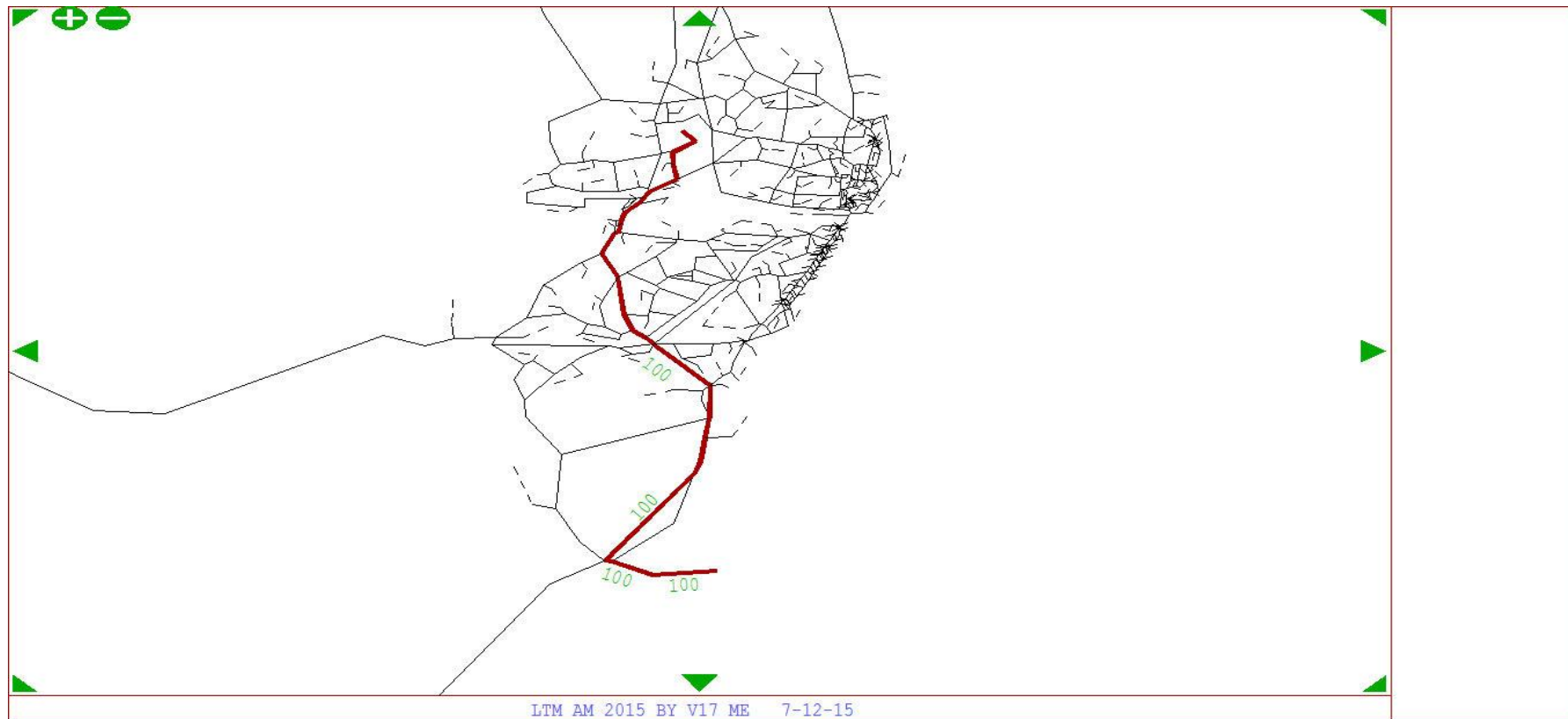


LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 113 - User Class 3

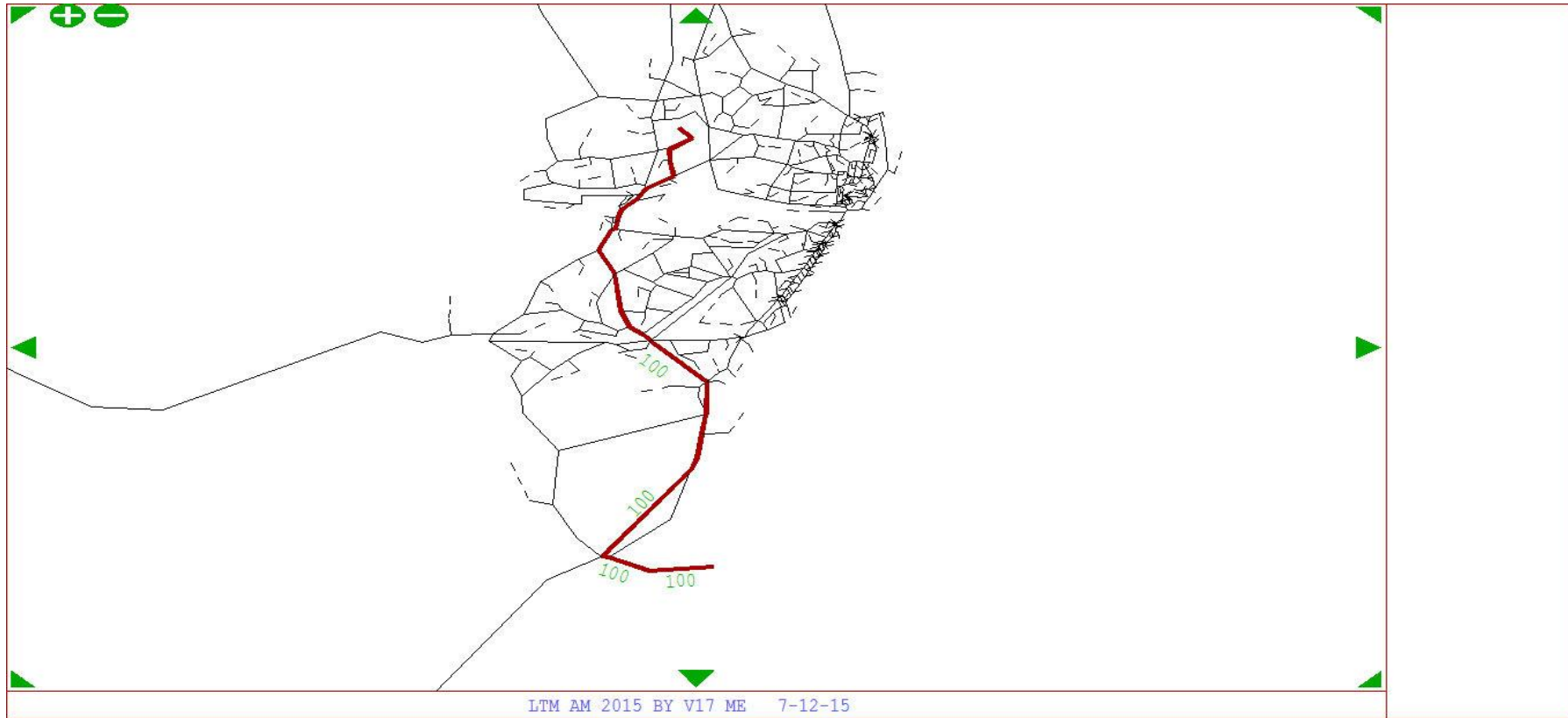


From Zone 149 To Zone 113 - User Class 4

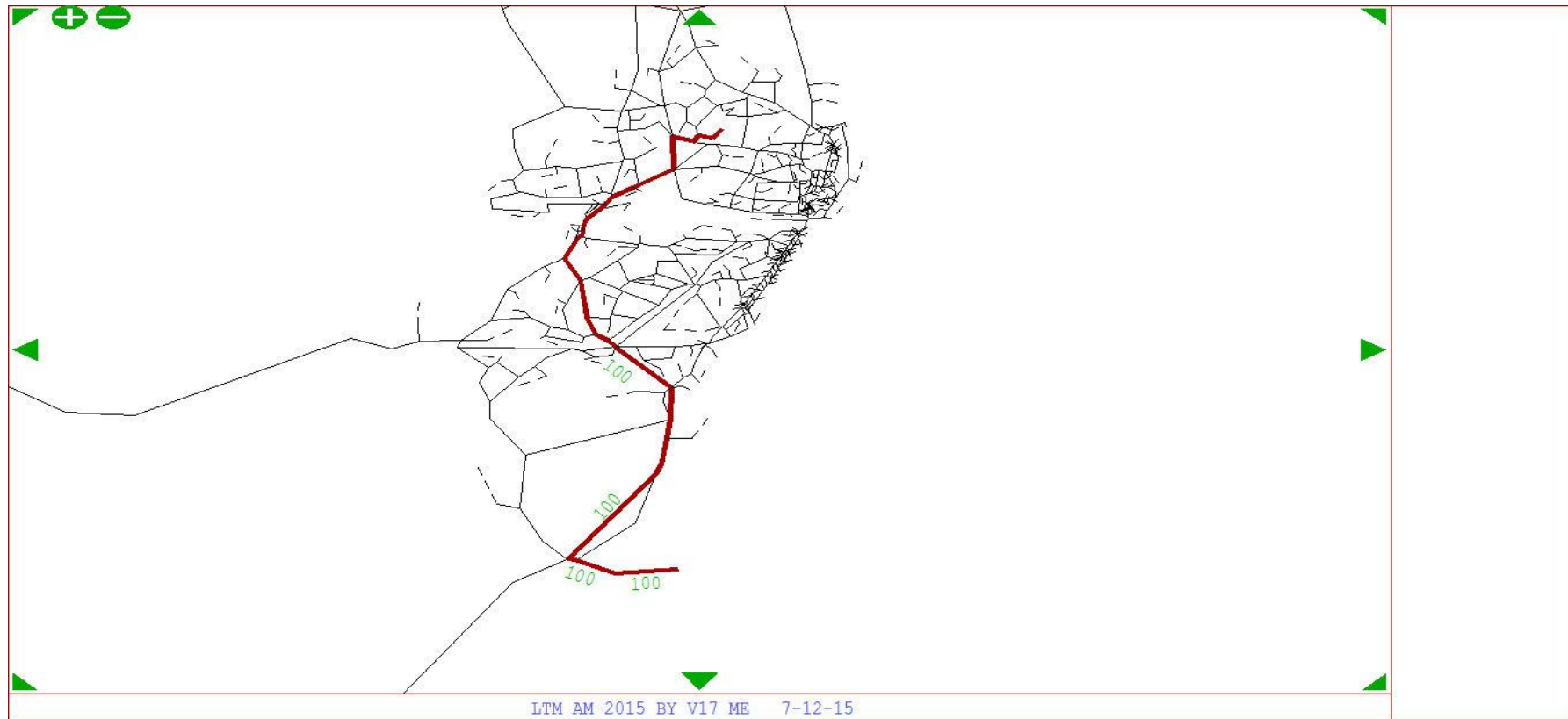


LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 113 - User Class 5

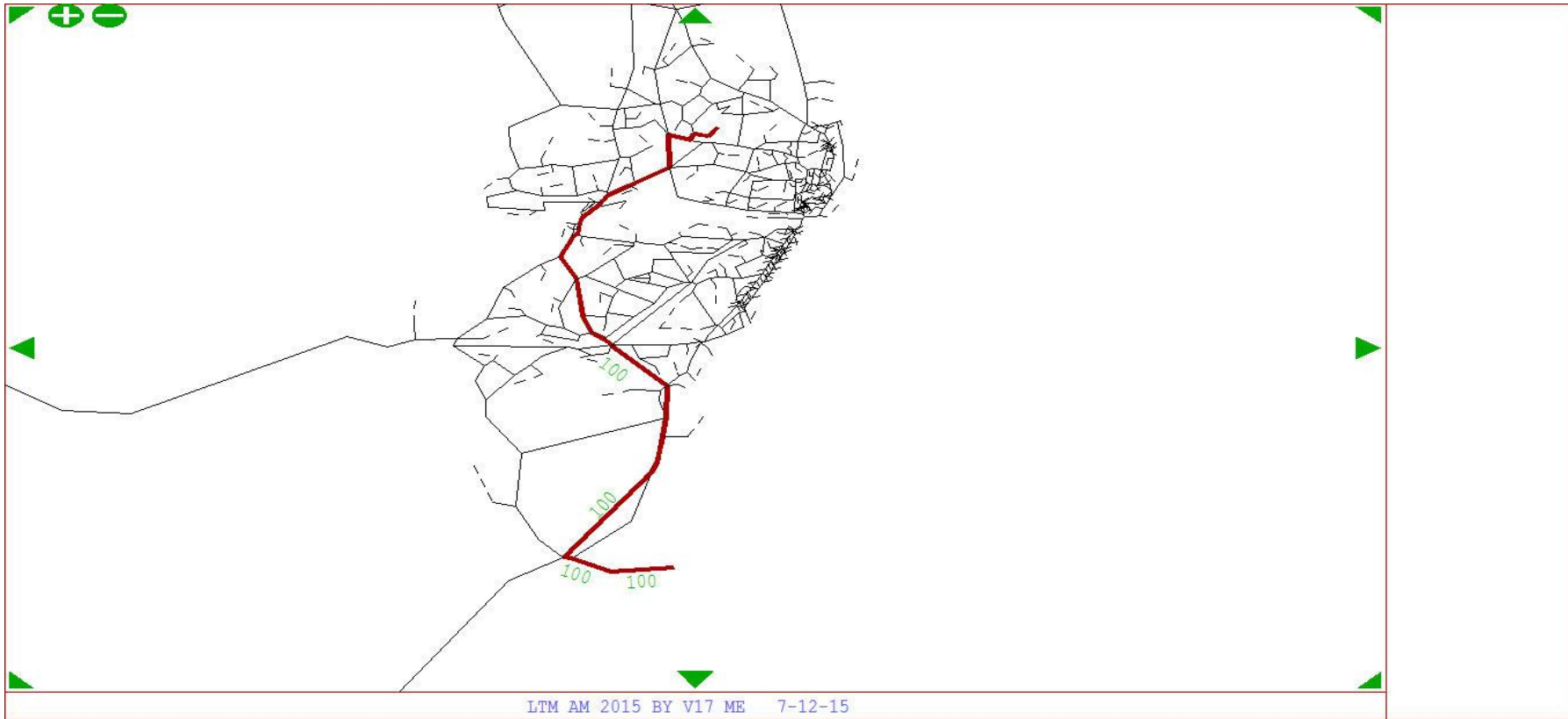


From Zone 149 To Zone 114 - User Class 1



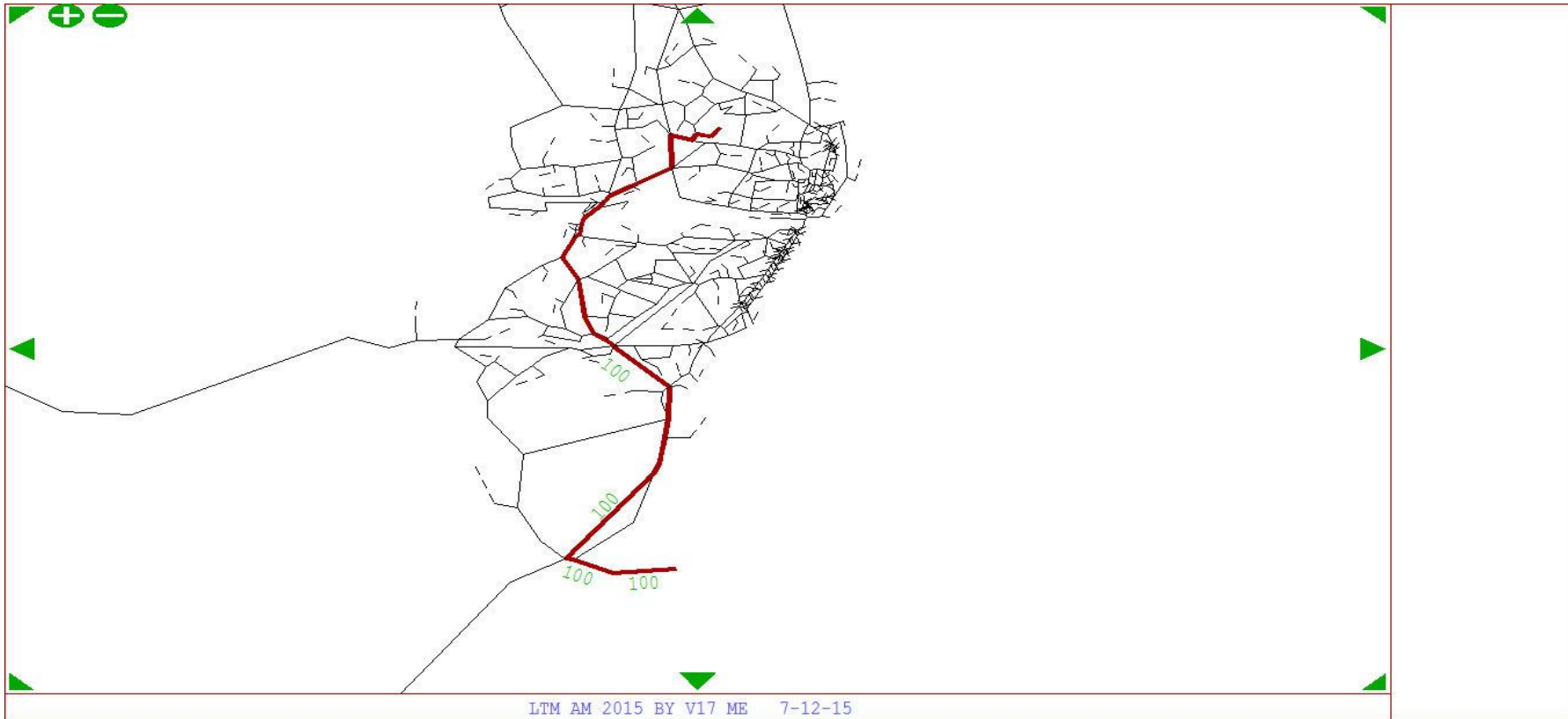
LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 114 - User Class 2

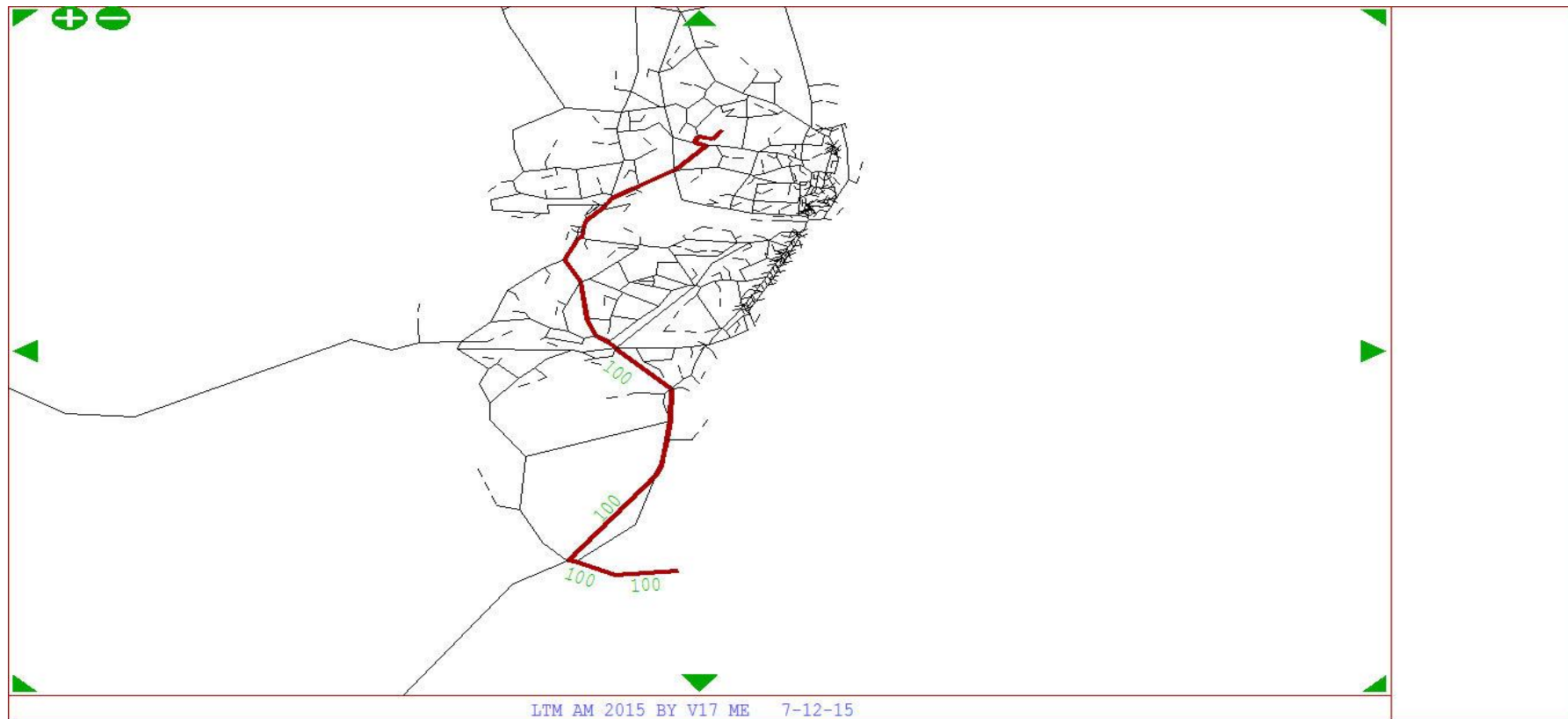


LTM AM 2015 BY V17 ME 7-12-15

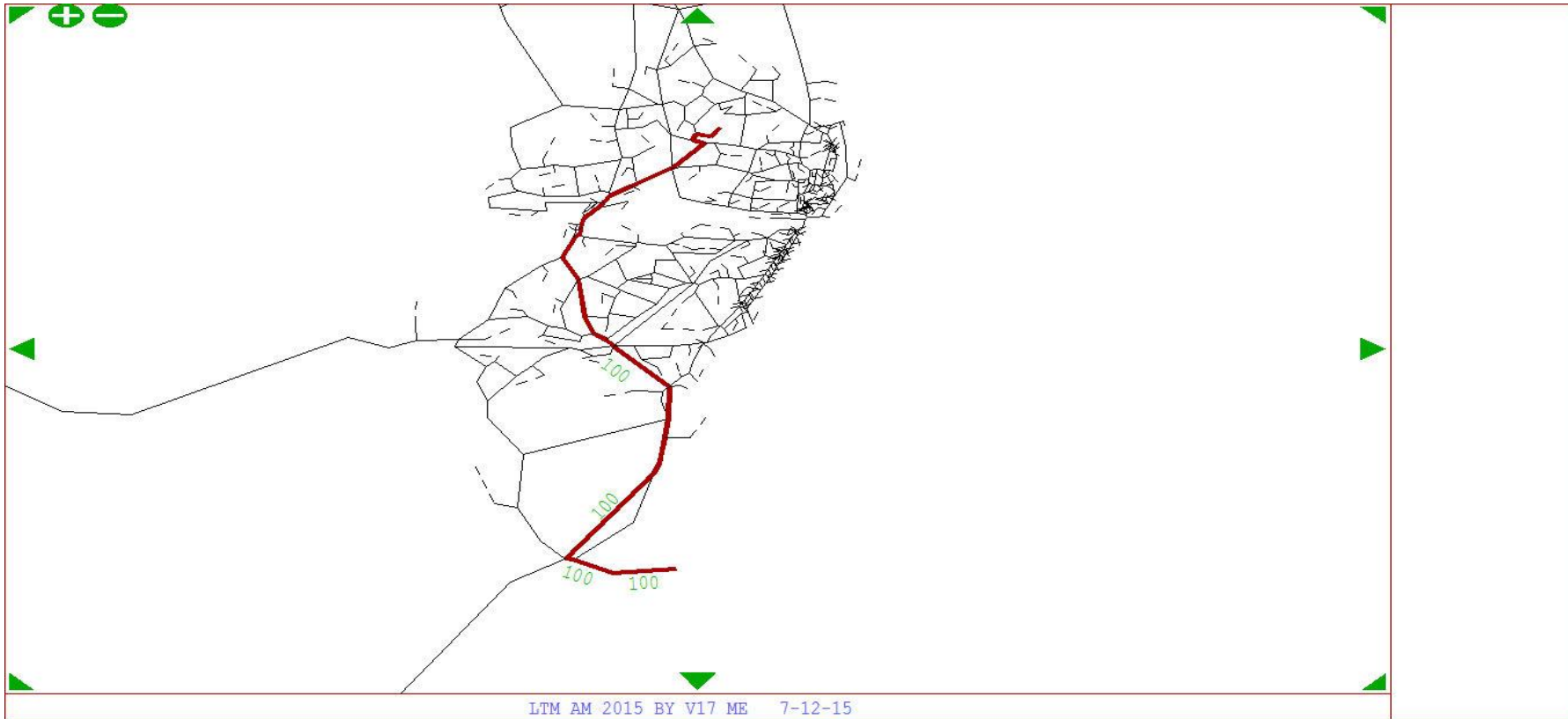
From Zone 149 To Zone 114 - User Class 3



From Zone 149 To Zone 114 - User Class 4

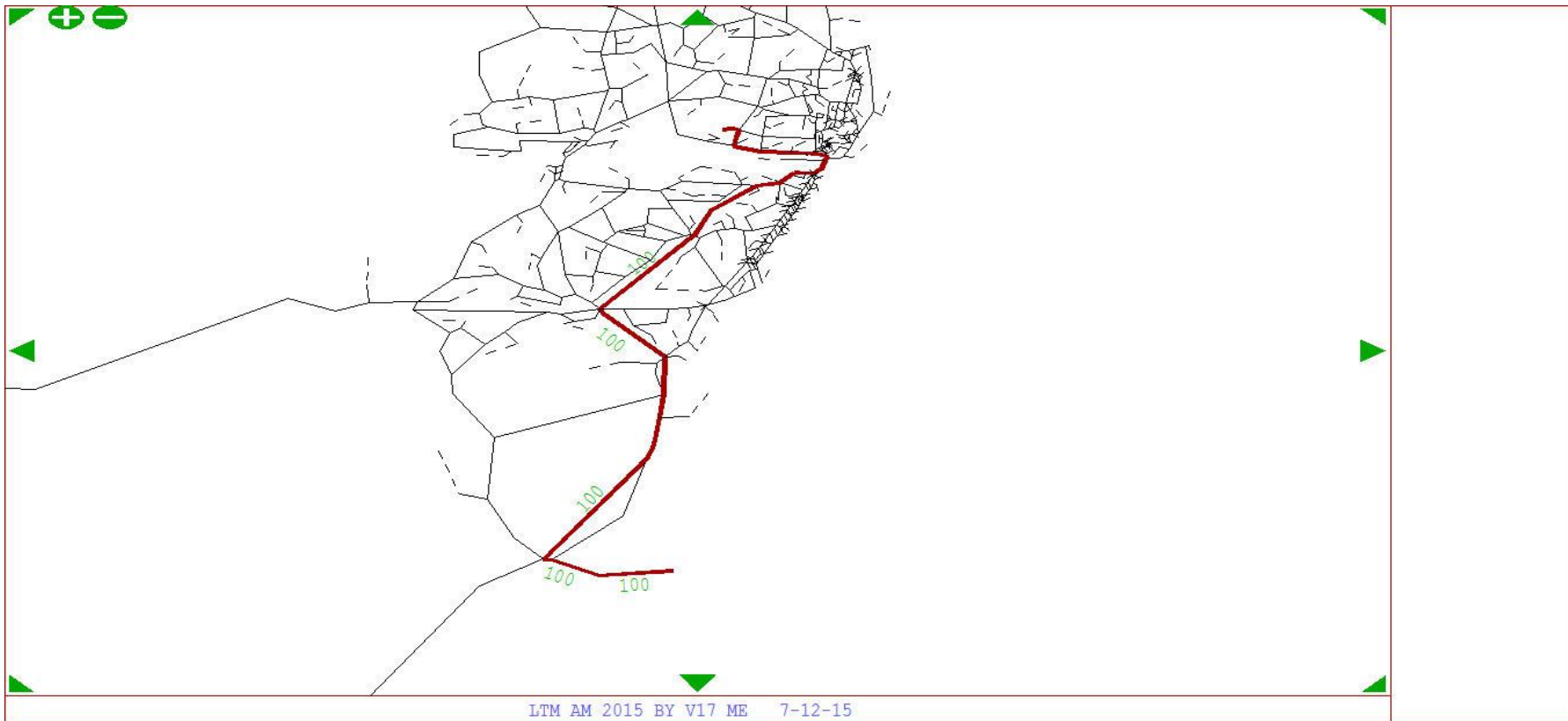


From Zone 149 To Zone 114 - User Class 5

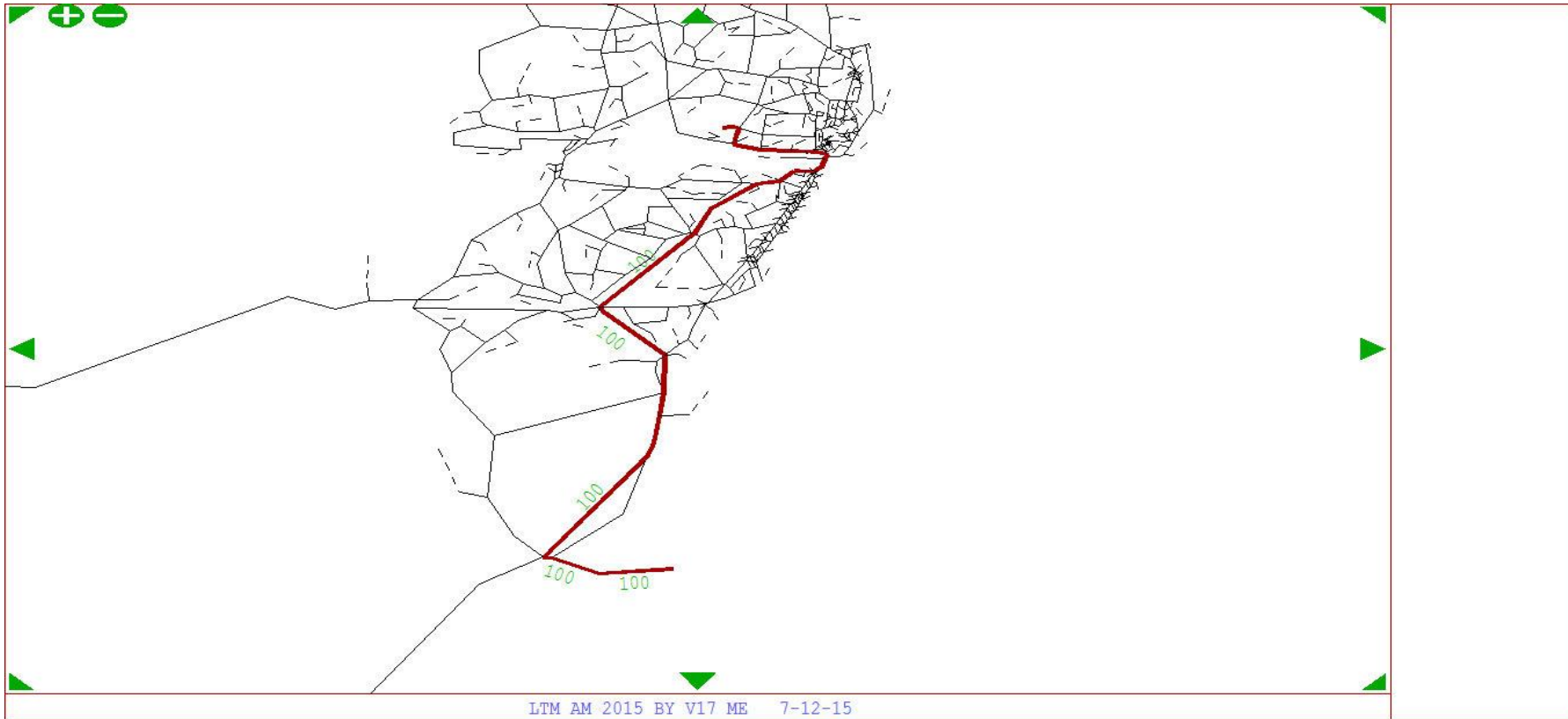


LTM AM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 120 - User Class 1

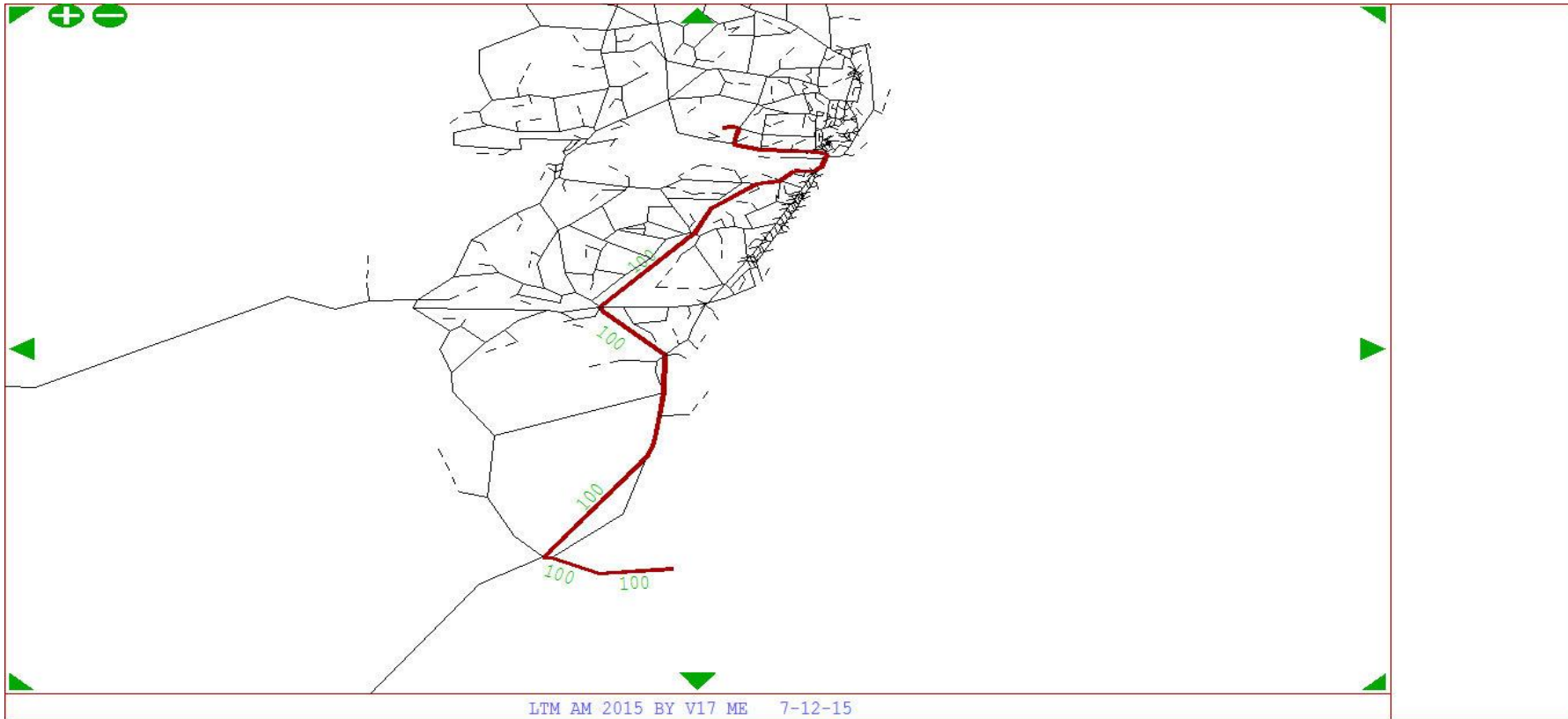


From Zone 149 To Zone 120 - User Class 2

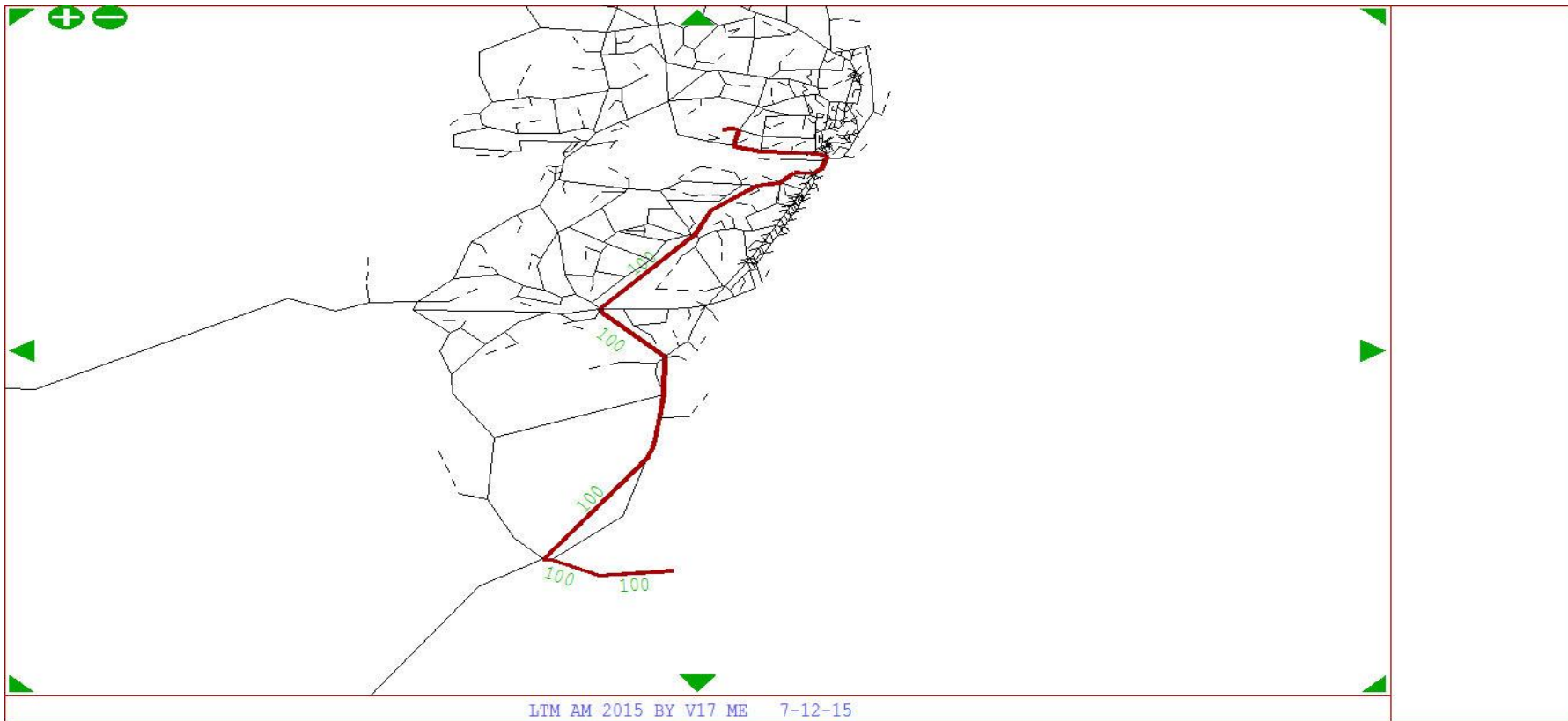


LTM AM 2015 BY V17 ME 7-12-15

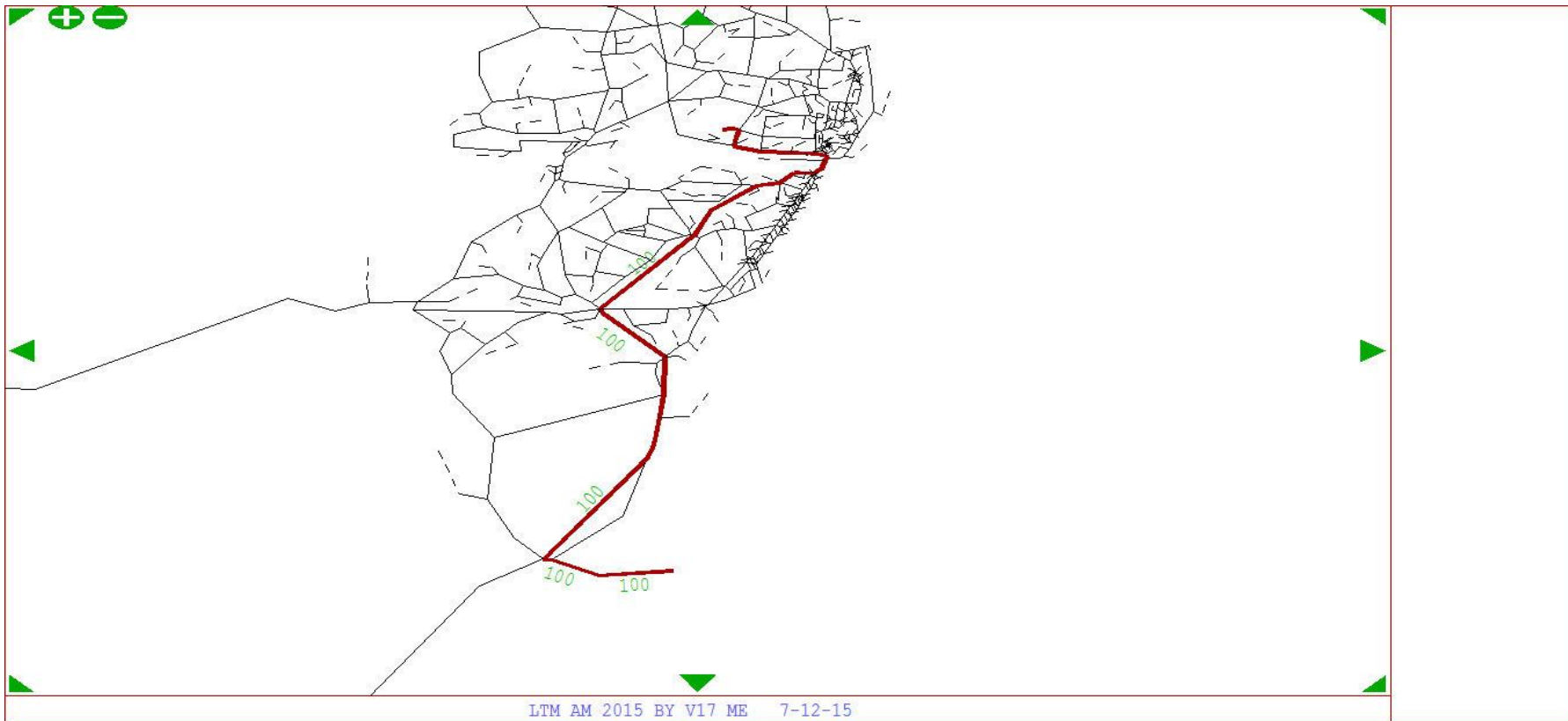
From Zone 149 To Zone 120 - User Class 3



From Zone 149 To Zone 120 - User Class 4

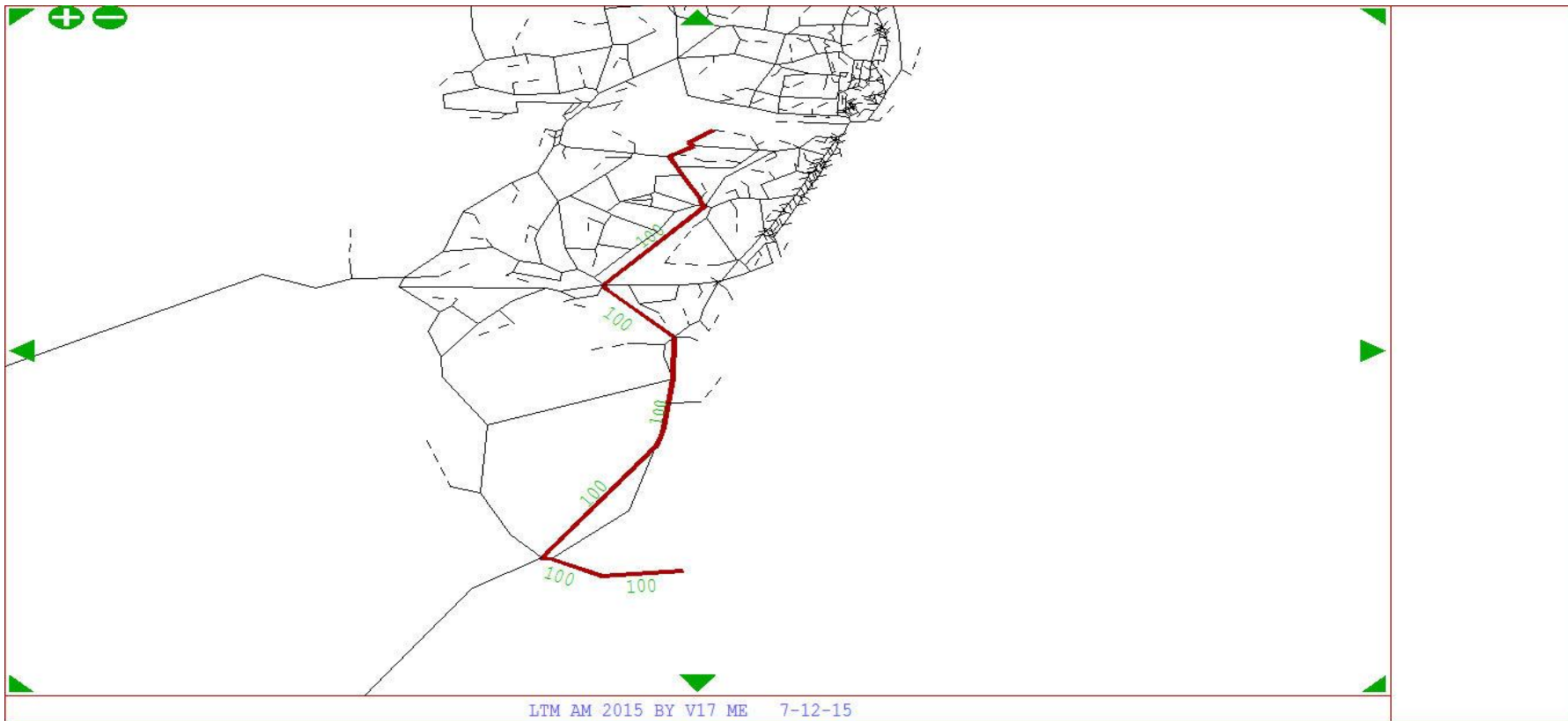


From Zone 149 To Zone 120 - User Class 5

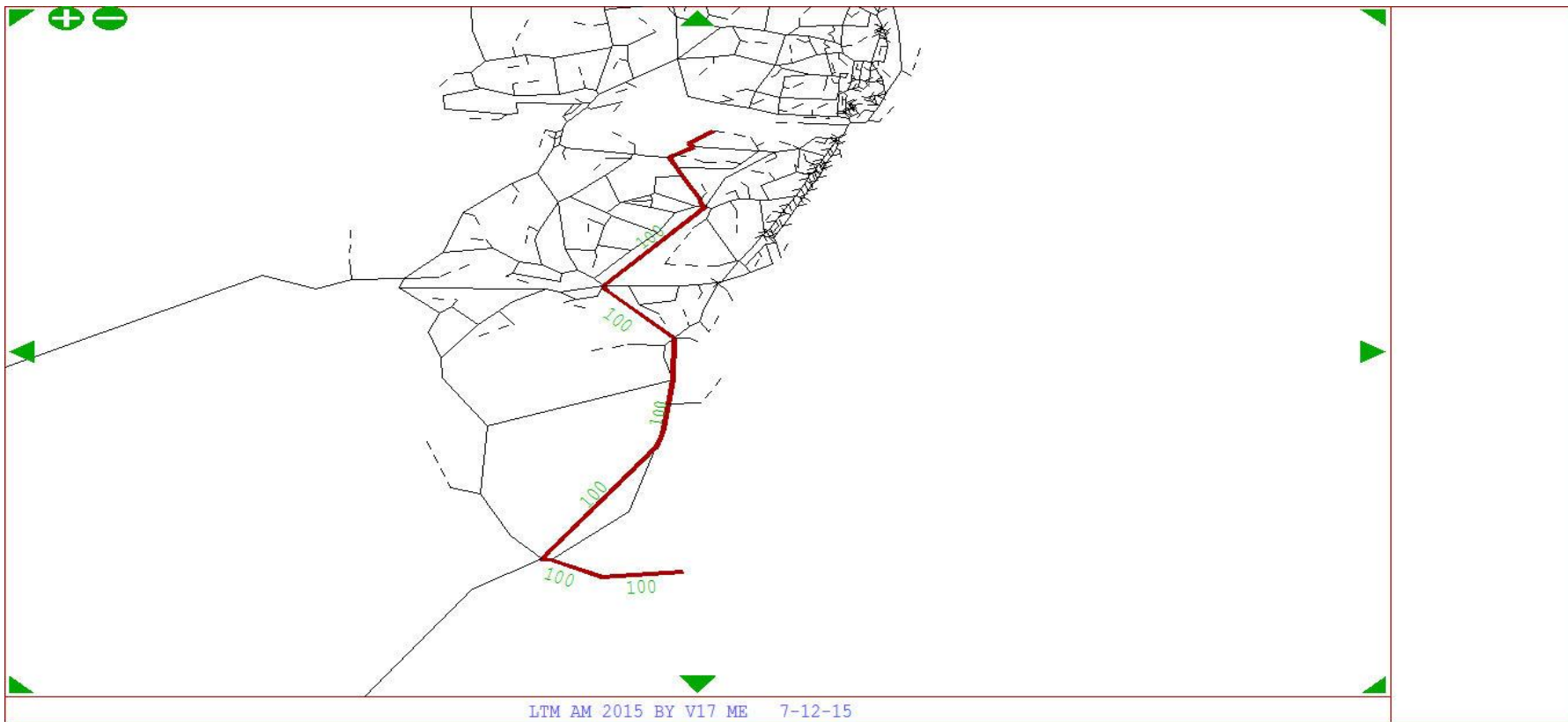


LTM AM 2015 BY V17 ME 7-12-15

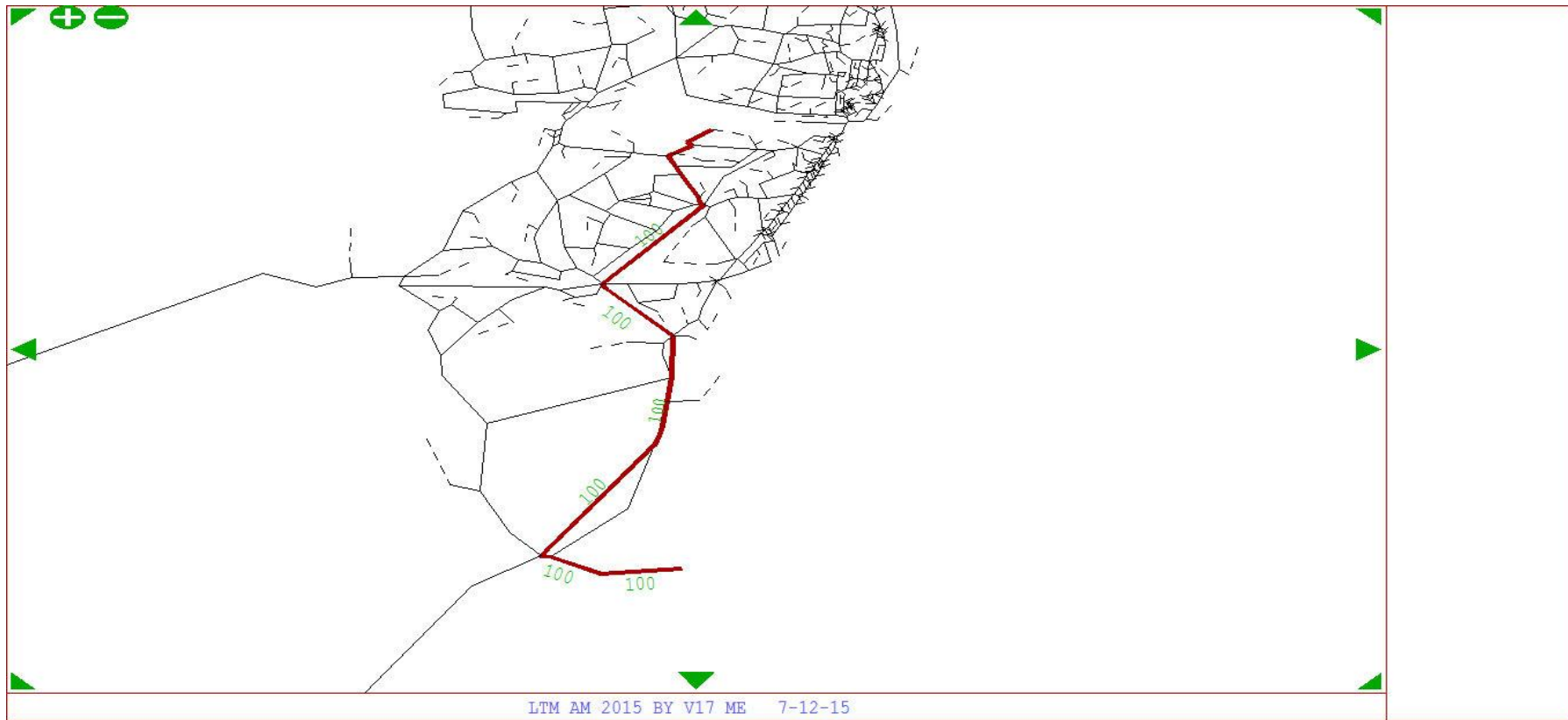
From Zone 149 To Zone 122 - User Class 1



From Zone 149 To Zone 122 - User Class 2

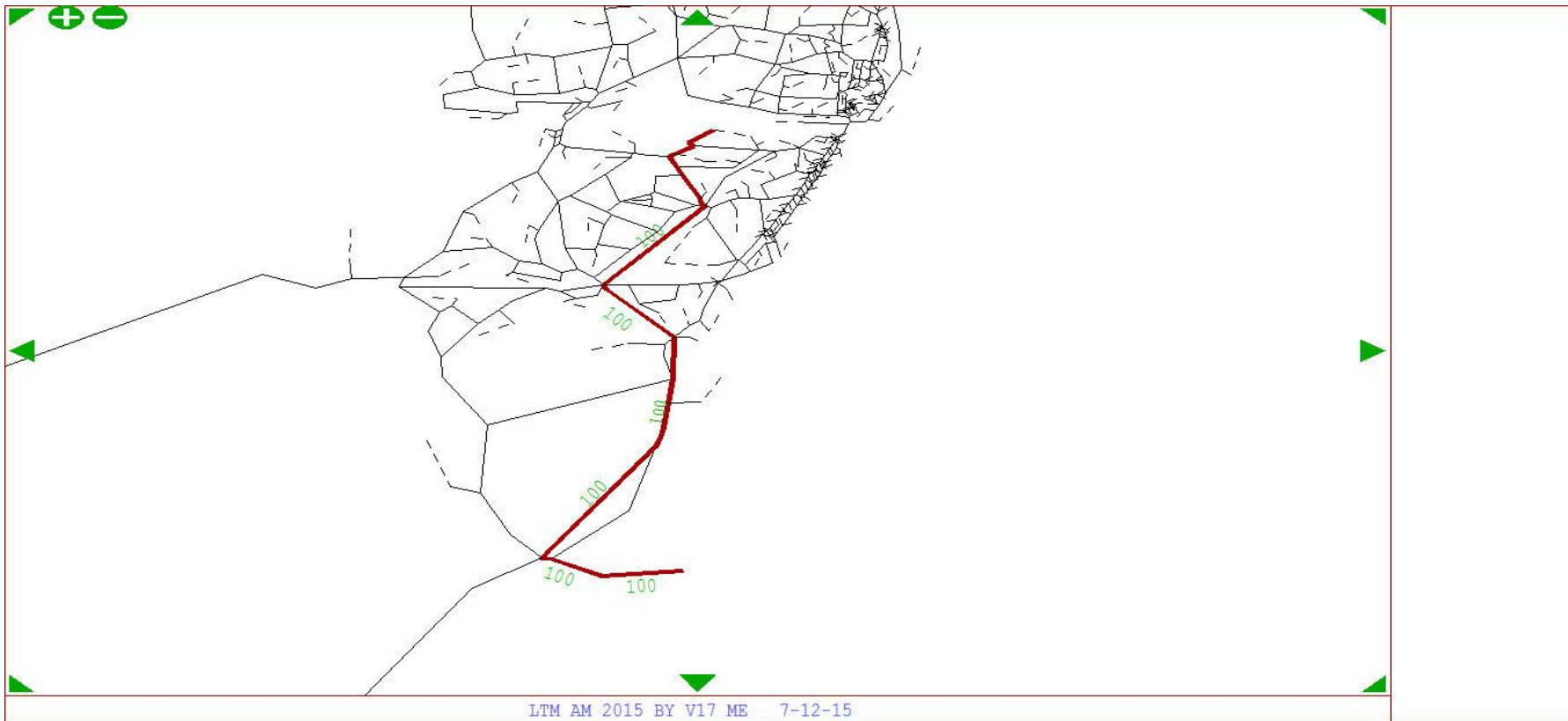


From Zone 149 To Zone 122 - User Class 3

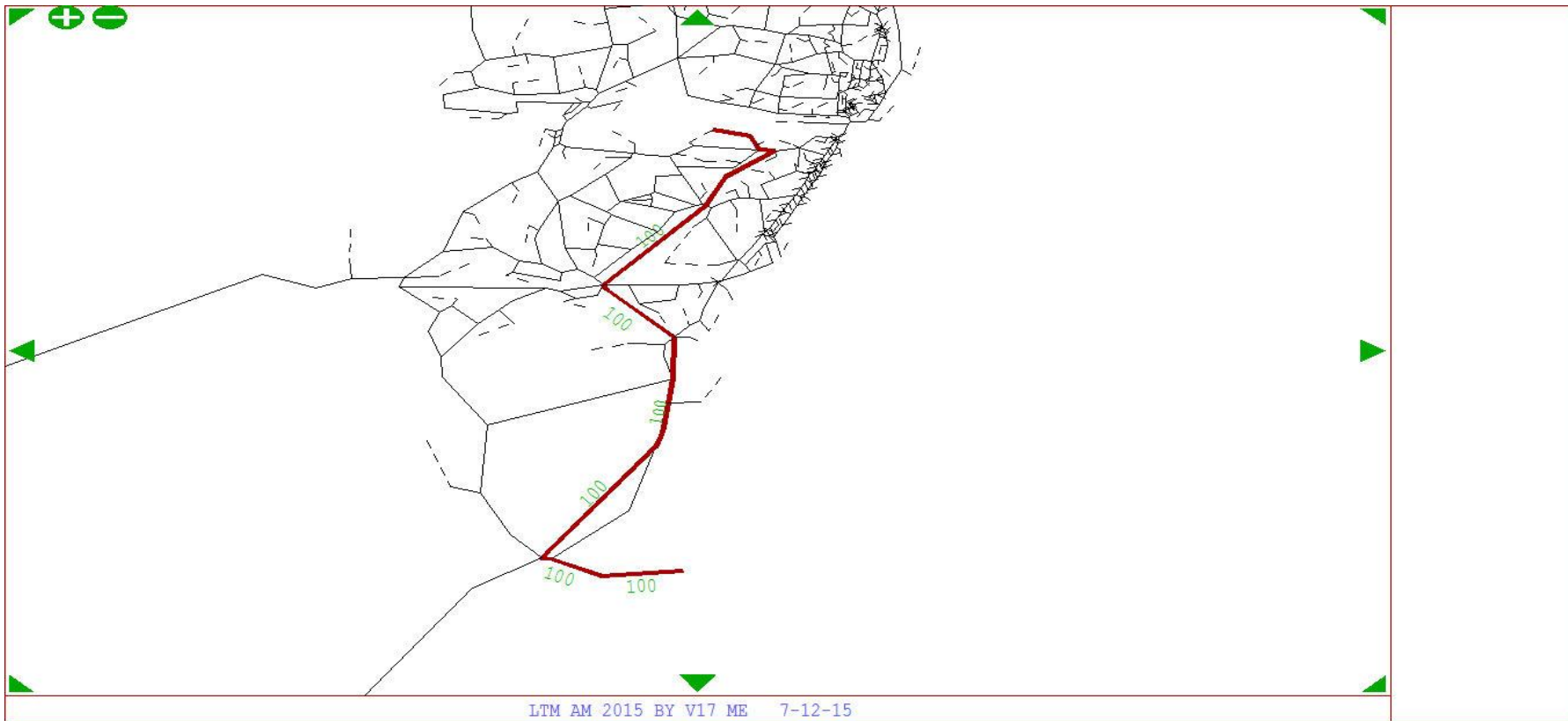


LTM AM 2015 BY V17 ME 7-12-15

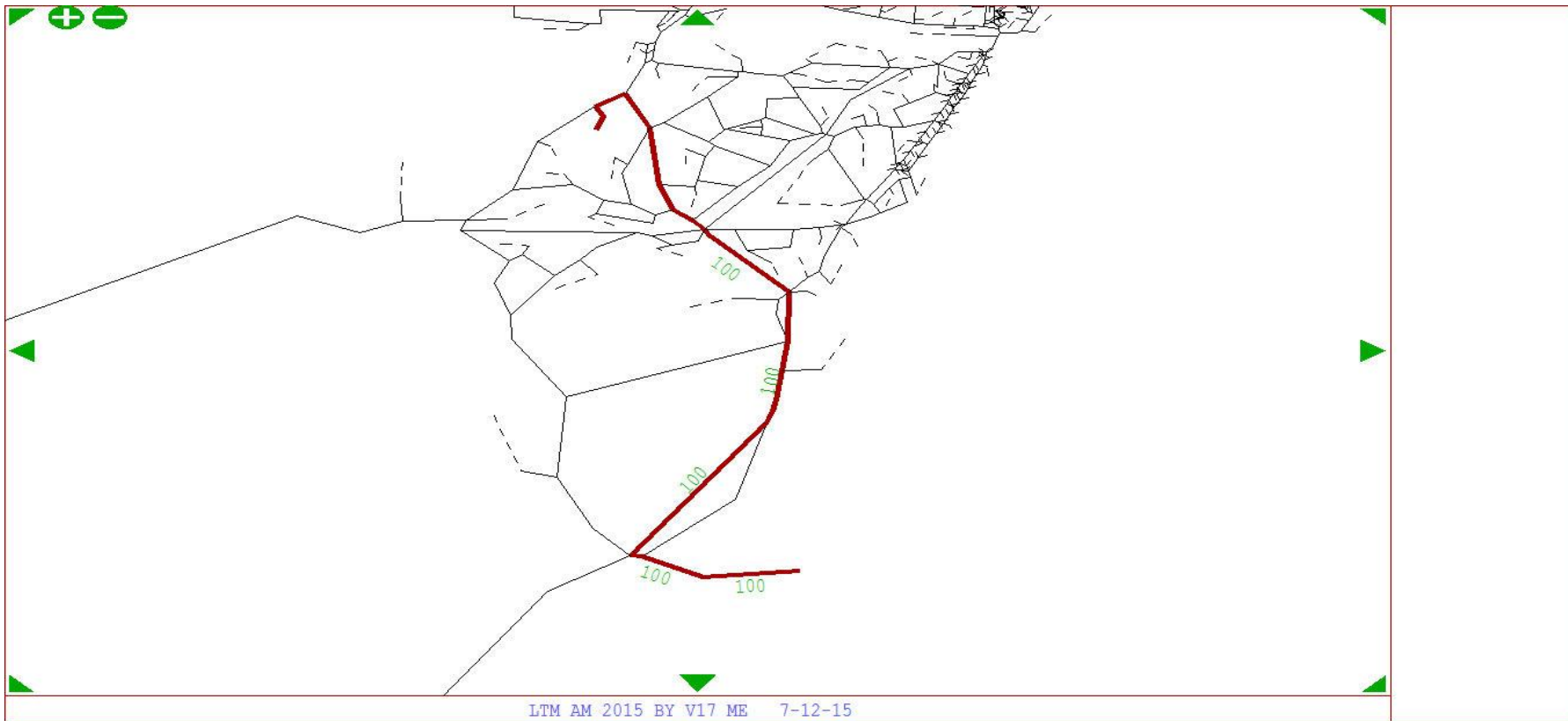
From Zone 149 To Zone 122 - User Class 4



From Zone 149 To Zone 122 - User Class 5

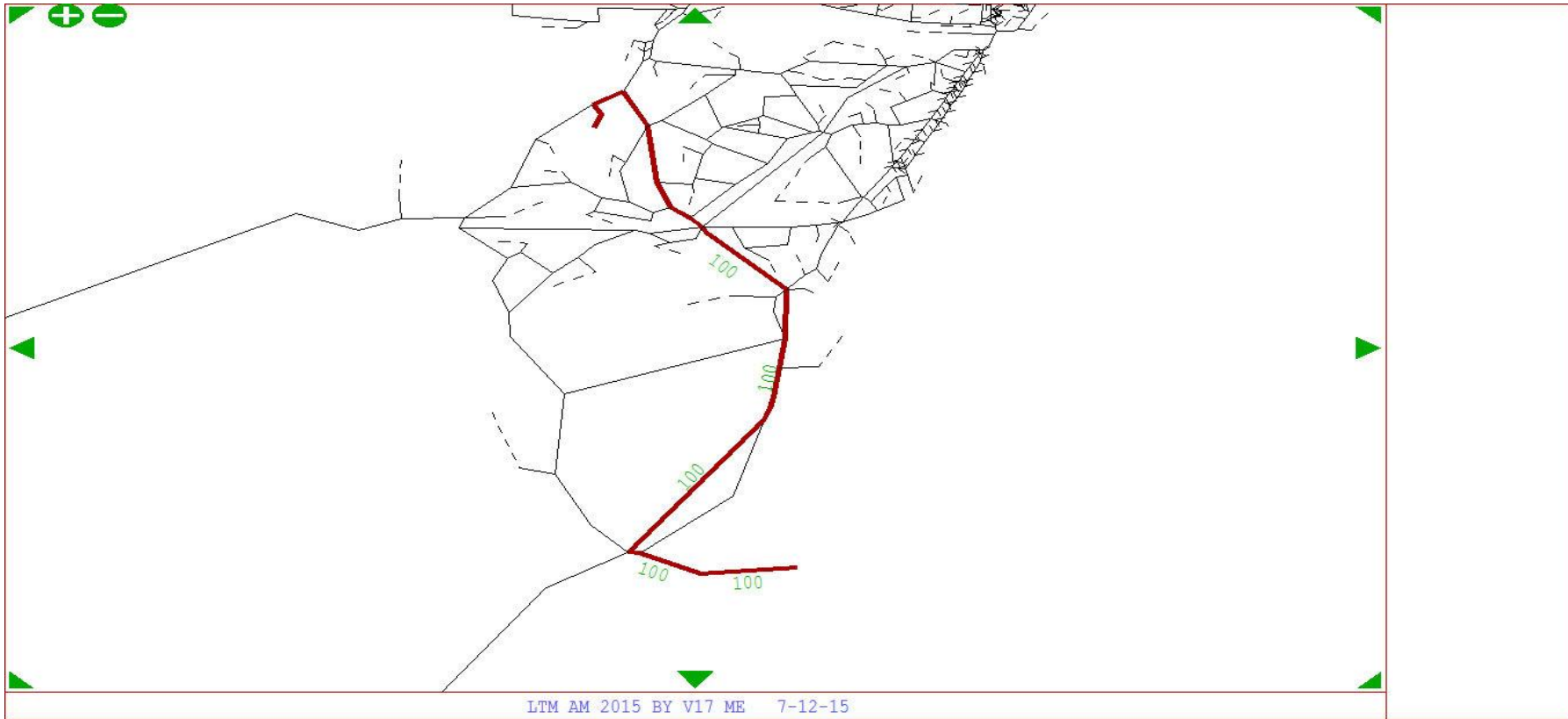


From Zone 149 To Zone 128 - User Class 1

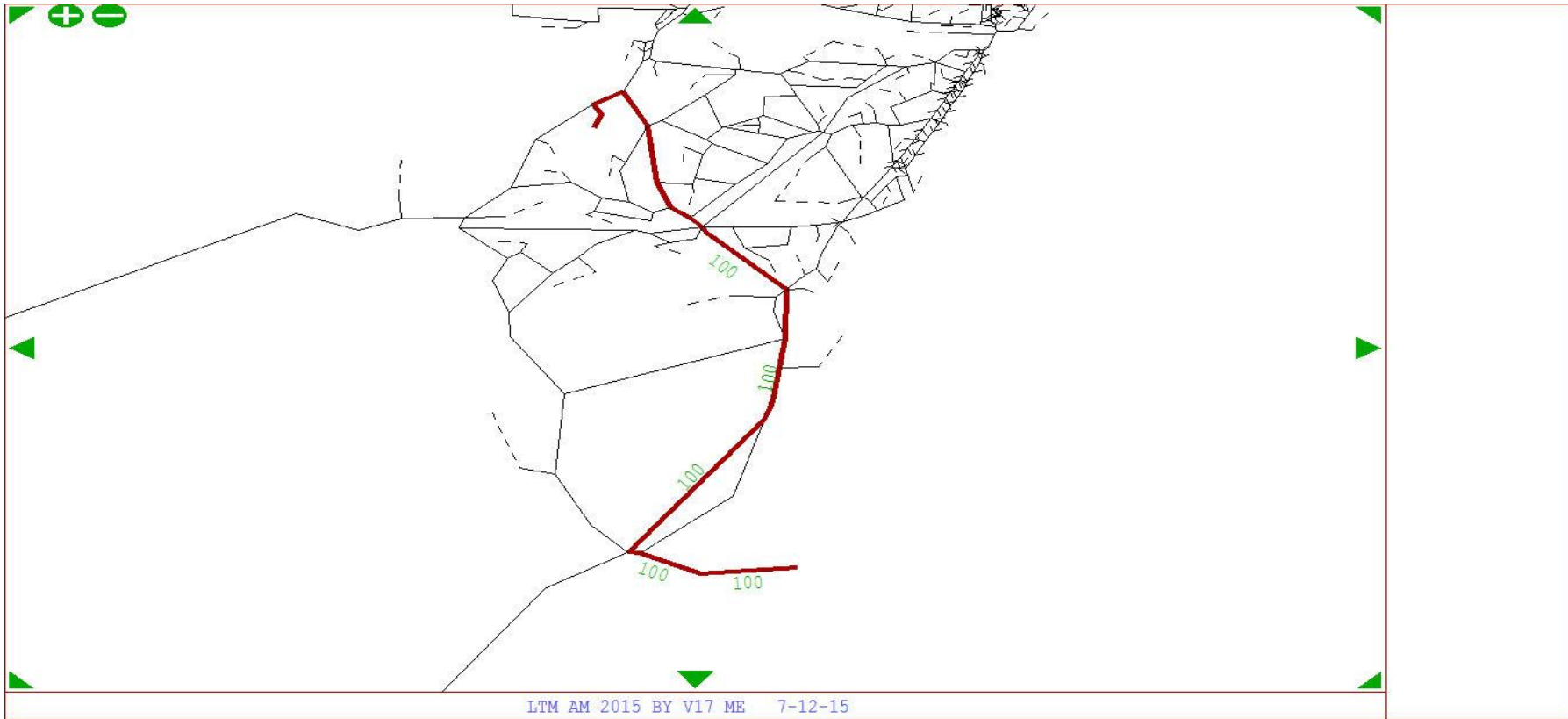


LTM AM 2015 BY V17 ME 7-12-15

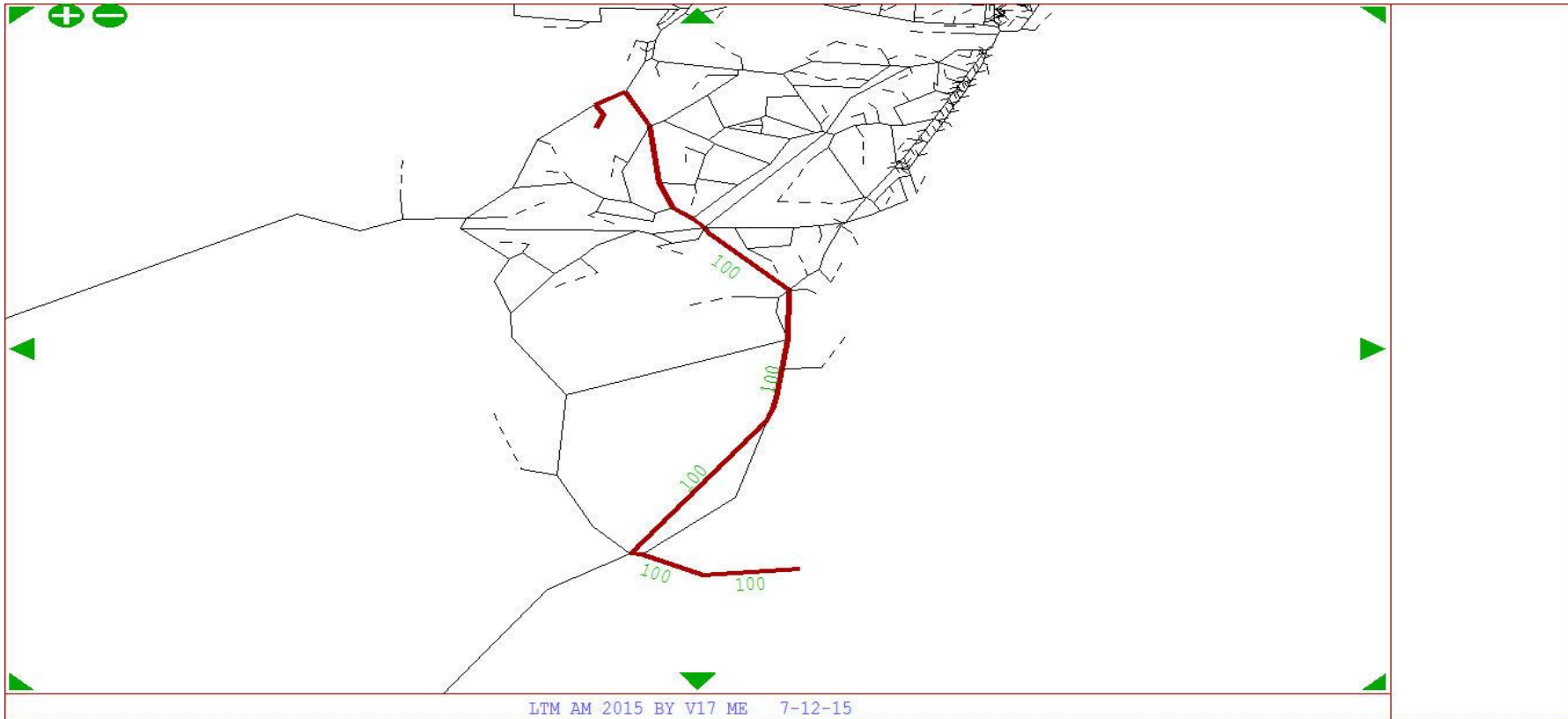
From Zone 149 To Zone 128 - User Class 2



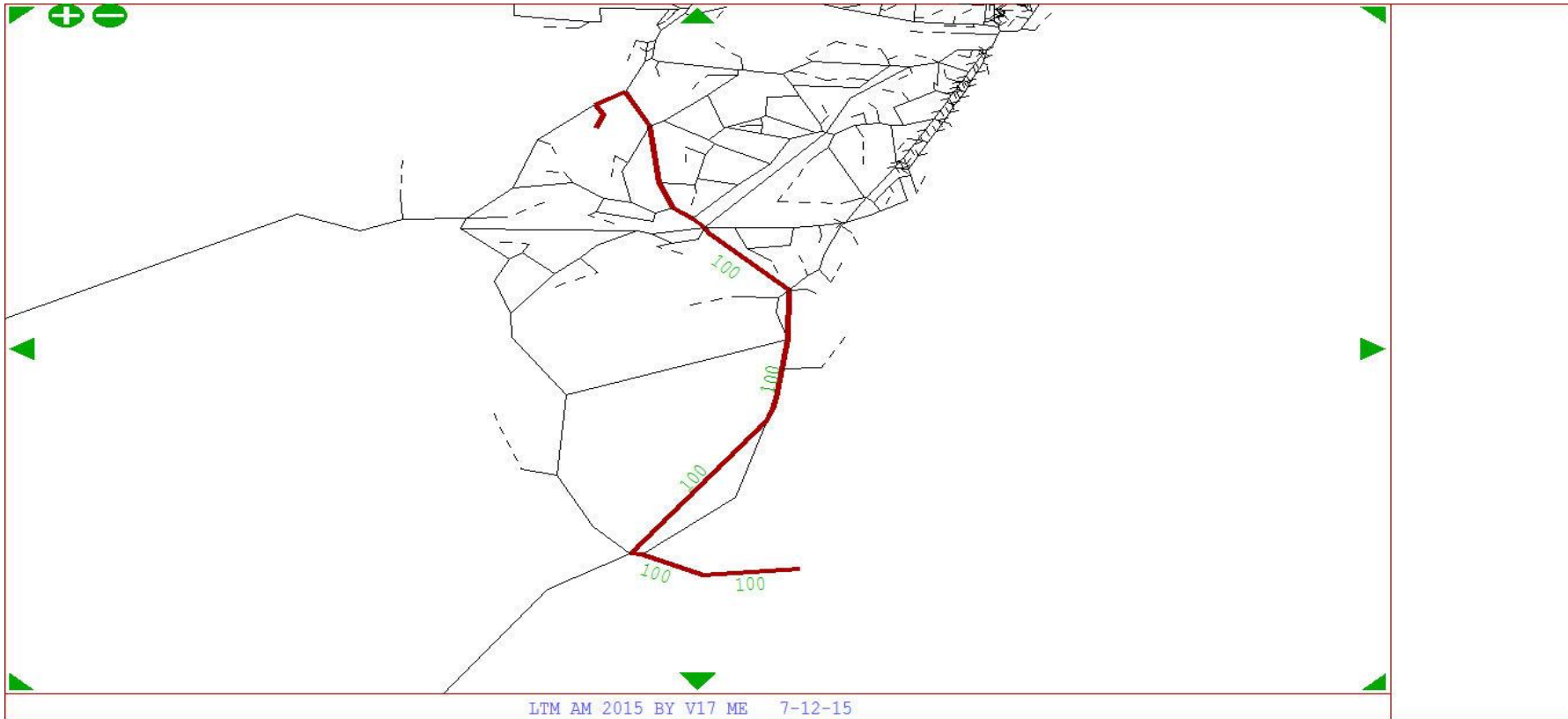
From Zone 149 To Zone 128 - User Class 3



From Zone 149 To Zone 128 - User Class 4

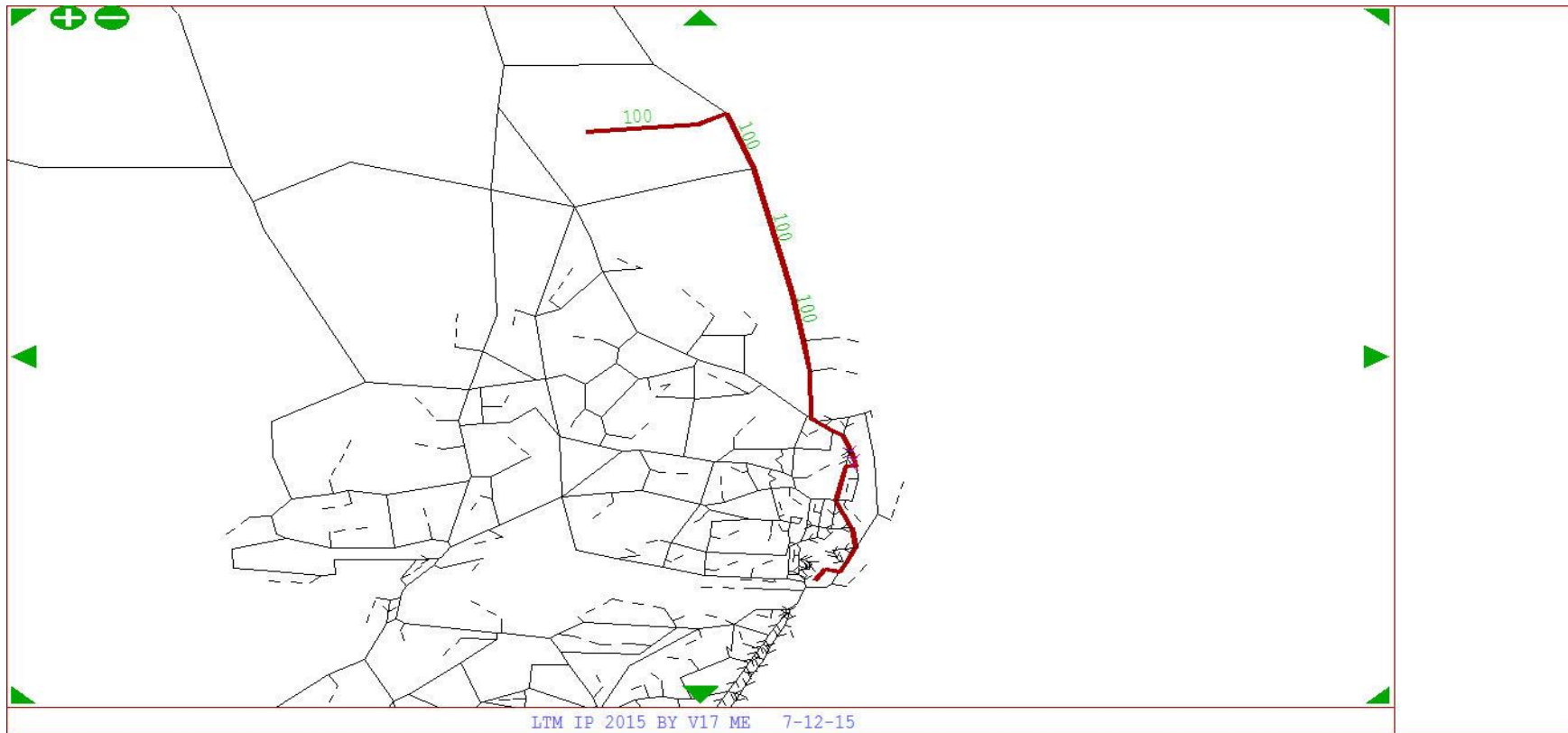


From Zone 149 To Zone 128 - User Class 5

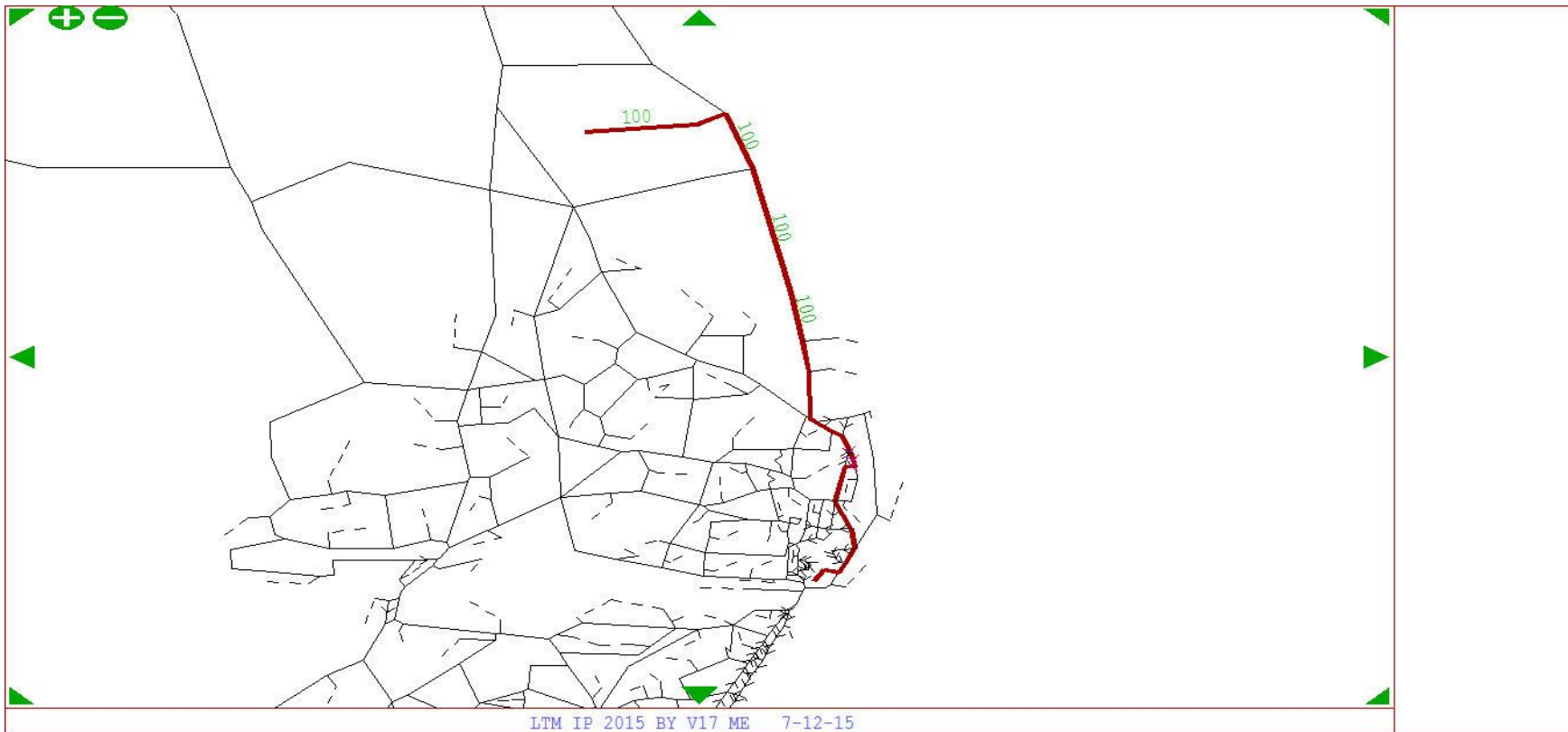


Lowestoft: OD Tree Plots - Interpeak

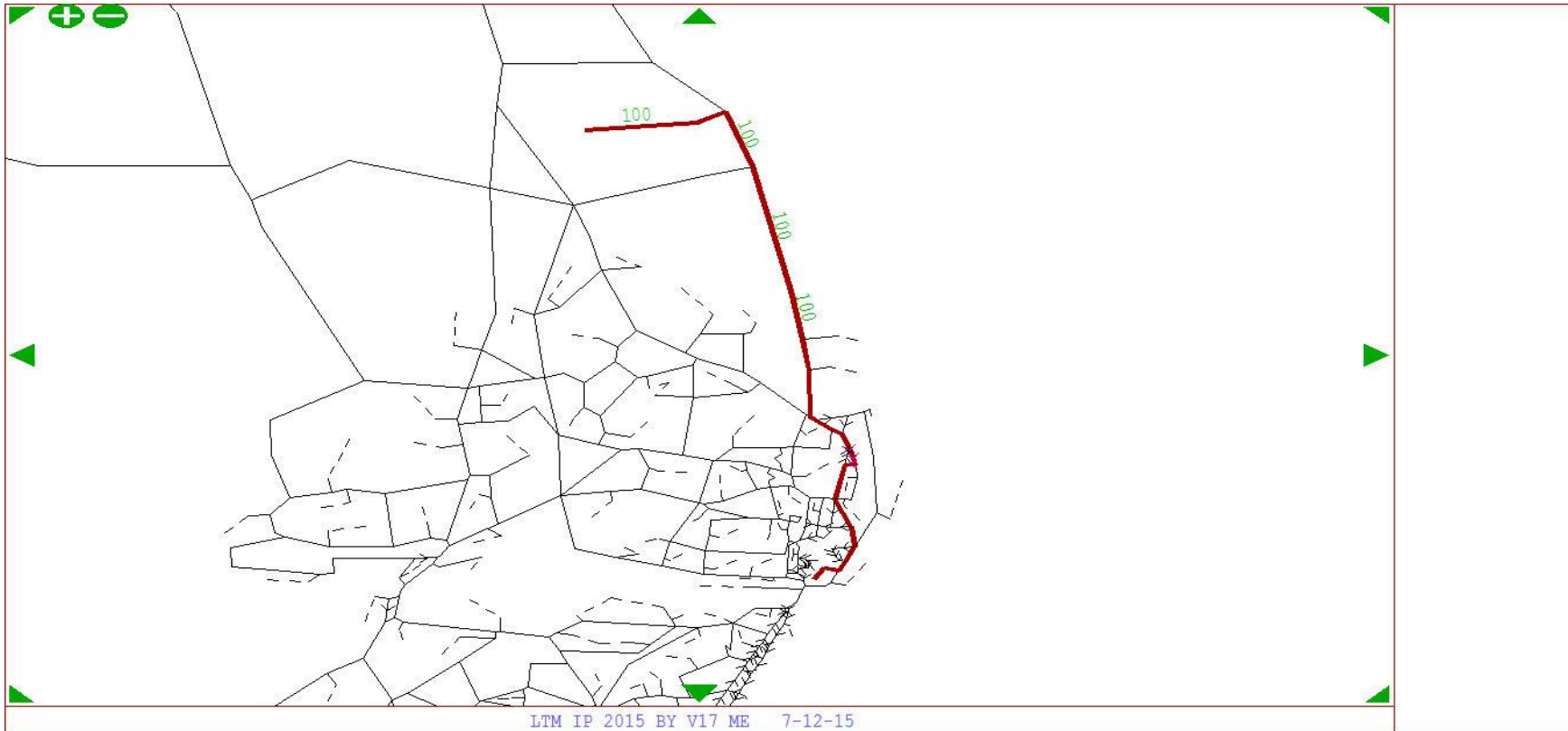
From Zone 102 To Zone 119 - User Class 1



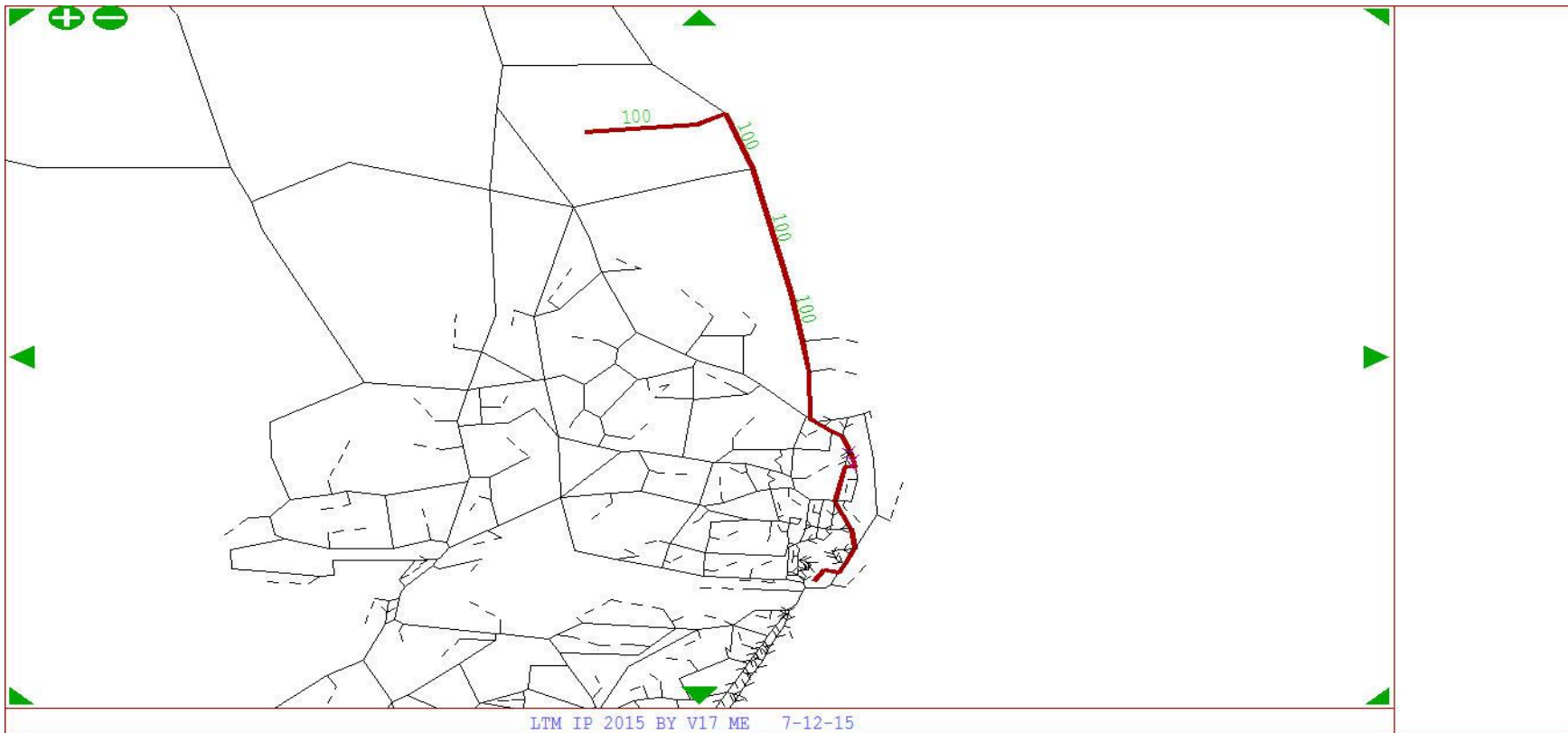
From Zone 102 To Zone 119 - User Class 2



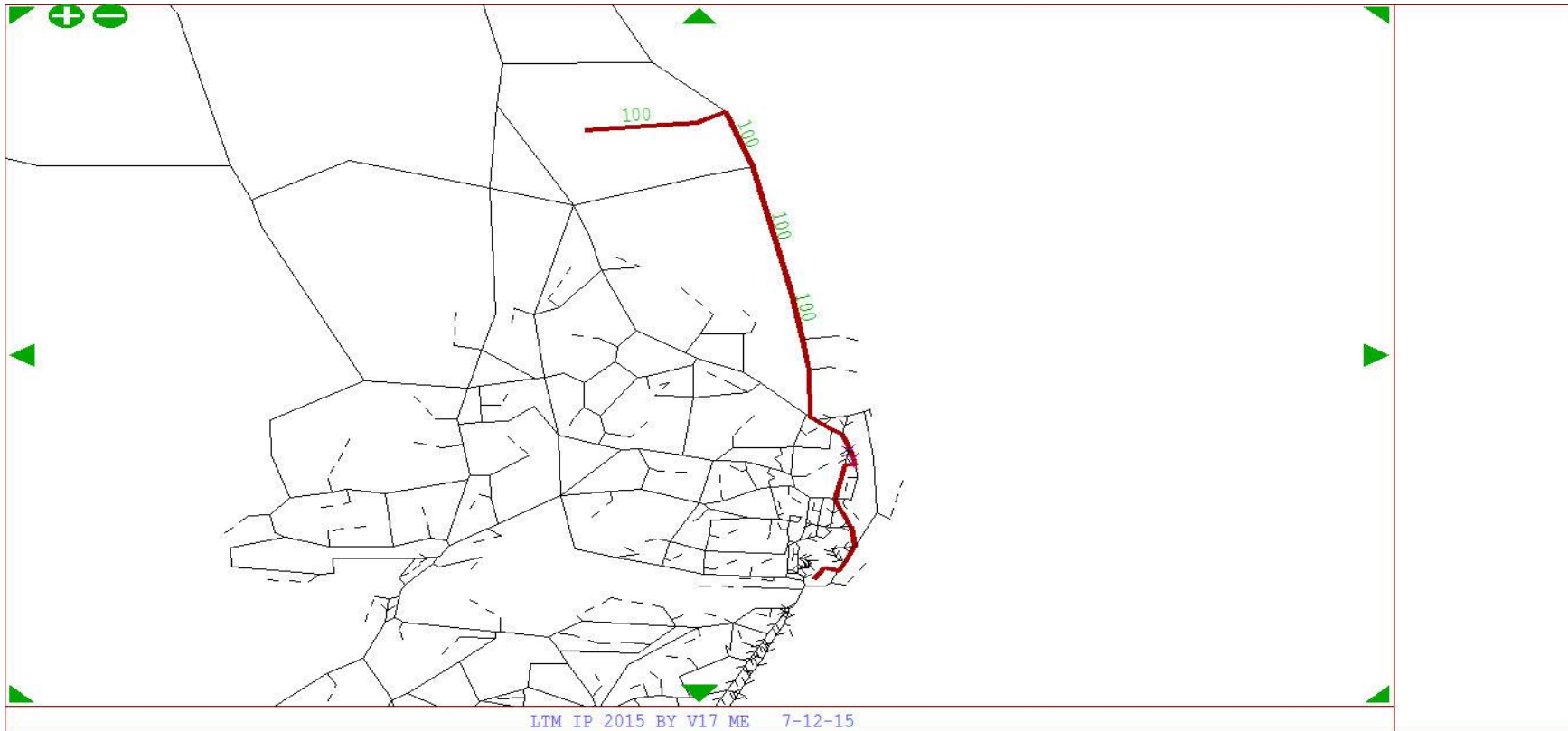
From Zone 102 To Zone 119 - User Class 3



From Zone 102 To Zone 119 - User Class 4

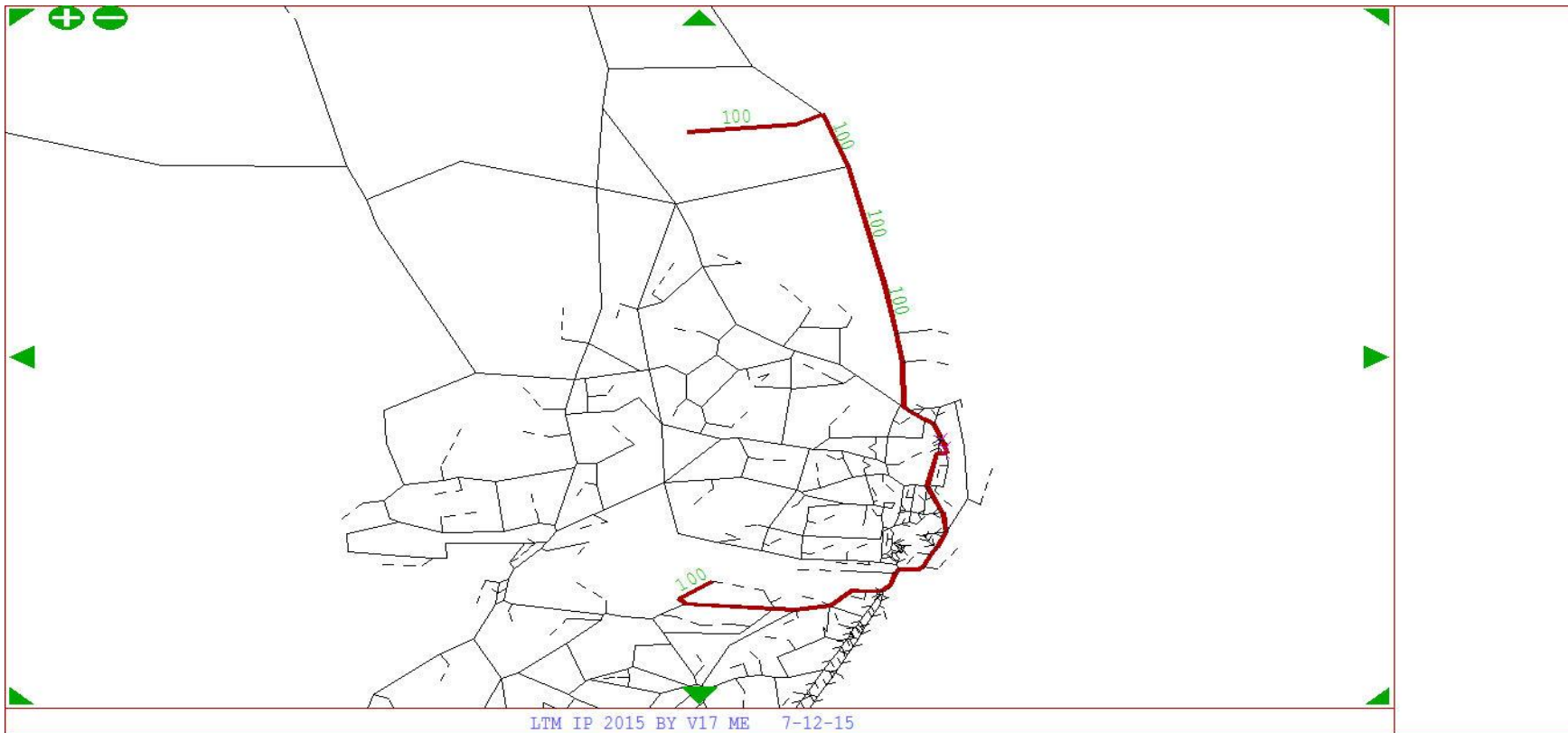


From Zone 102 To Zone 119 - User Class 5

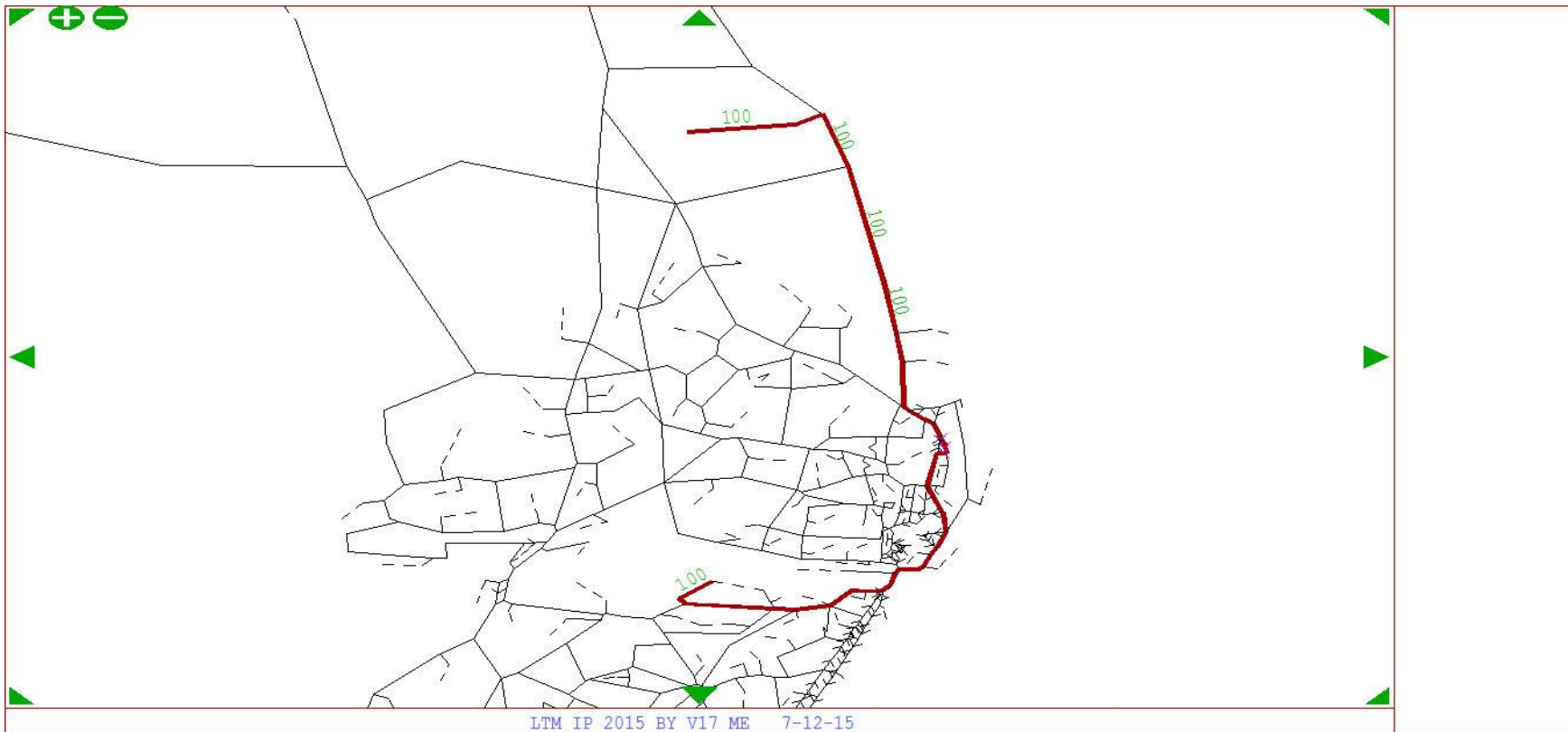


LTM IP 2015 BY V17 ME 7-12-15

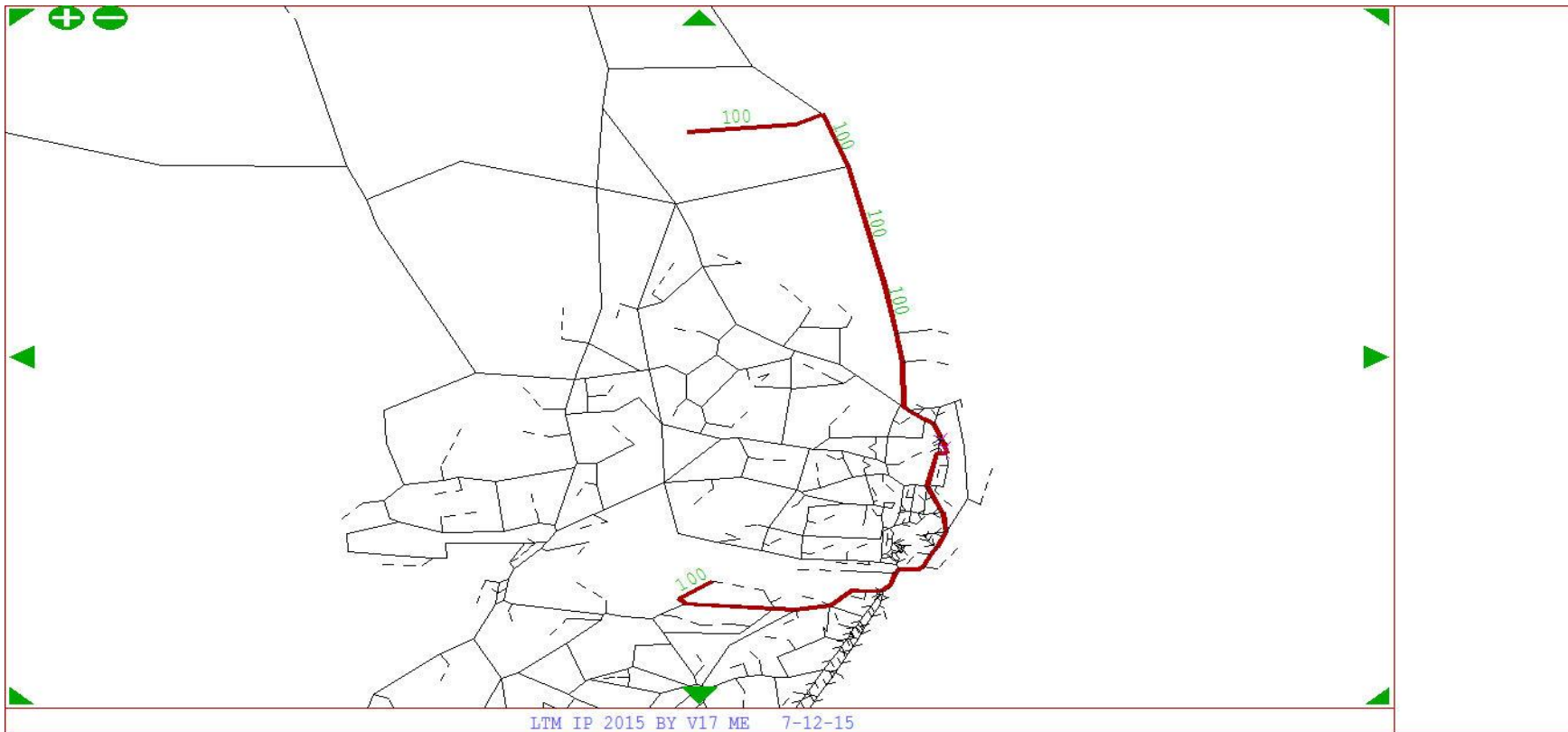
From Zone 102 To Zone 122 - User Class 1



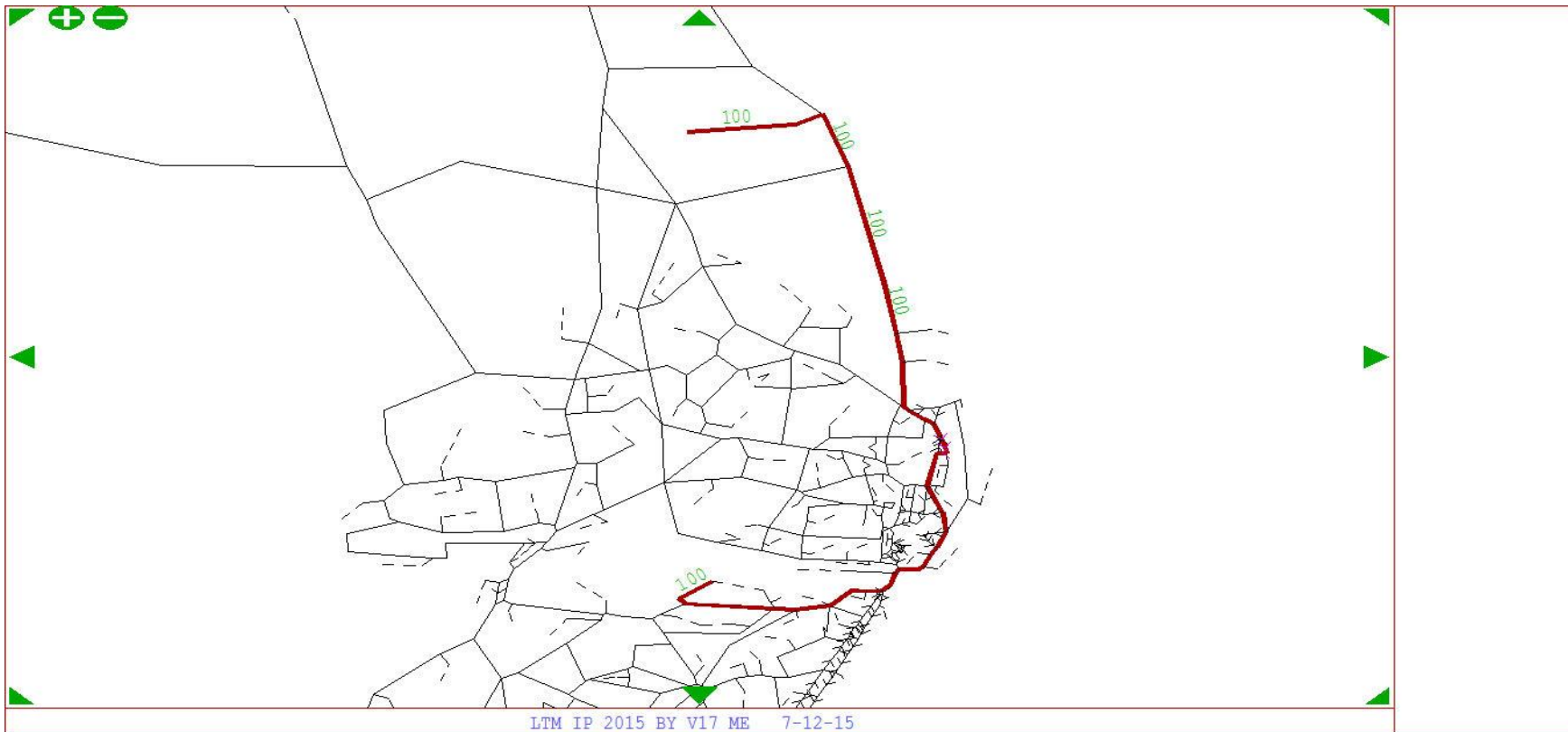
From Zone 102 To Zone 122 - User Class 2



From Zone 102 To Zone 122 - User Class 3

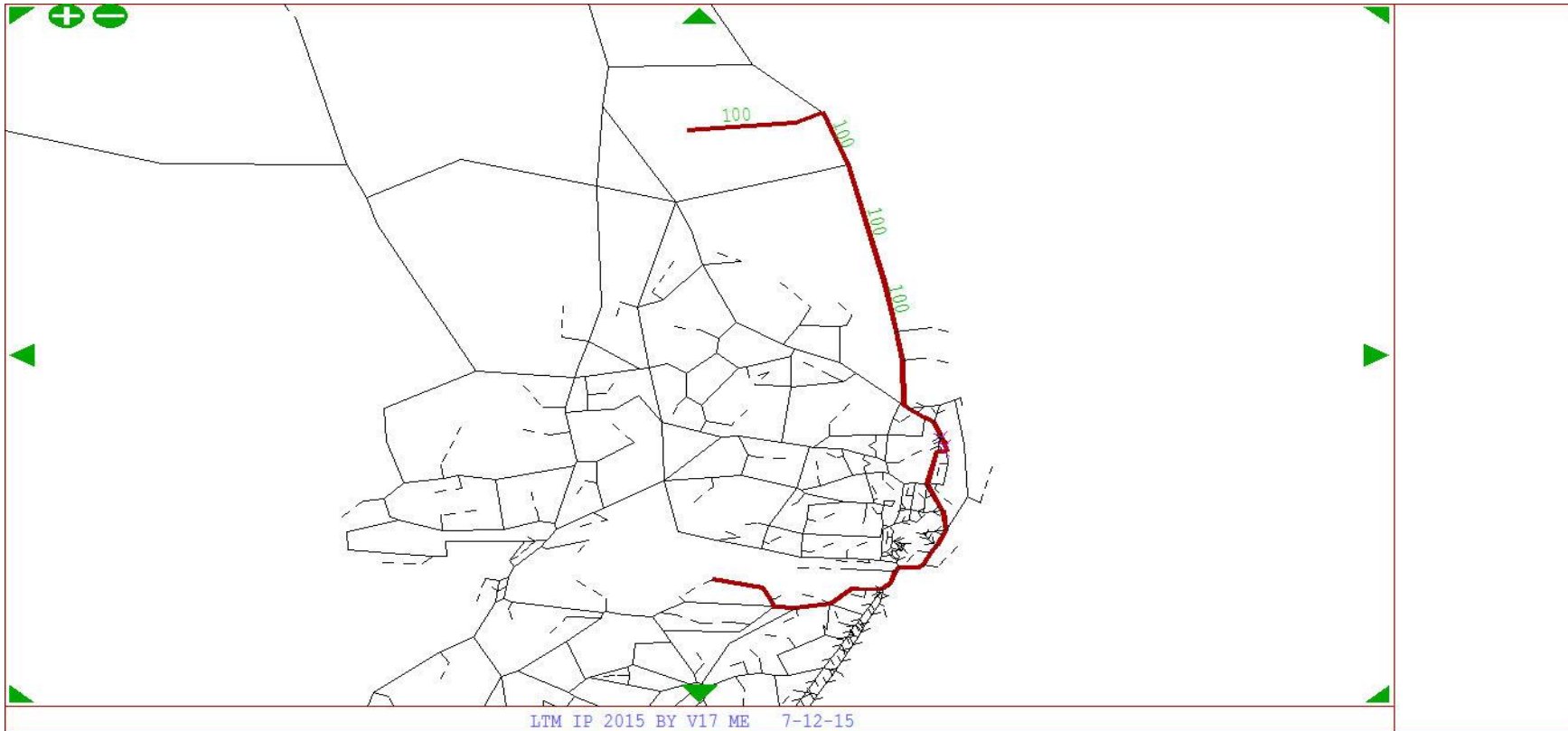


From Zone 102 To Zone 122 - User Class 4



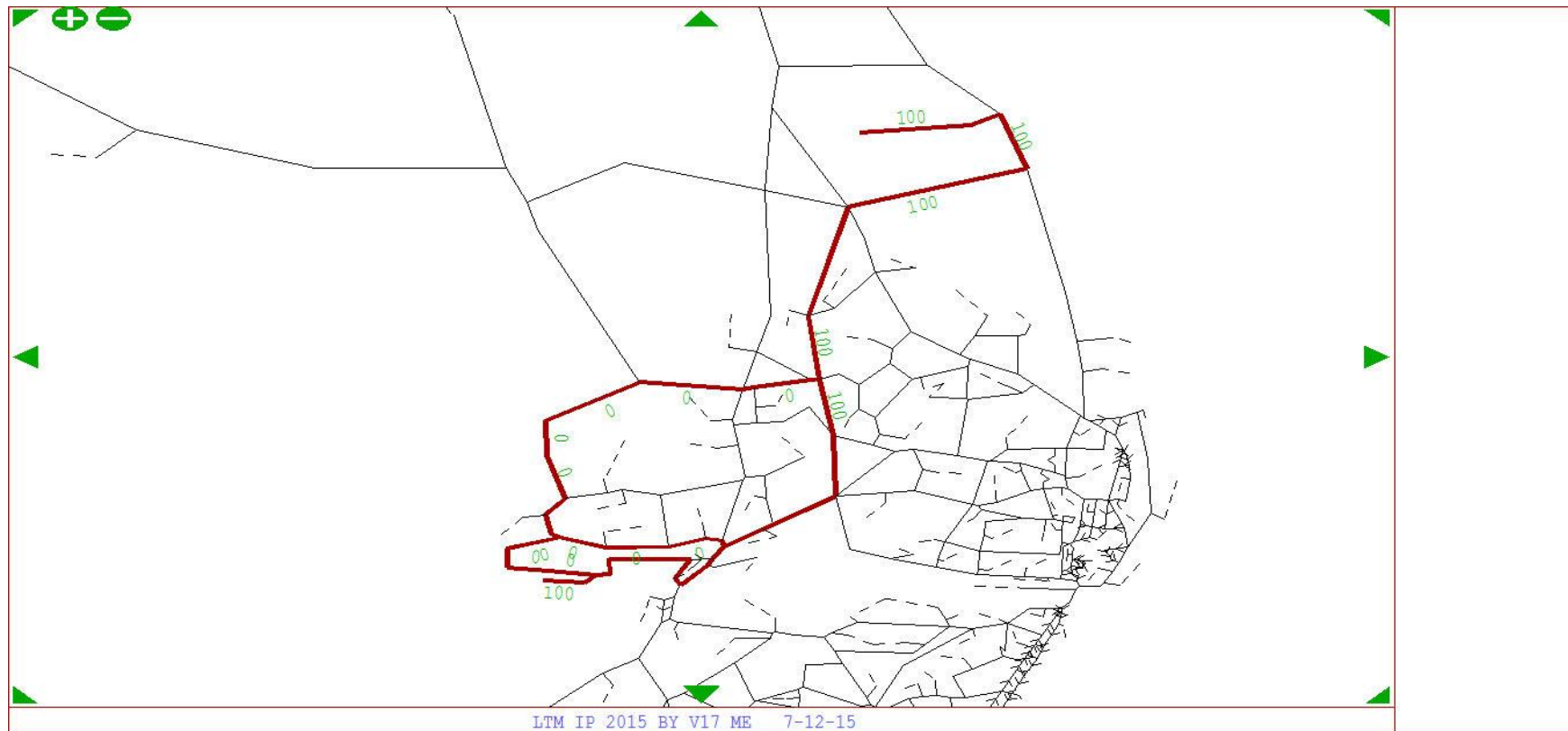
LTM IP 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 122 - User Class 5

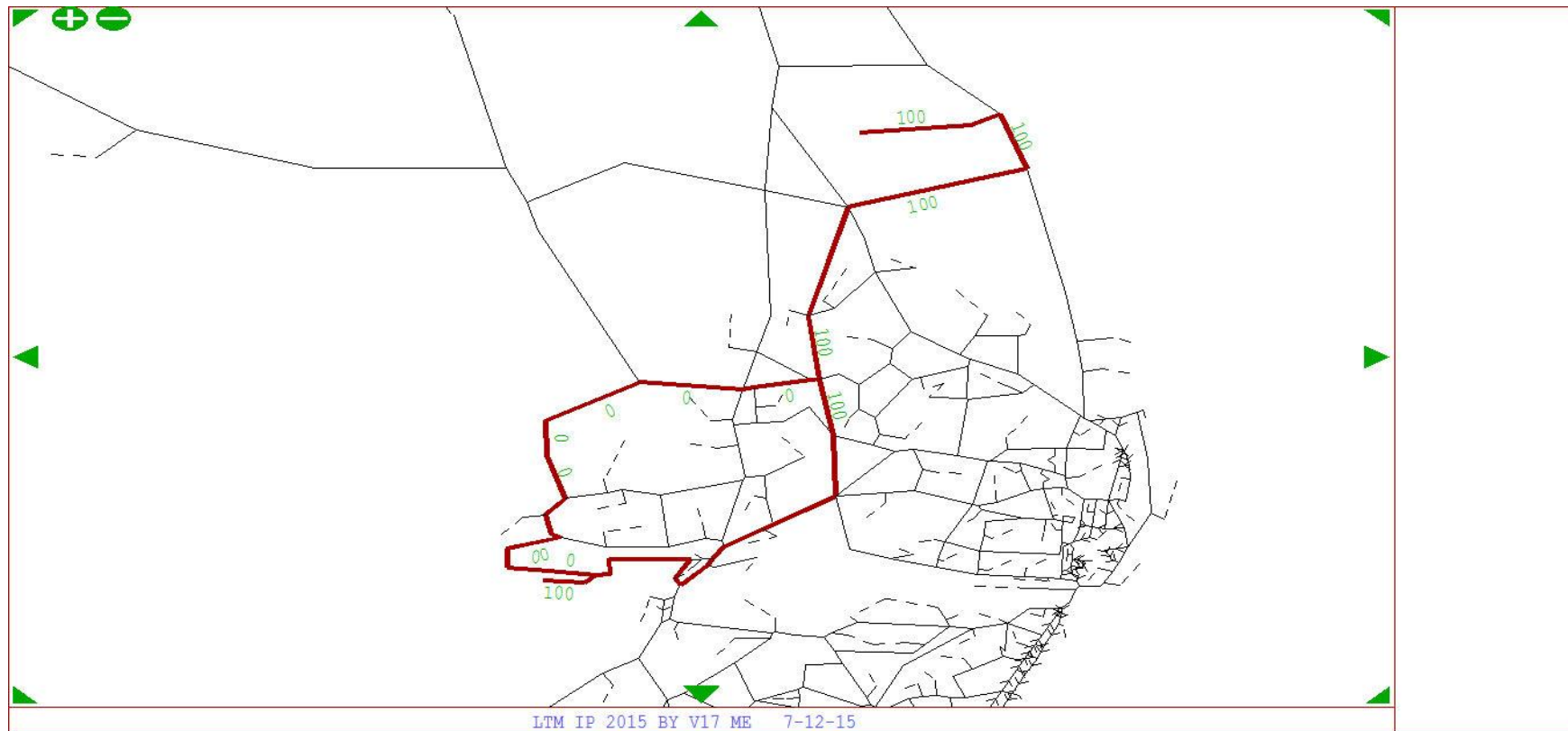


LTM IP 2015 BY V17 ME 7-12-15

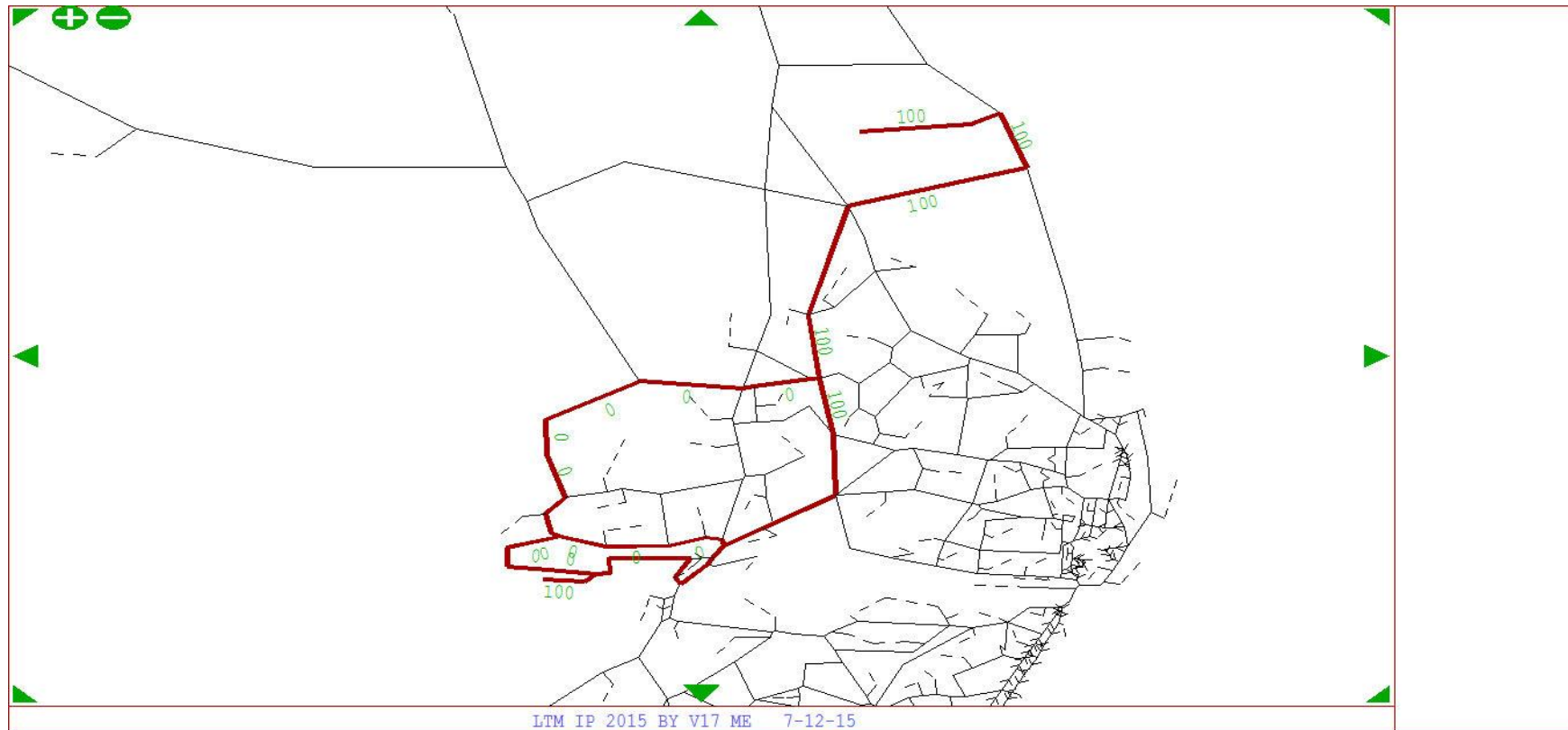
From Zone 102 To Zone 130 - User Class 1



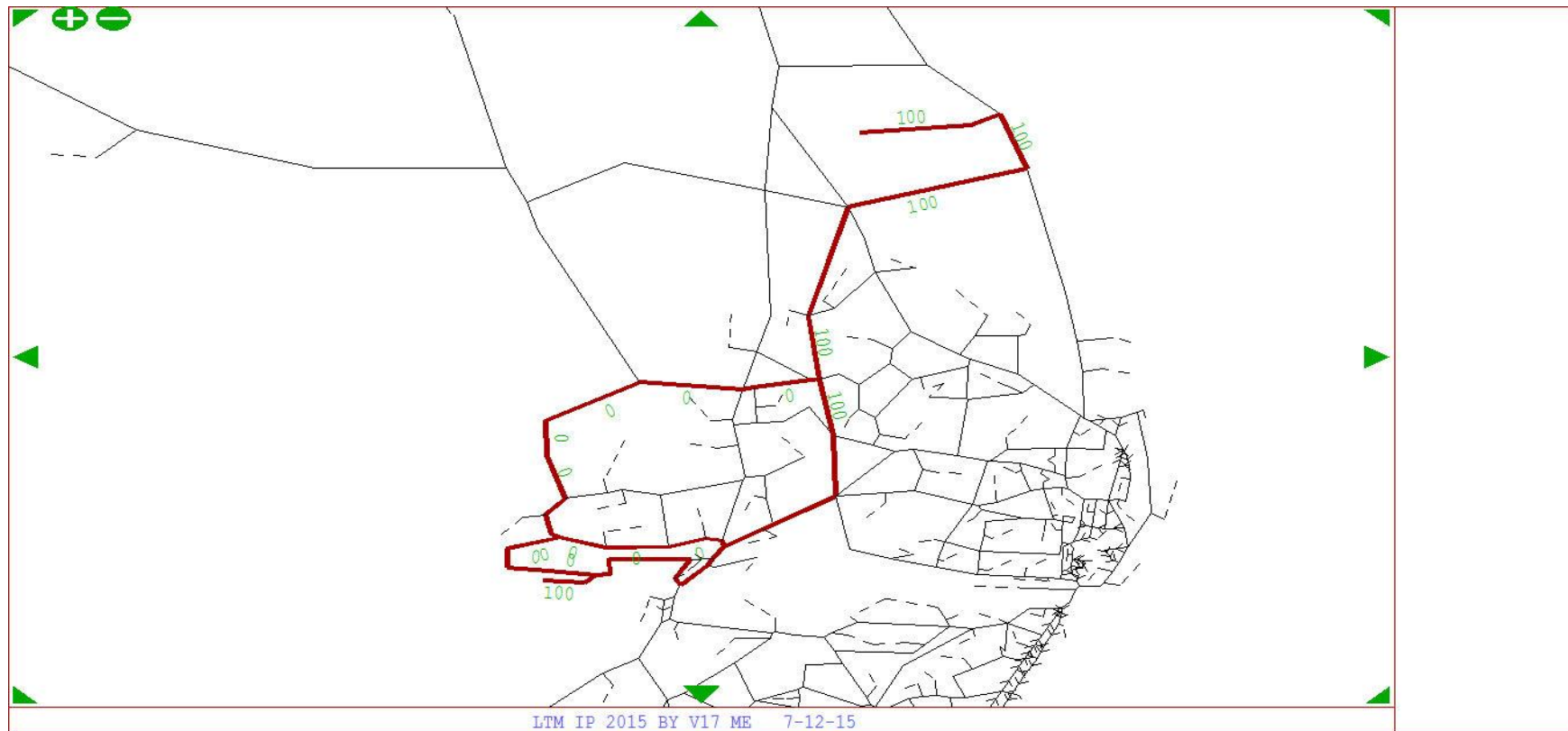
From Zone 102 To Zone 130 - User Class 2



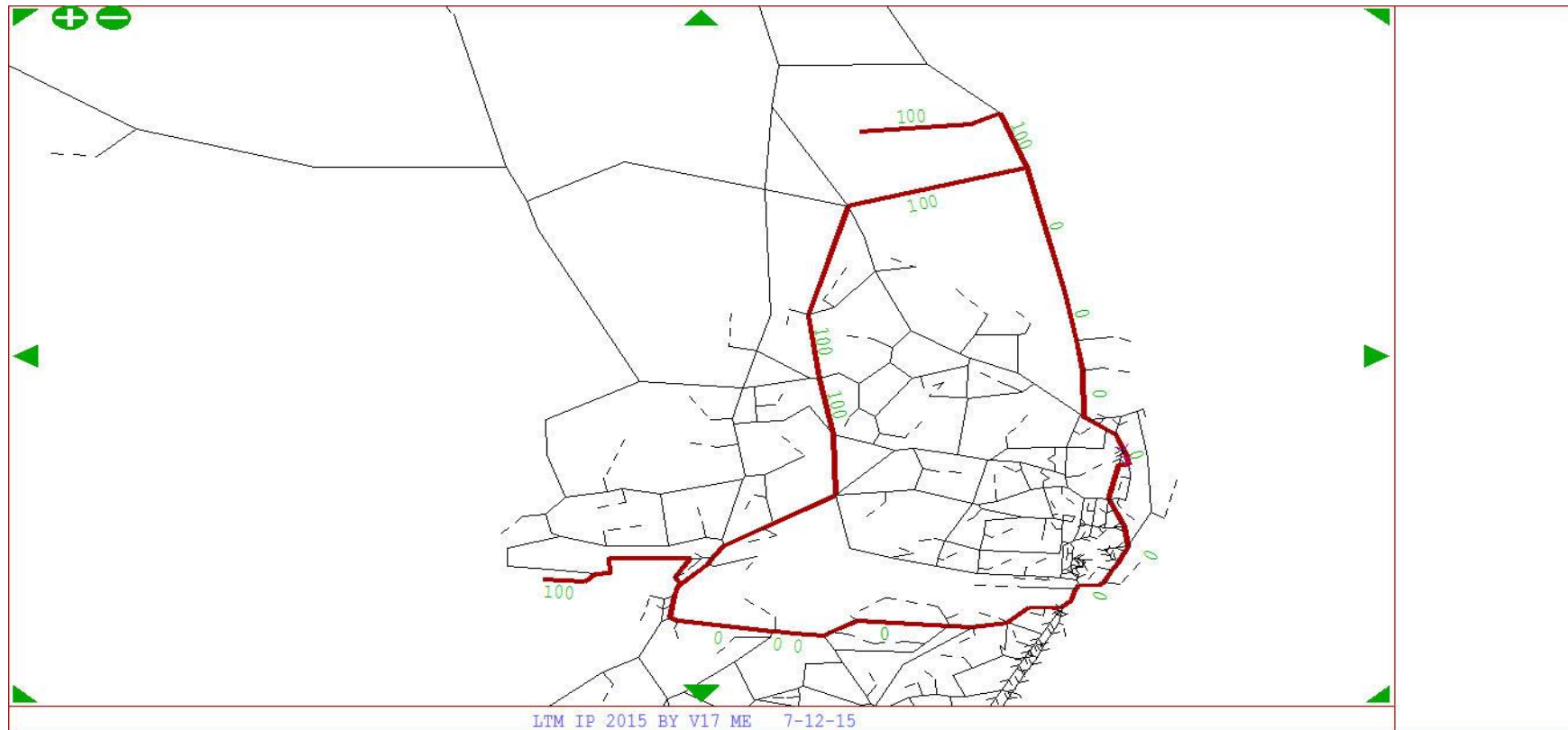
From Zone 102 To Zone 130 - User Class 3



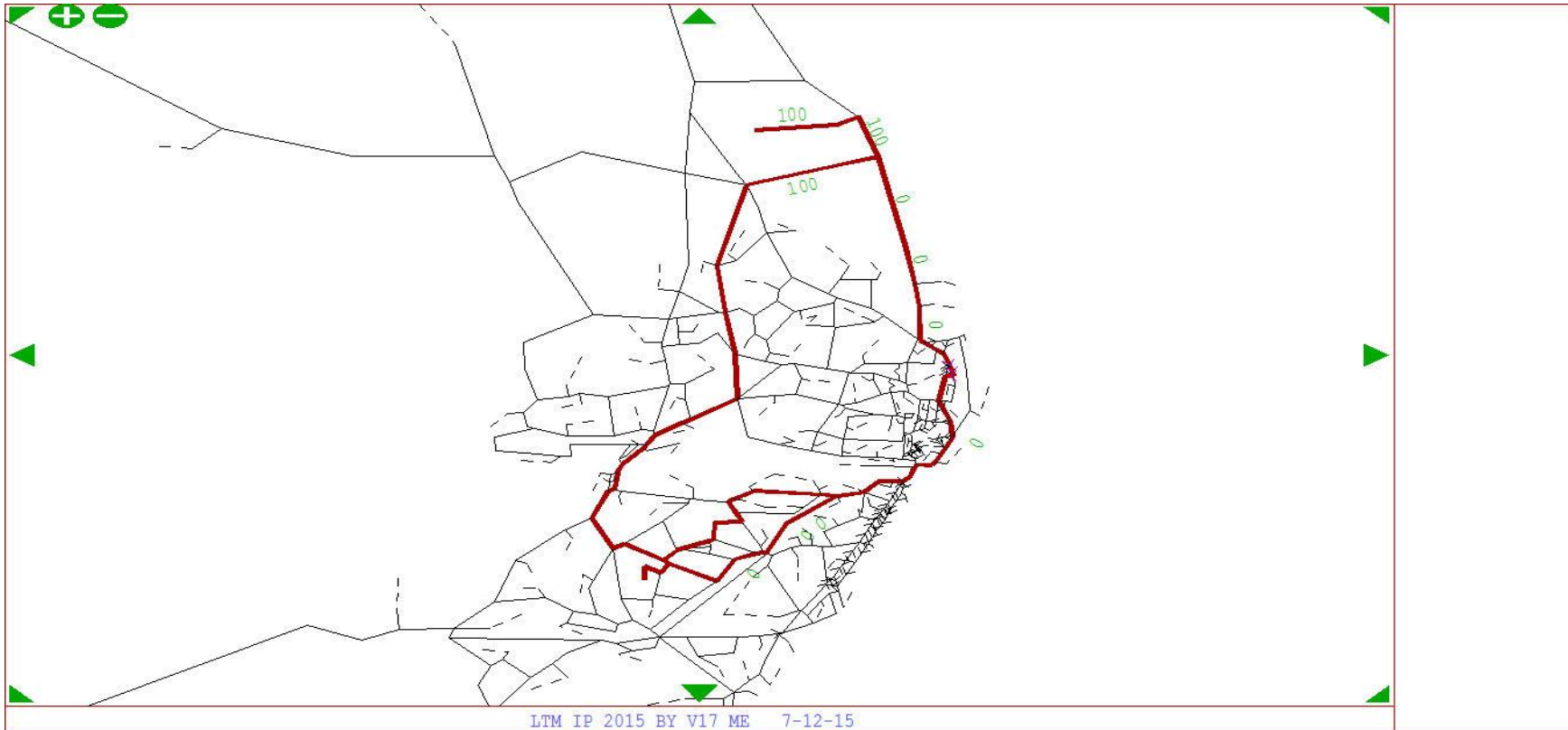
From Zone 102 To Zone 130 - User Class 4



From Zone 102 To Zone 130 - User Class 5

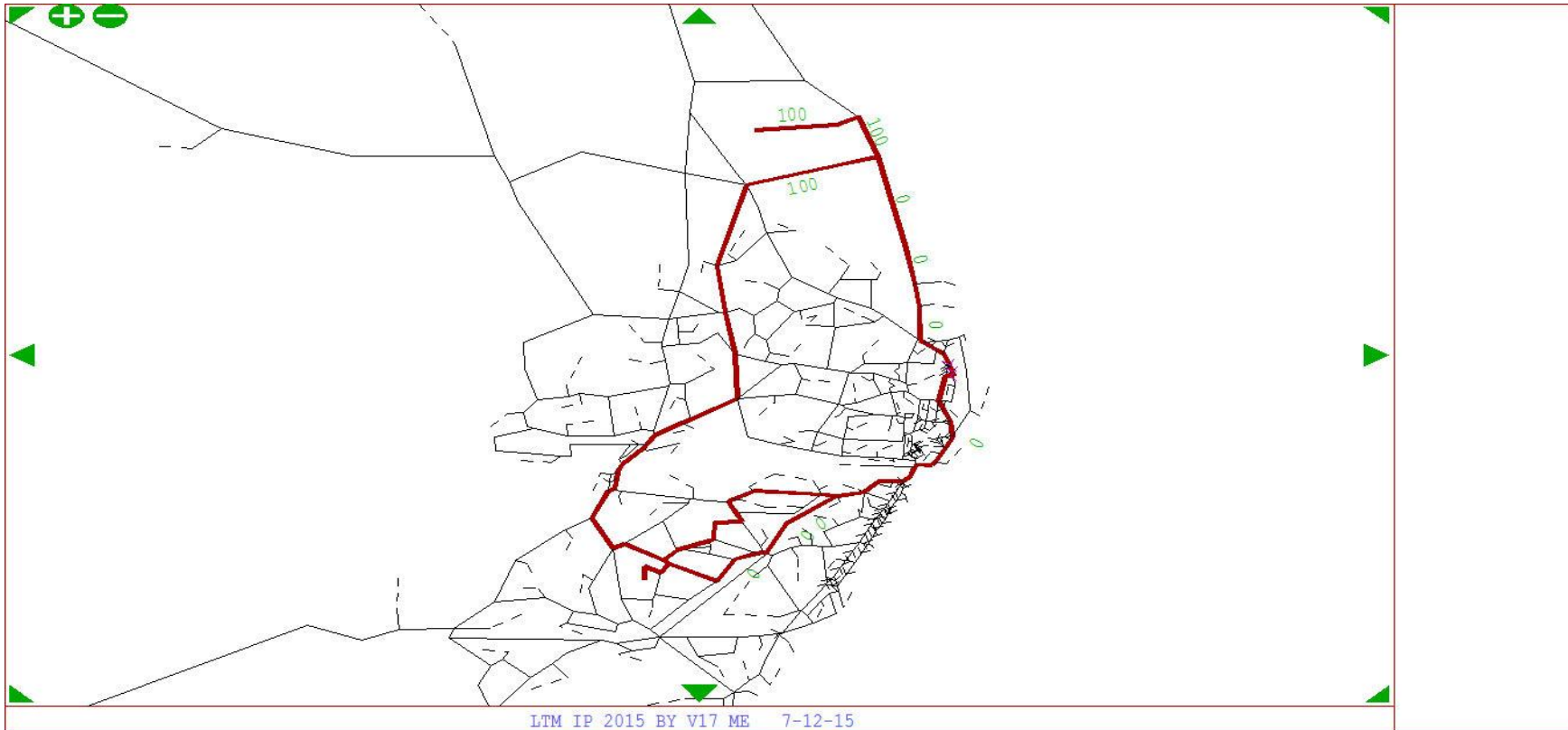


From Zone 102 To Zone 131 - User Class 1

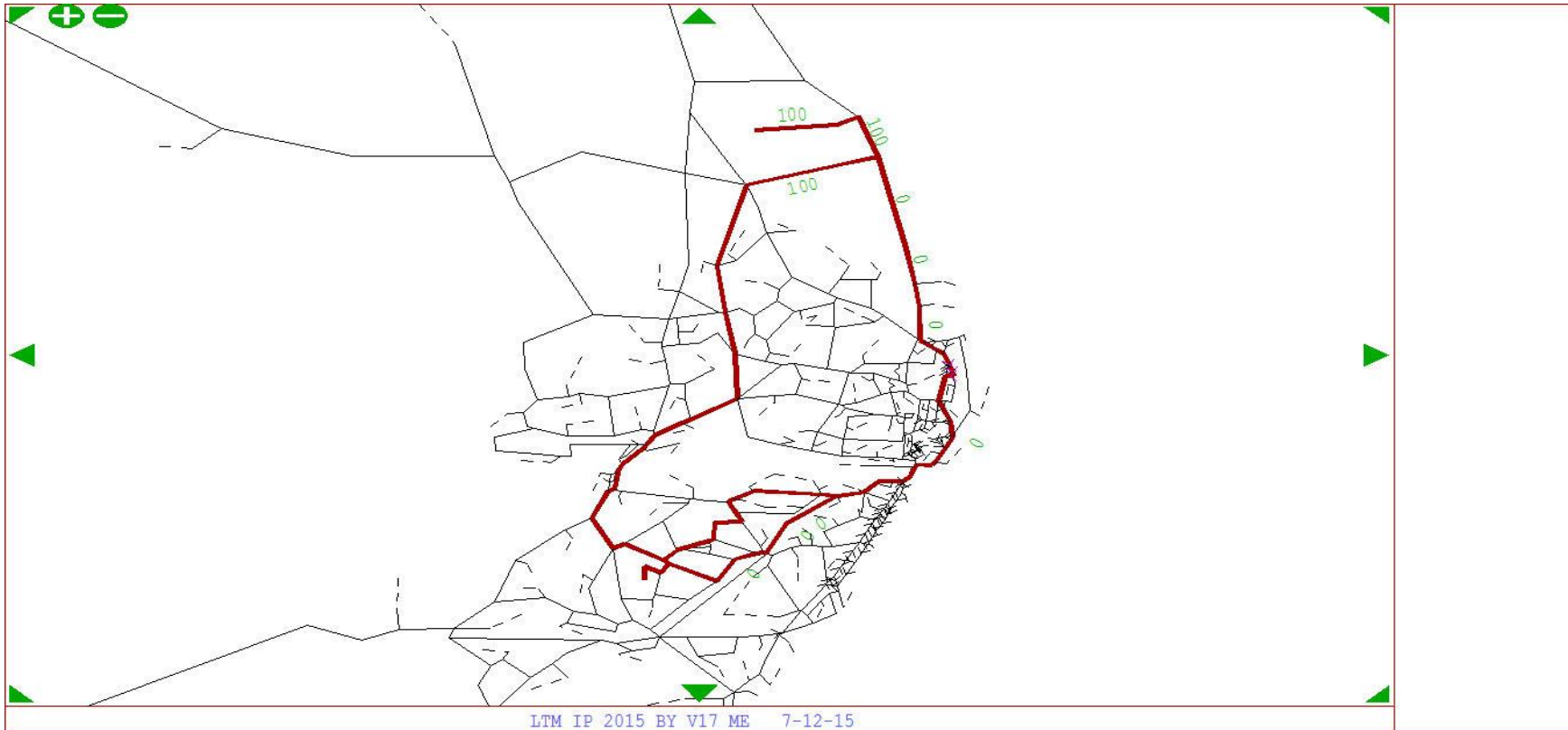


LTM IP 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 131 - User Class 2

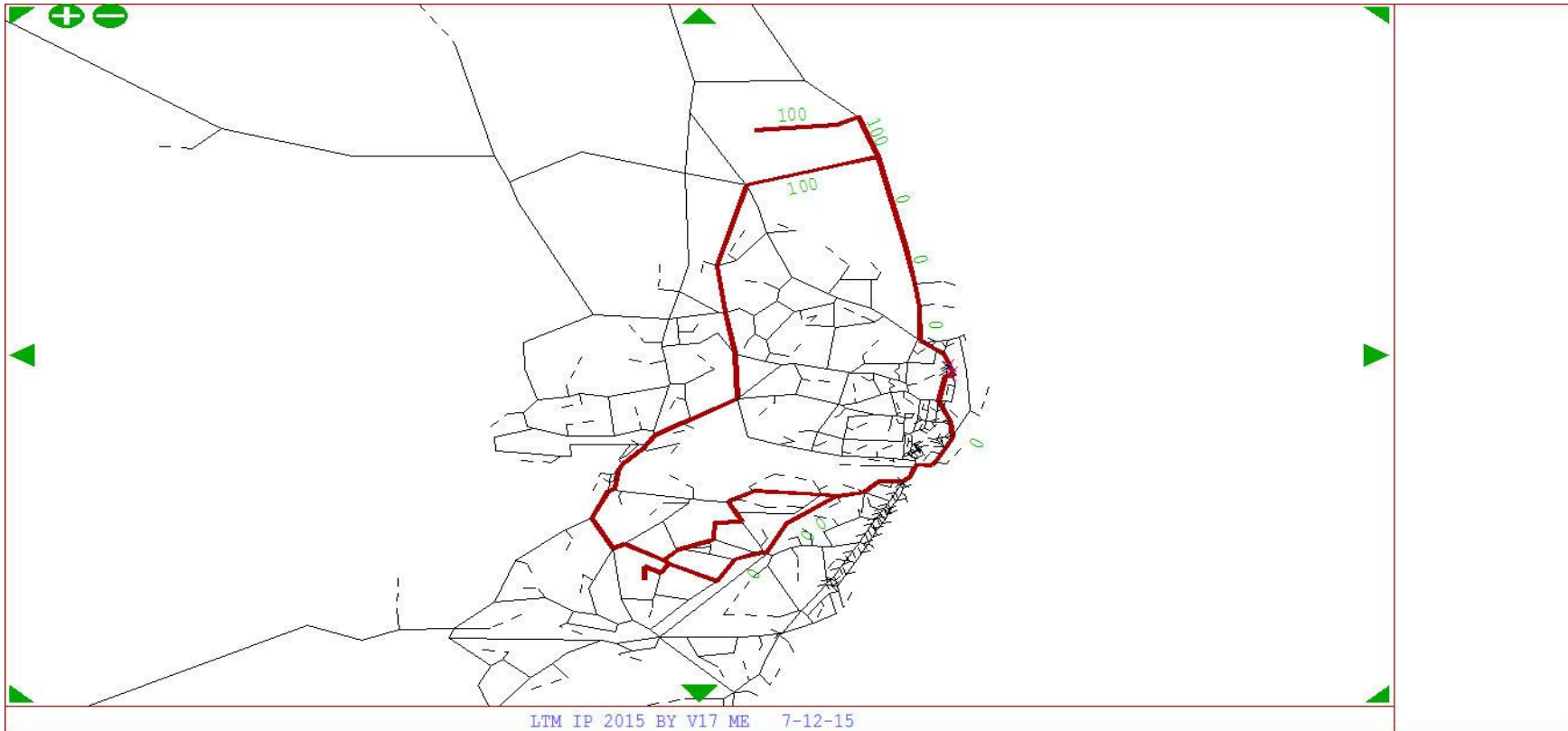


From Zone 102 To Zone 131 - User Class 3



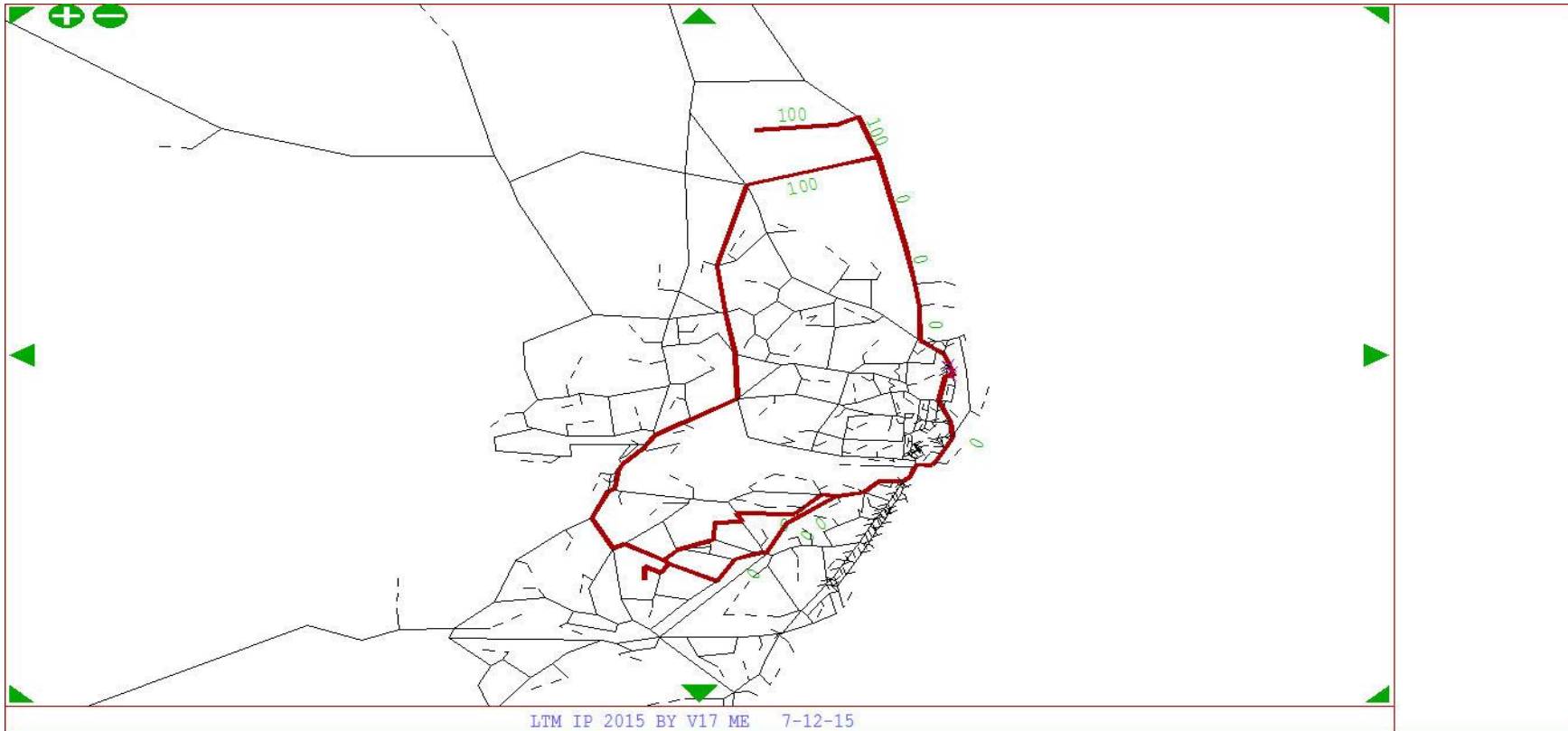
LTM IP 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 131 - User Class 4

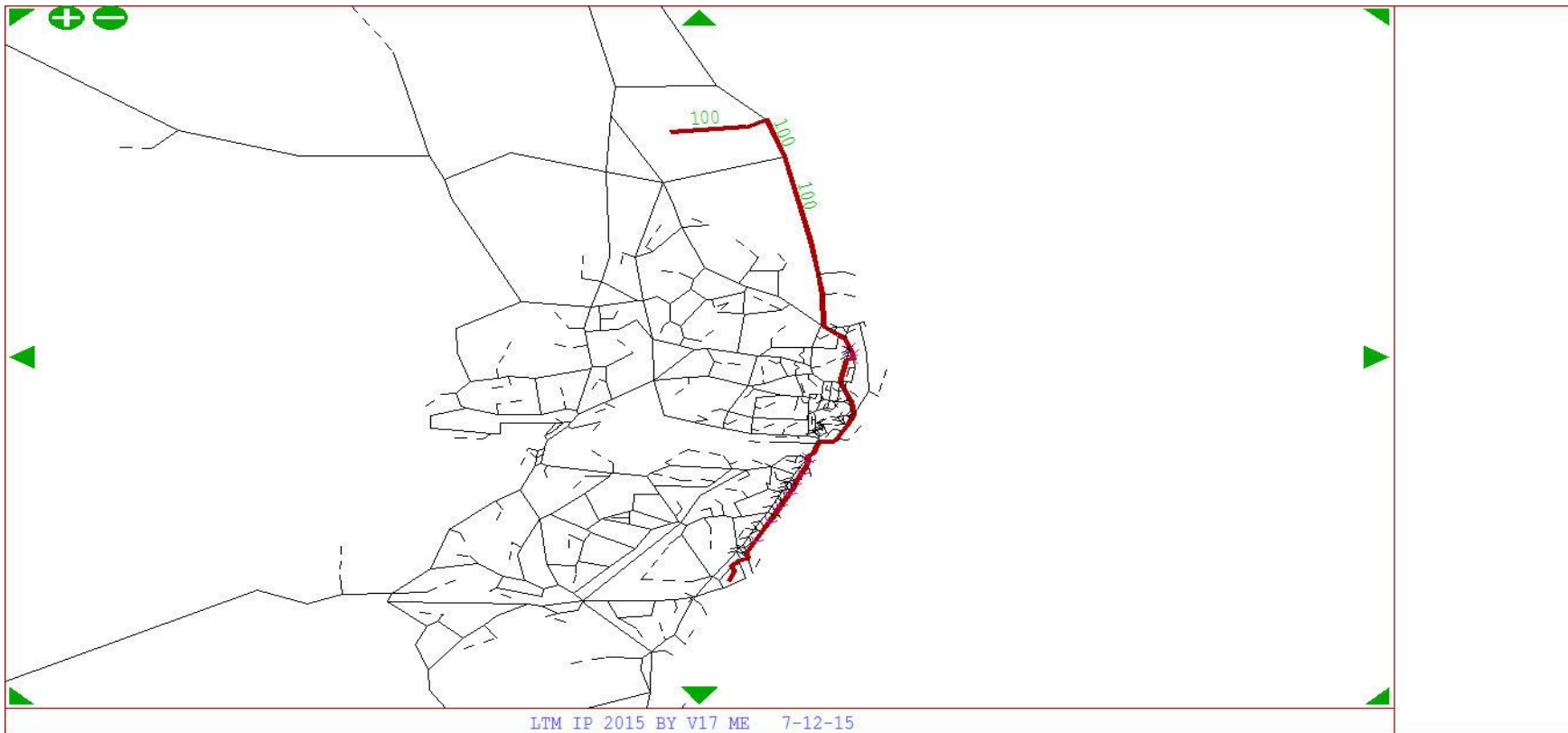


LTM IP 2015 BY V17 ME 7-12-15

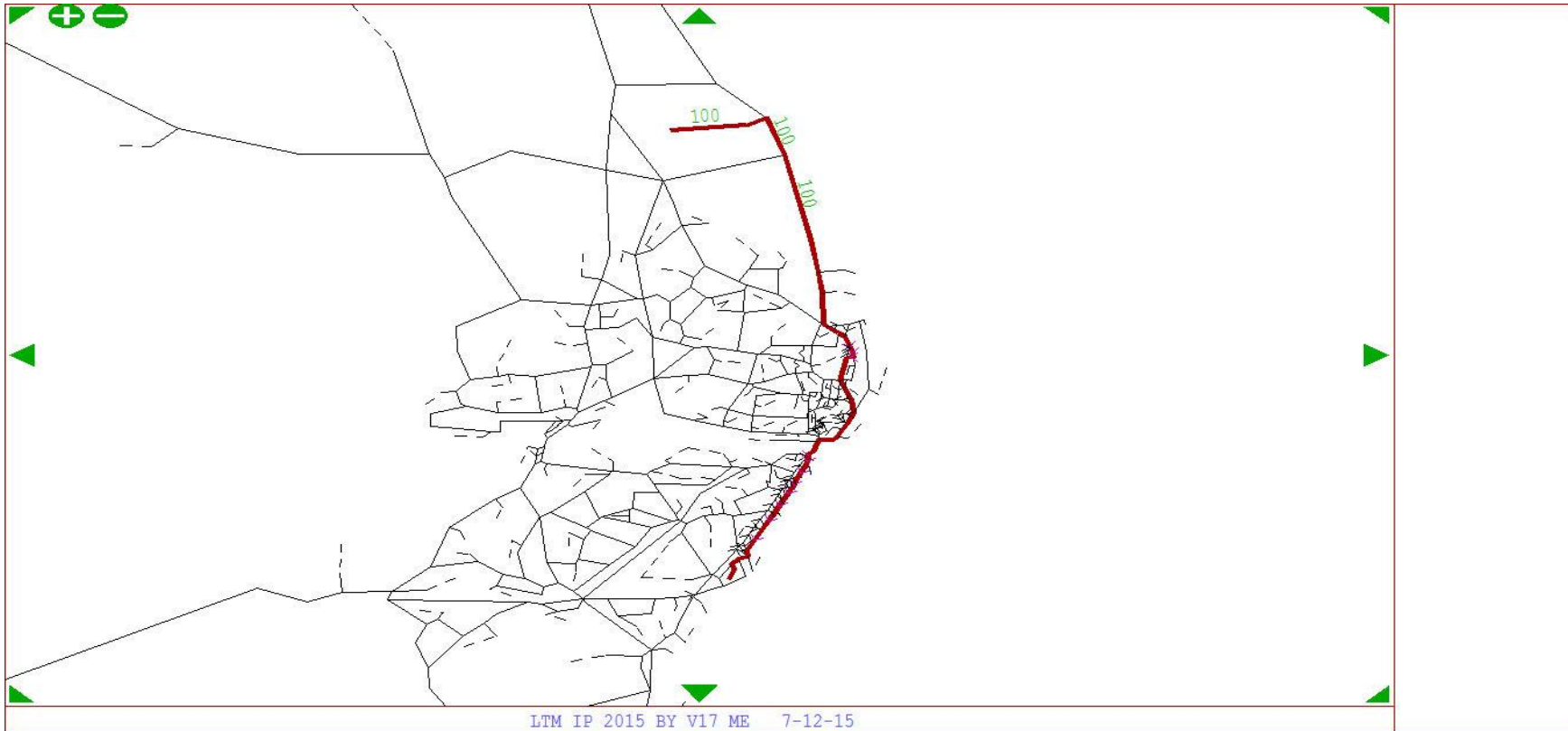
From Zone 102 To Zone 131 - User Class 5



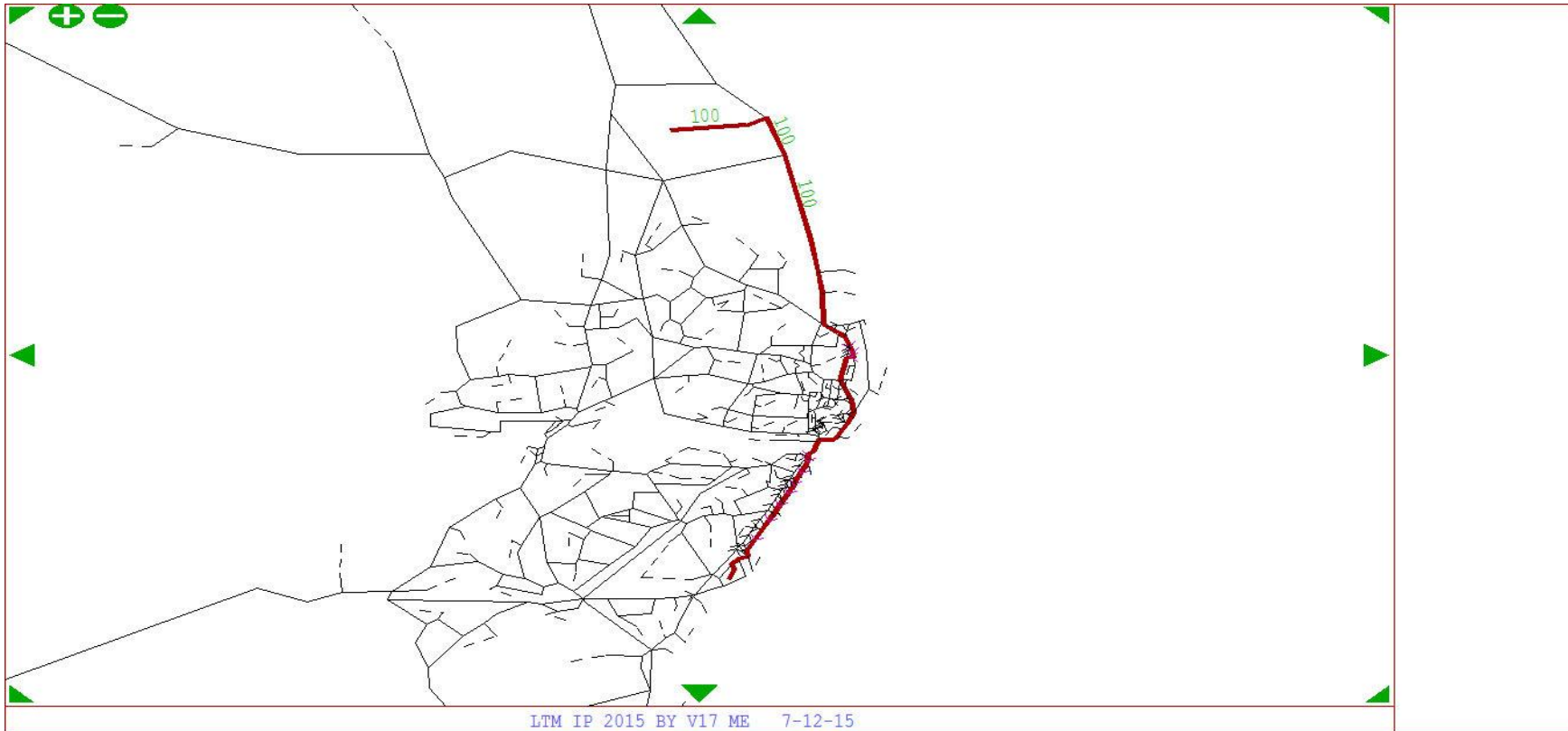
From Zone 102 To Zone 136 - User Class 1



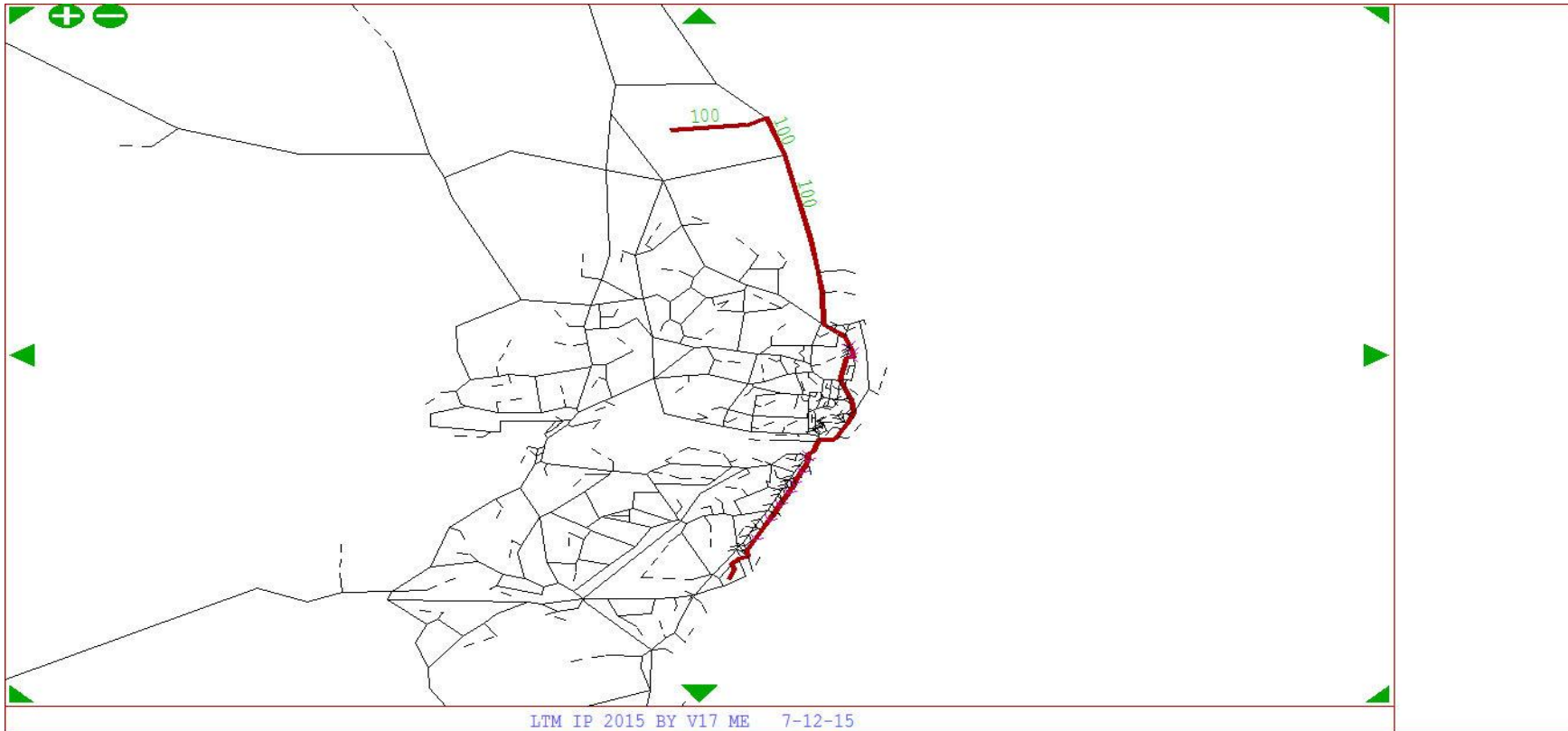
From Zone 102 To Zone 136 - User Class 2



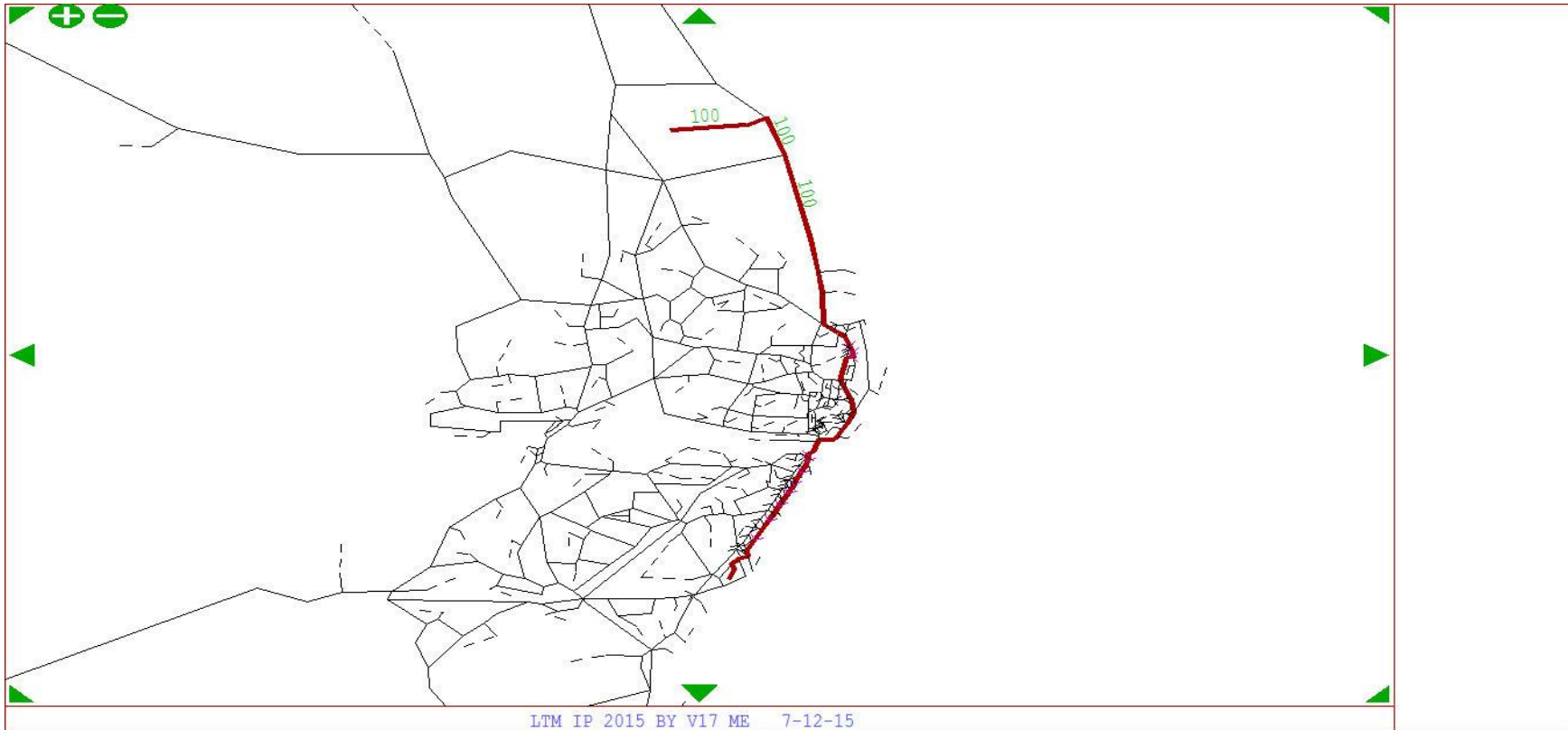
From Zone 102 To Zone 136 - User Class 3



From Zone 102 To Zone 136 - User Class 4

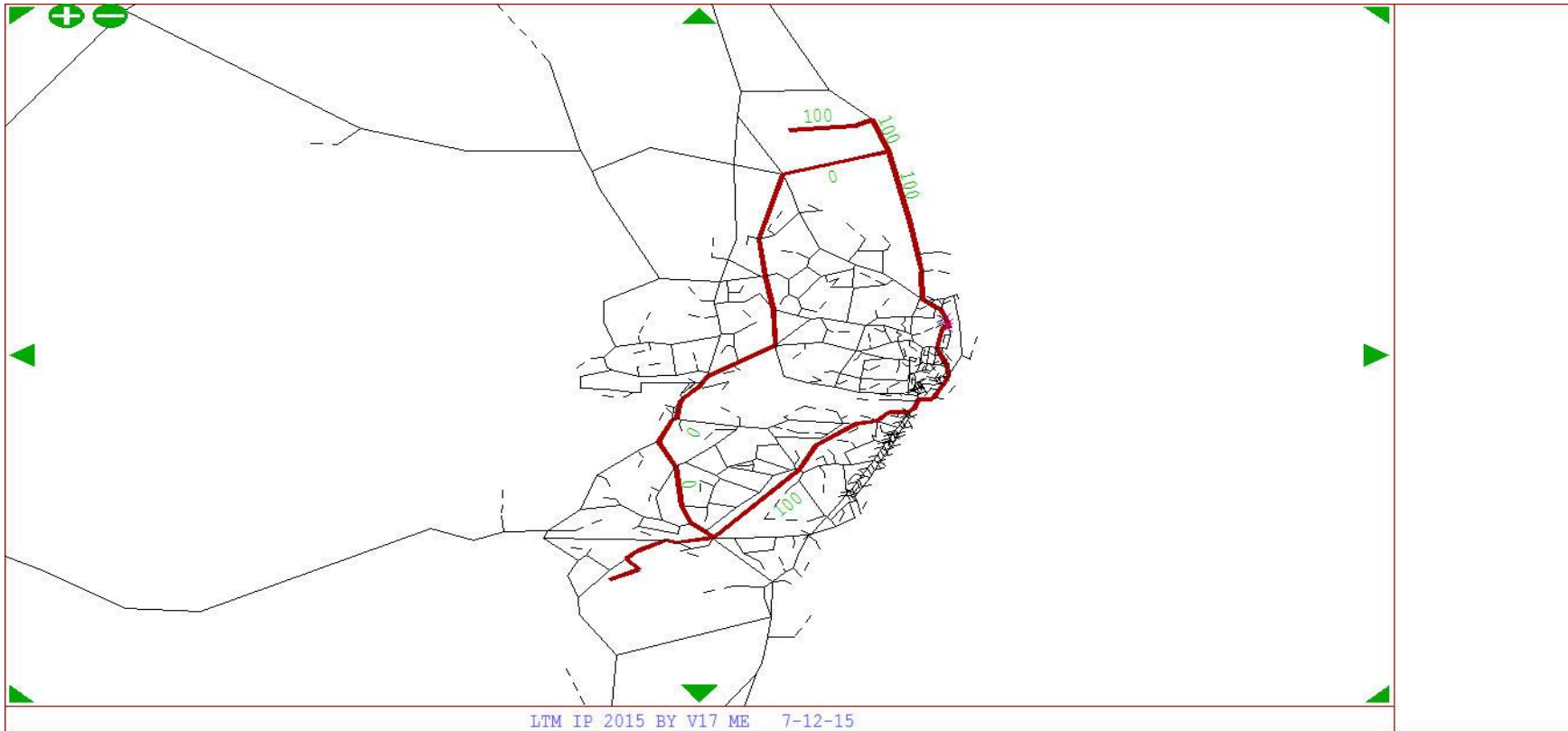


From Zone 102 To Zone 136 - User Class 5

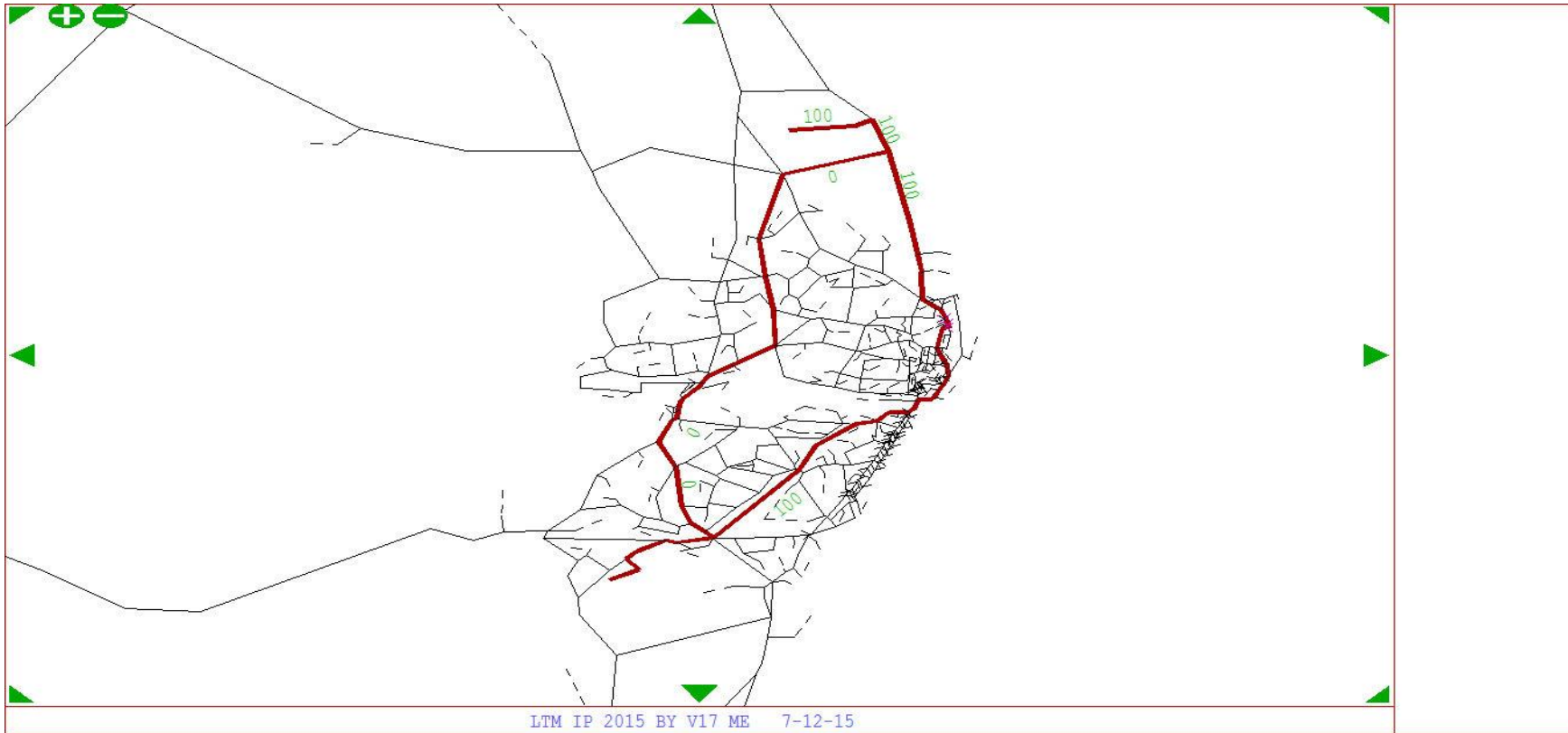


LTM IP 2015 BY V17 ME 7-12-15

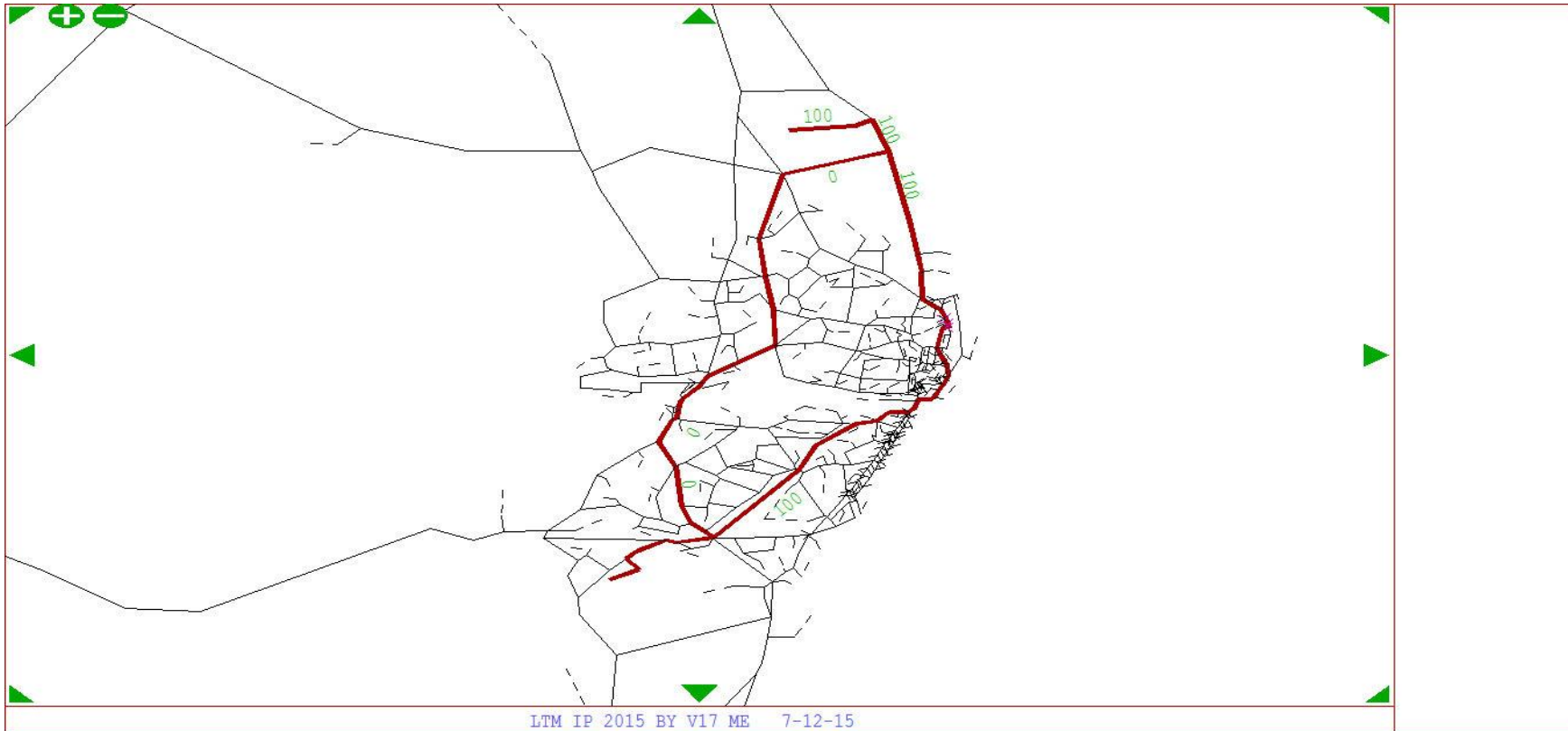
From Zone 102 To Zone 143 - User Class 1



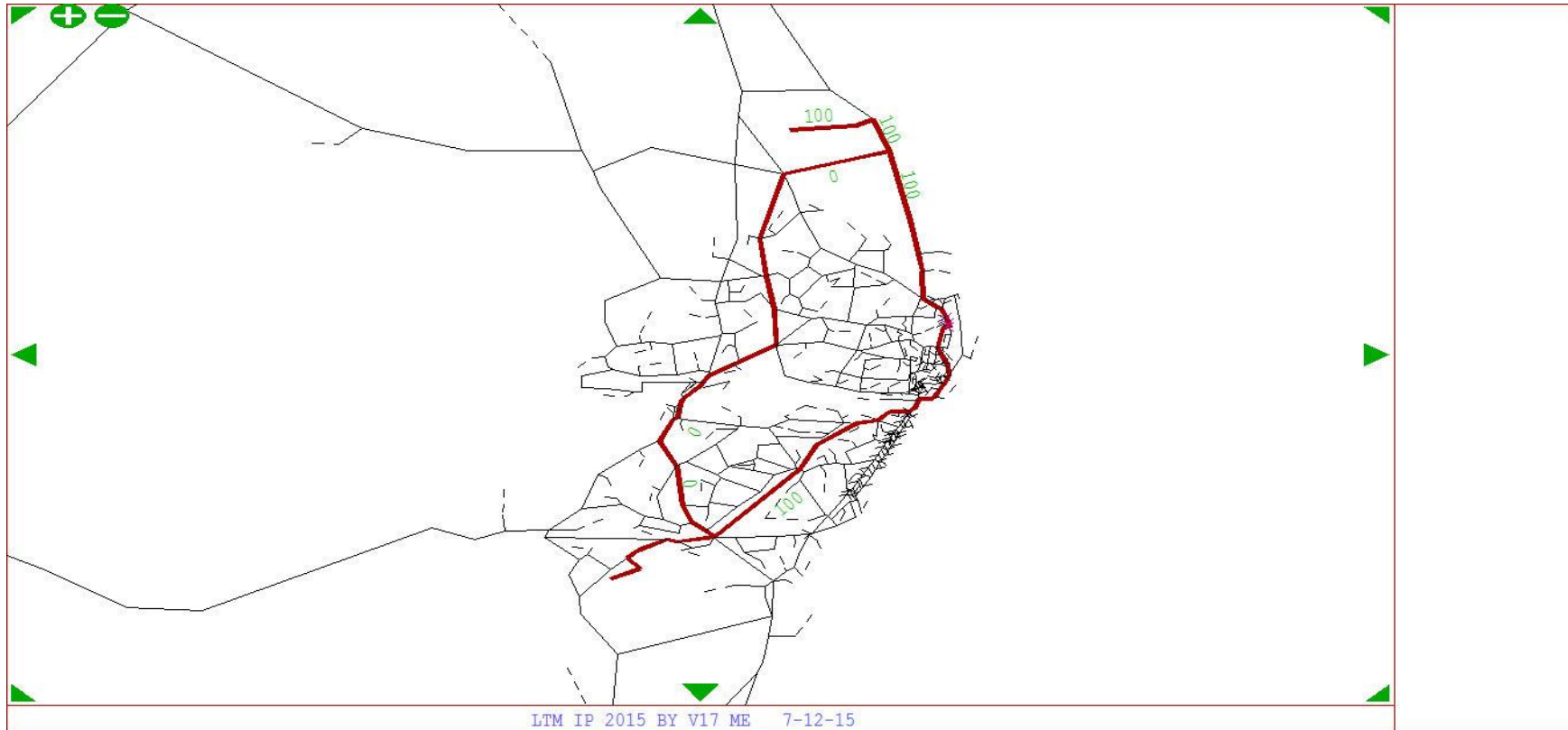
From Zone 102 To Zone 143 - User Class 2



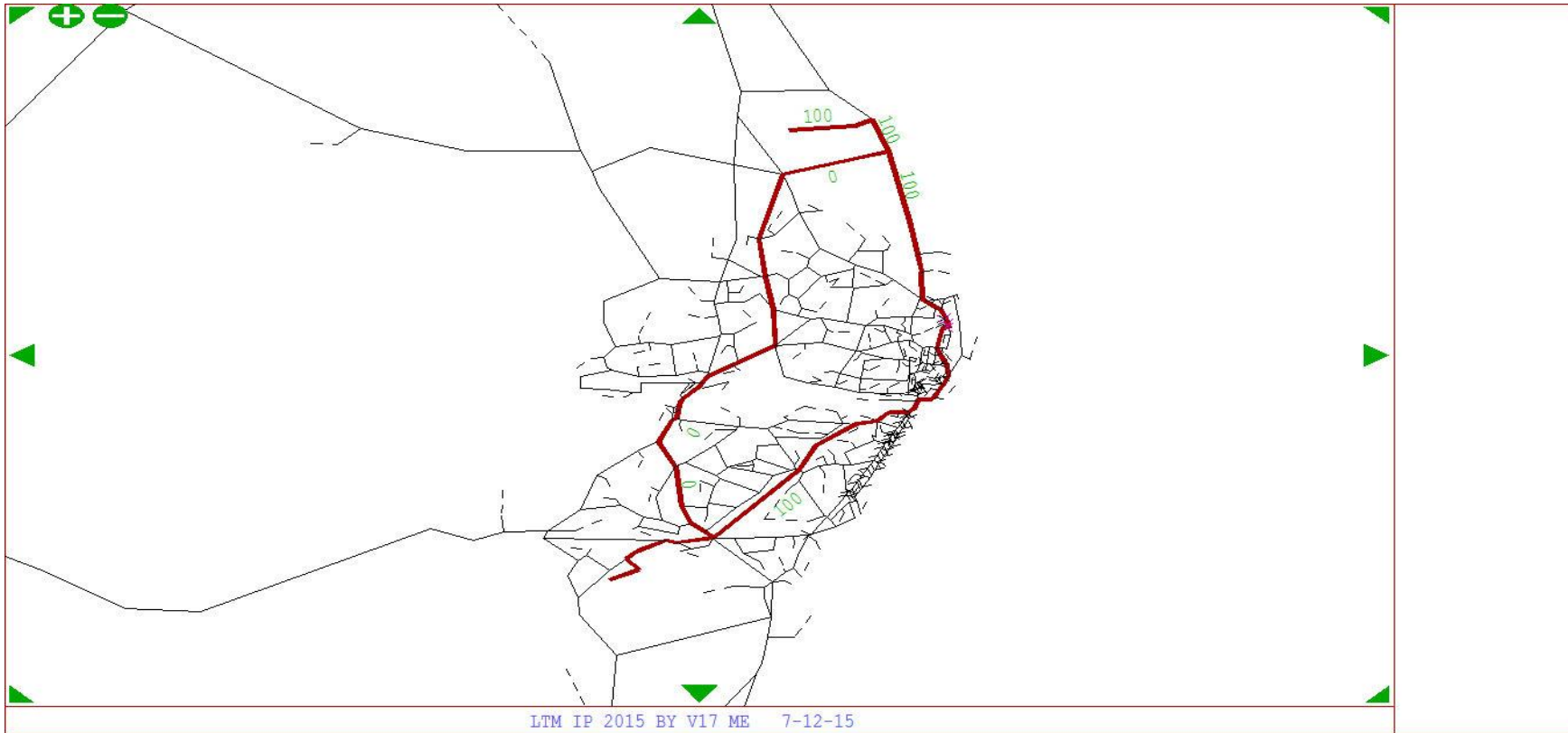
From Zone 102 To Zone 143 - User Class 3



From Zone 102 To Zone 143 - User Class 4

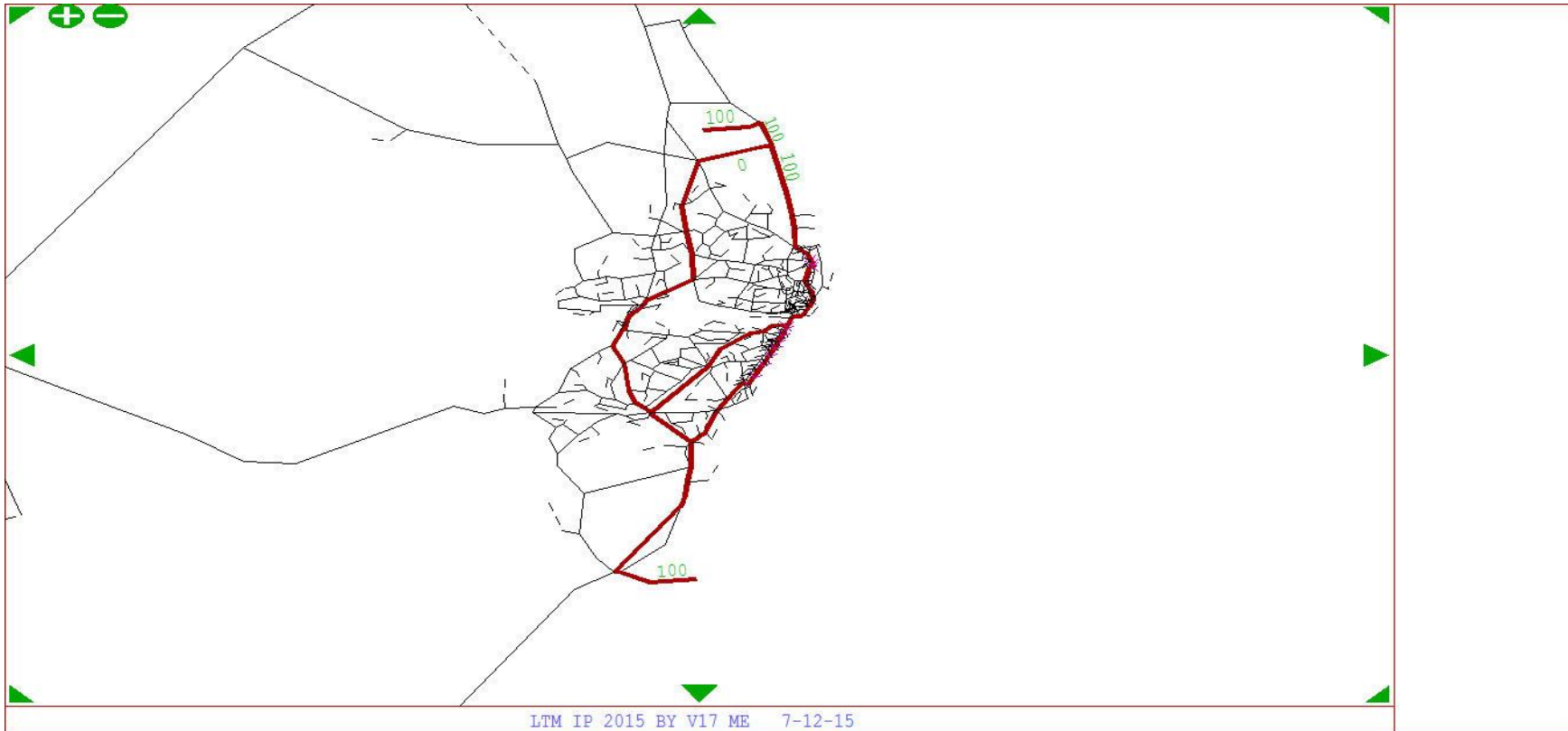


From Zone 102 To Zone 143 - User Class 5

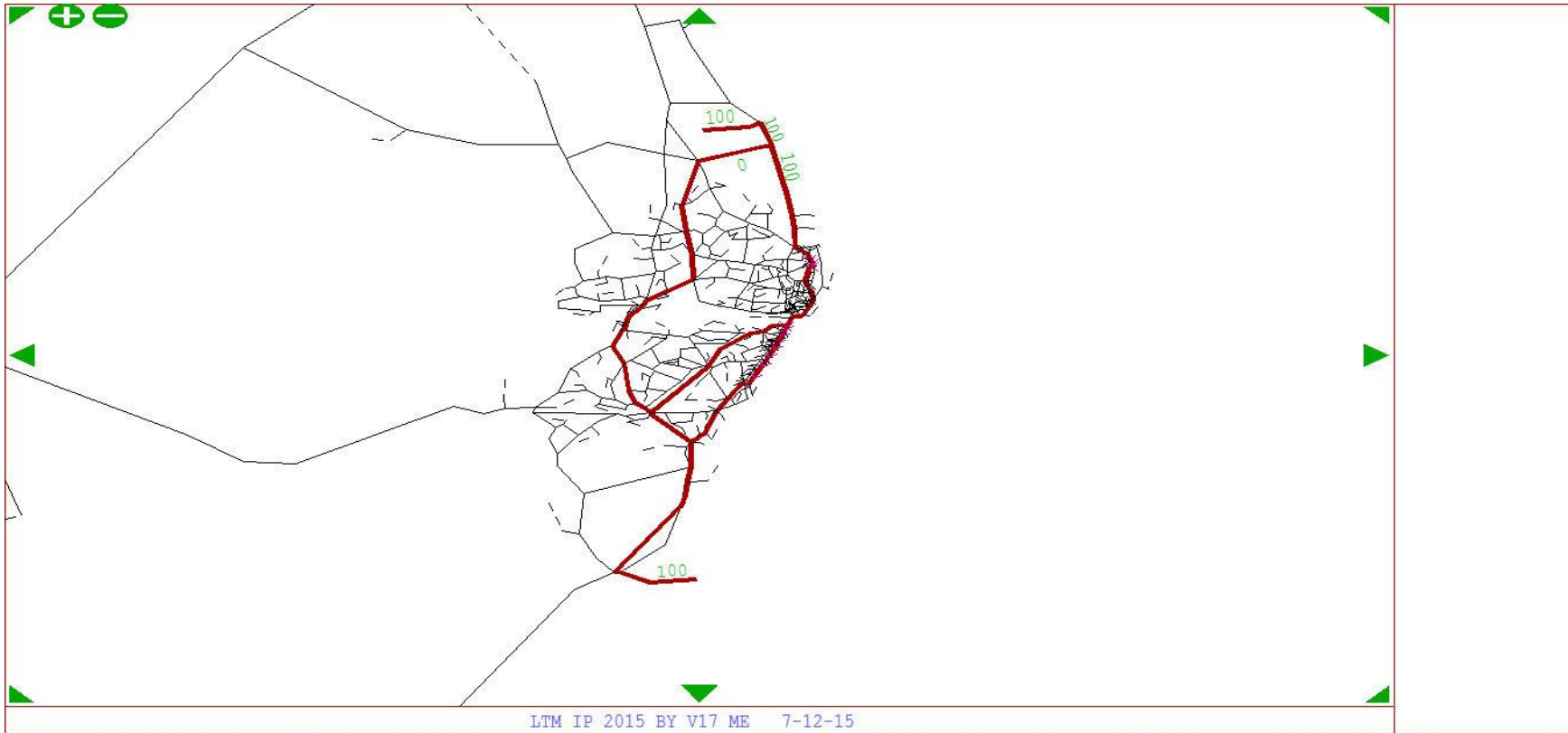


LTM IP 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 149 - User Class 1

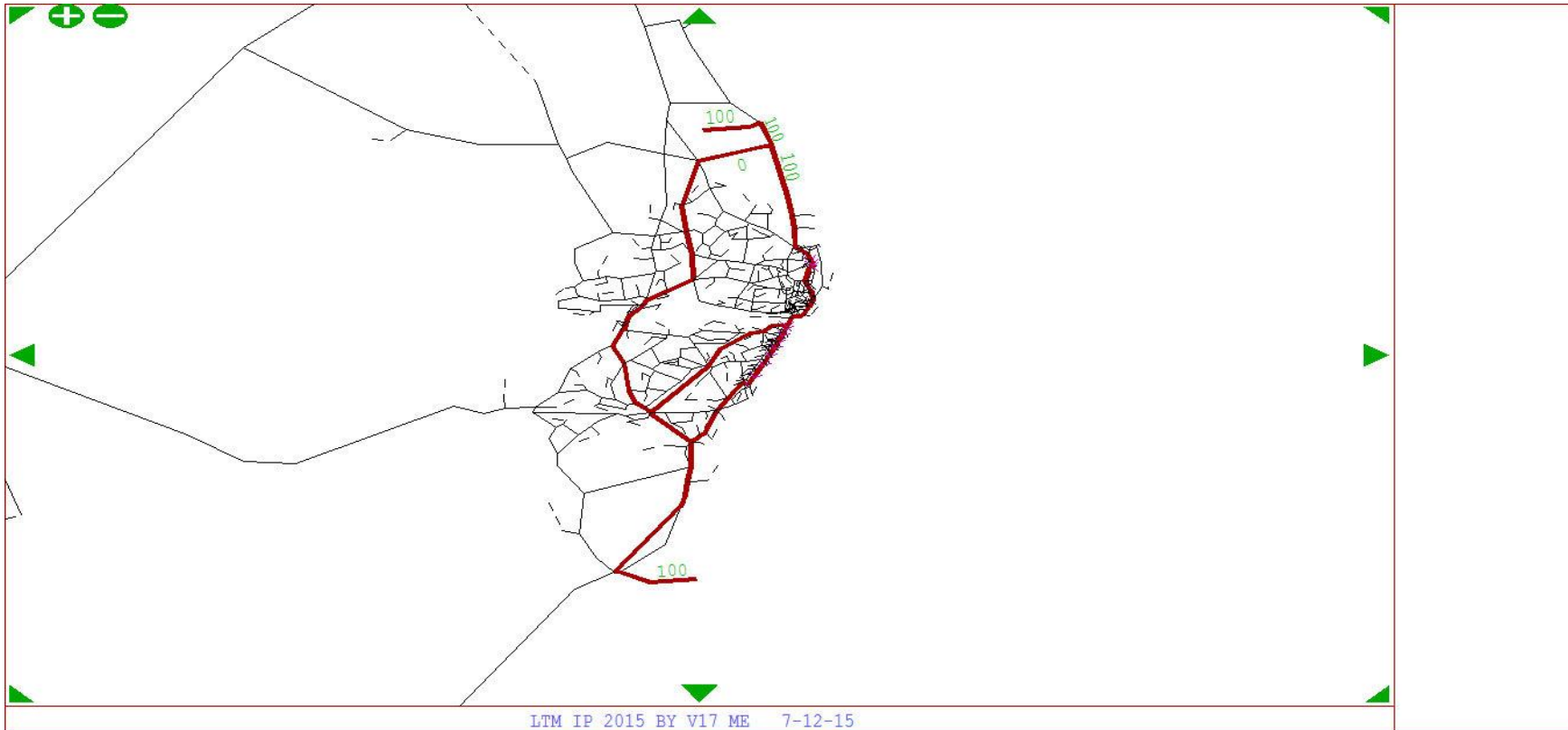


From Zone 102 To Zone 149 - User Class 2

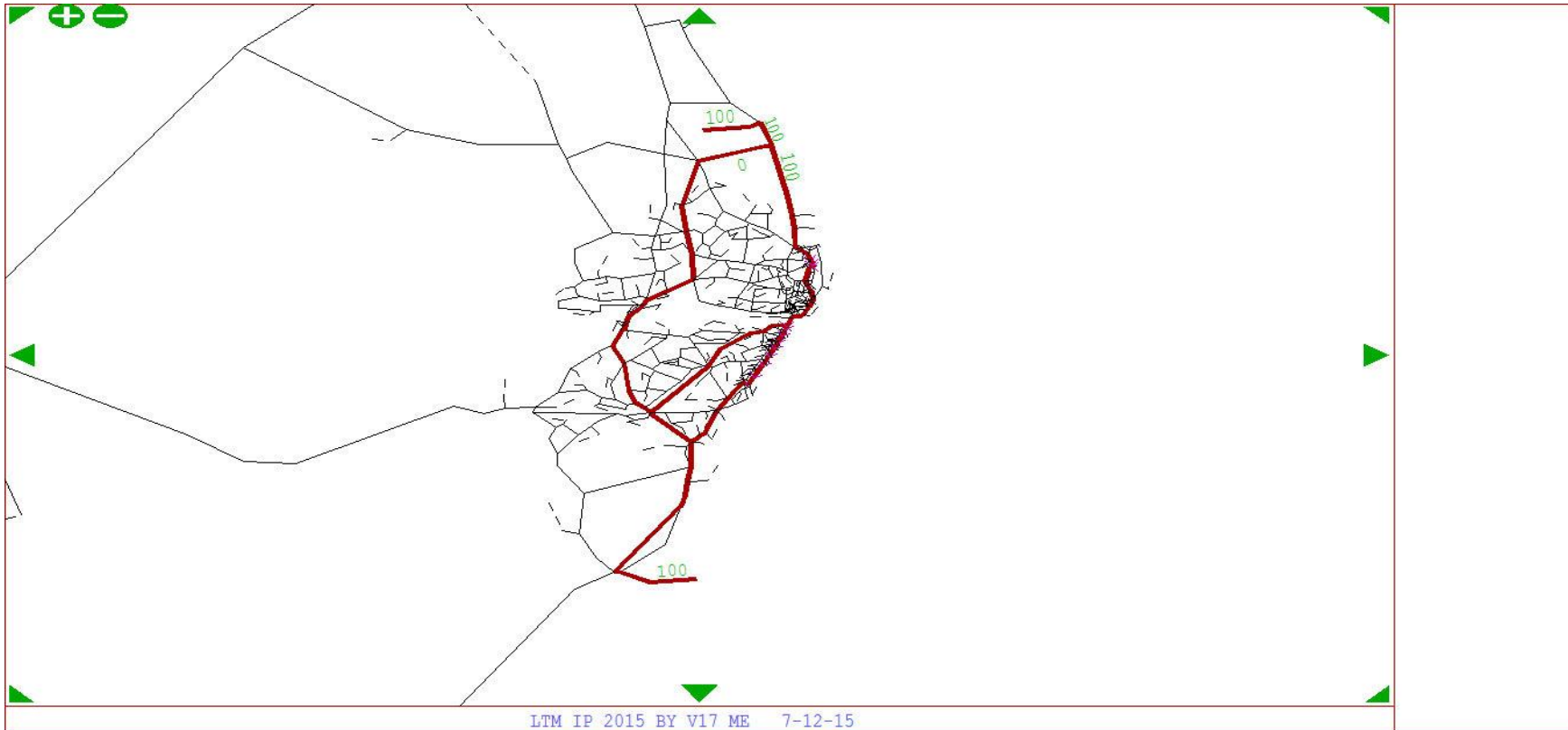


LTM IP 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 149 - User Class 3

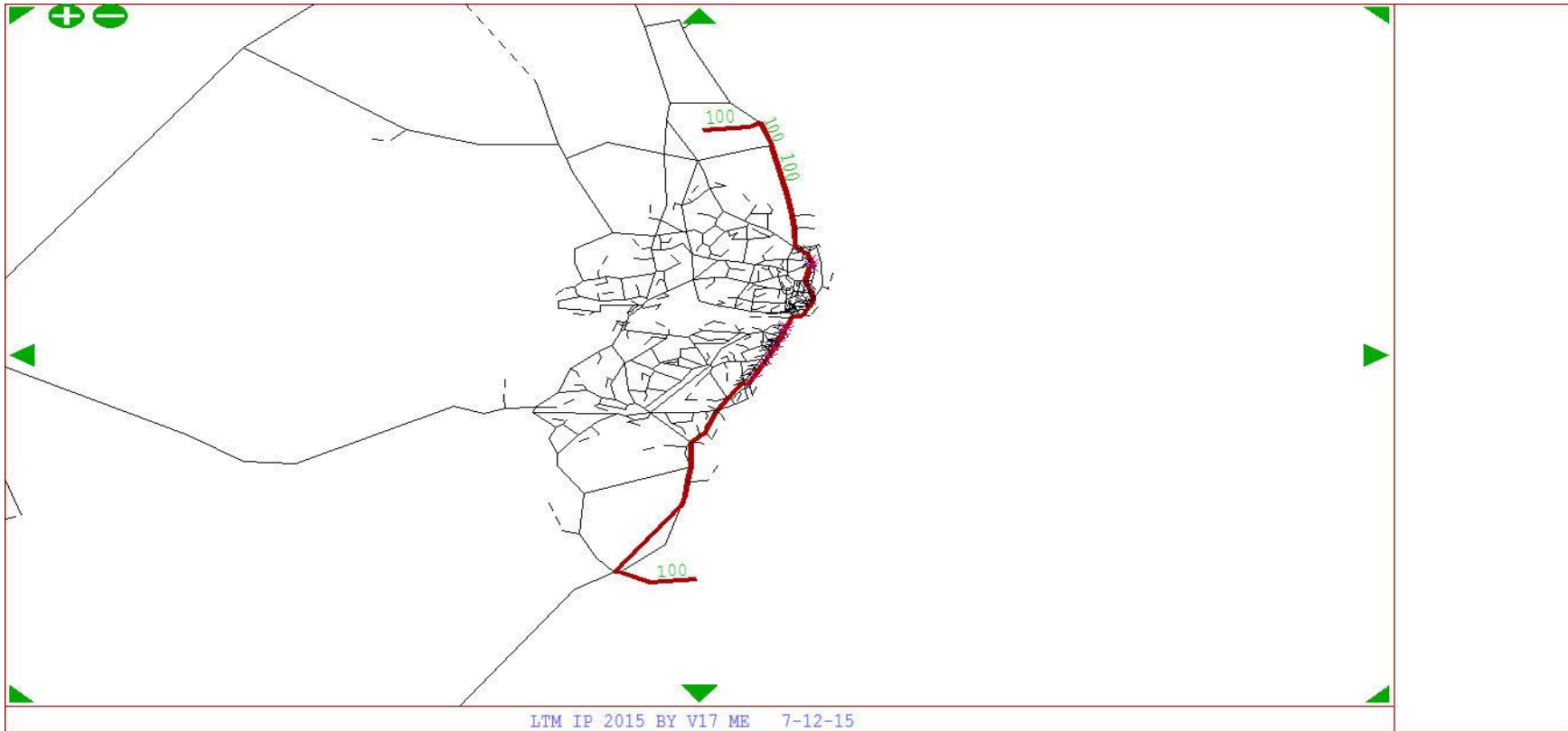


From Zone 102 To Zone 149 - User Class 4



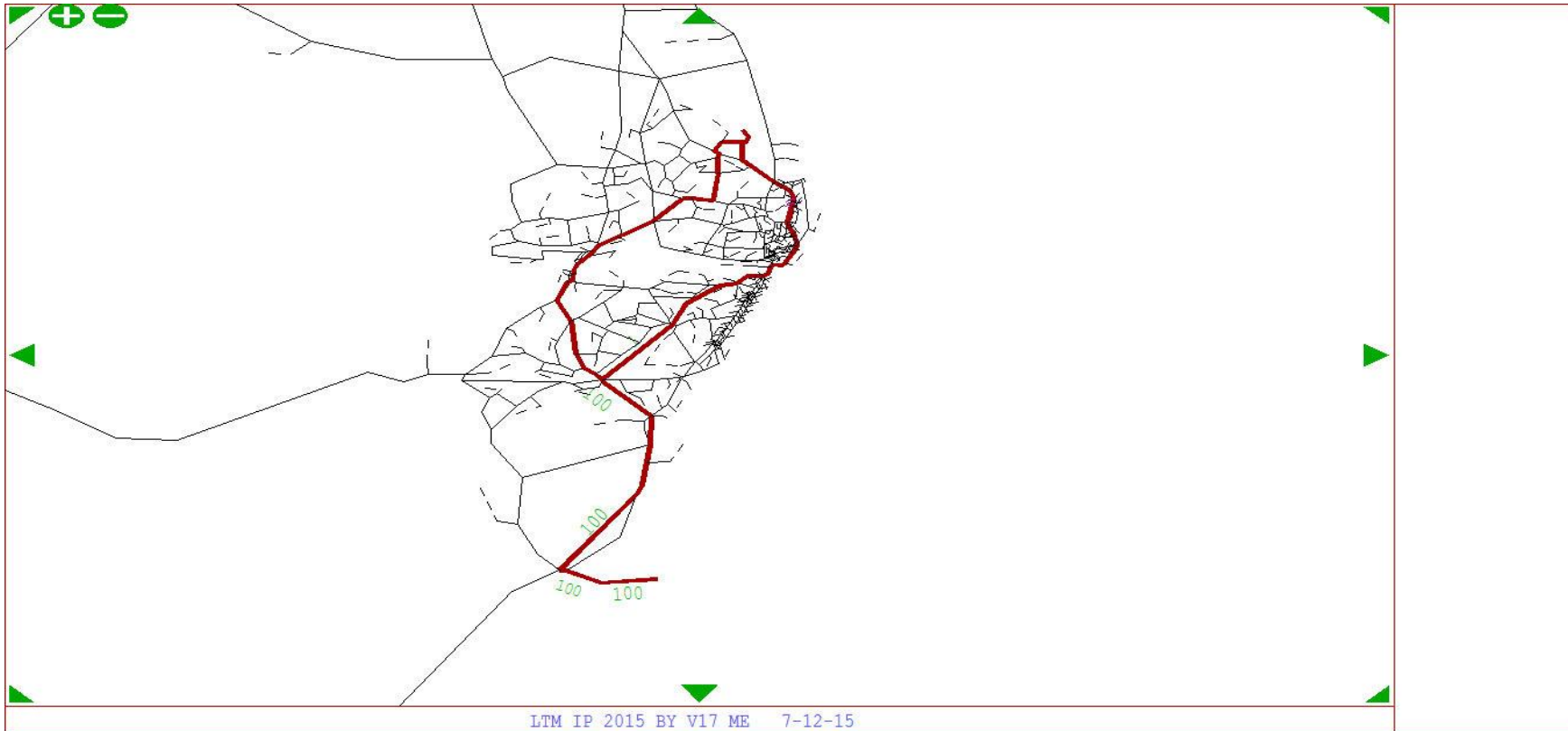
LTM IP 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 149 - User Class 5

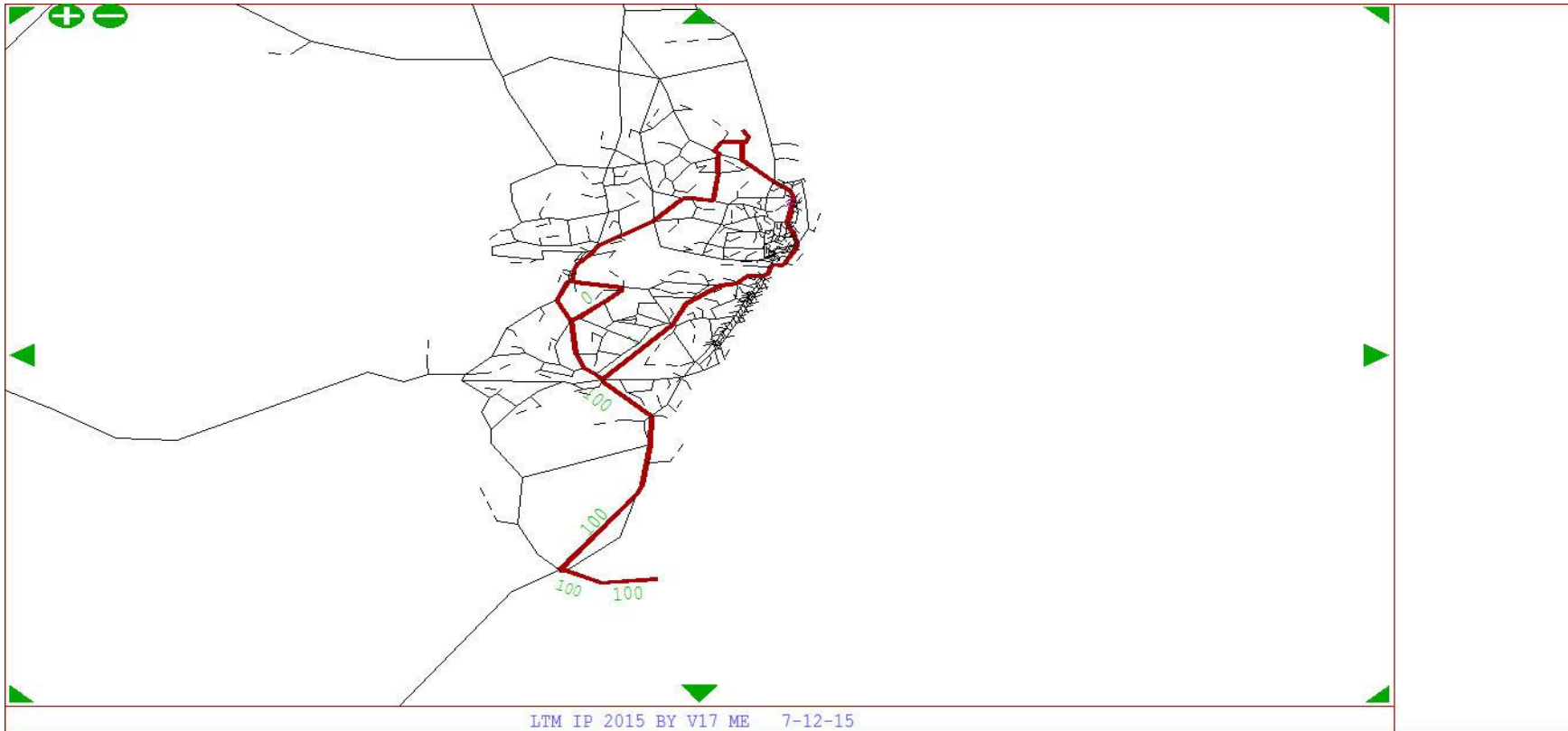


LTM IP 2015 BY V17 ME 7-12-15

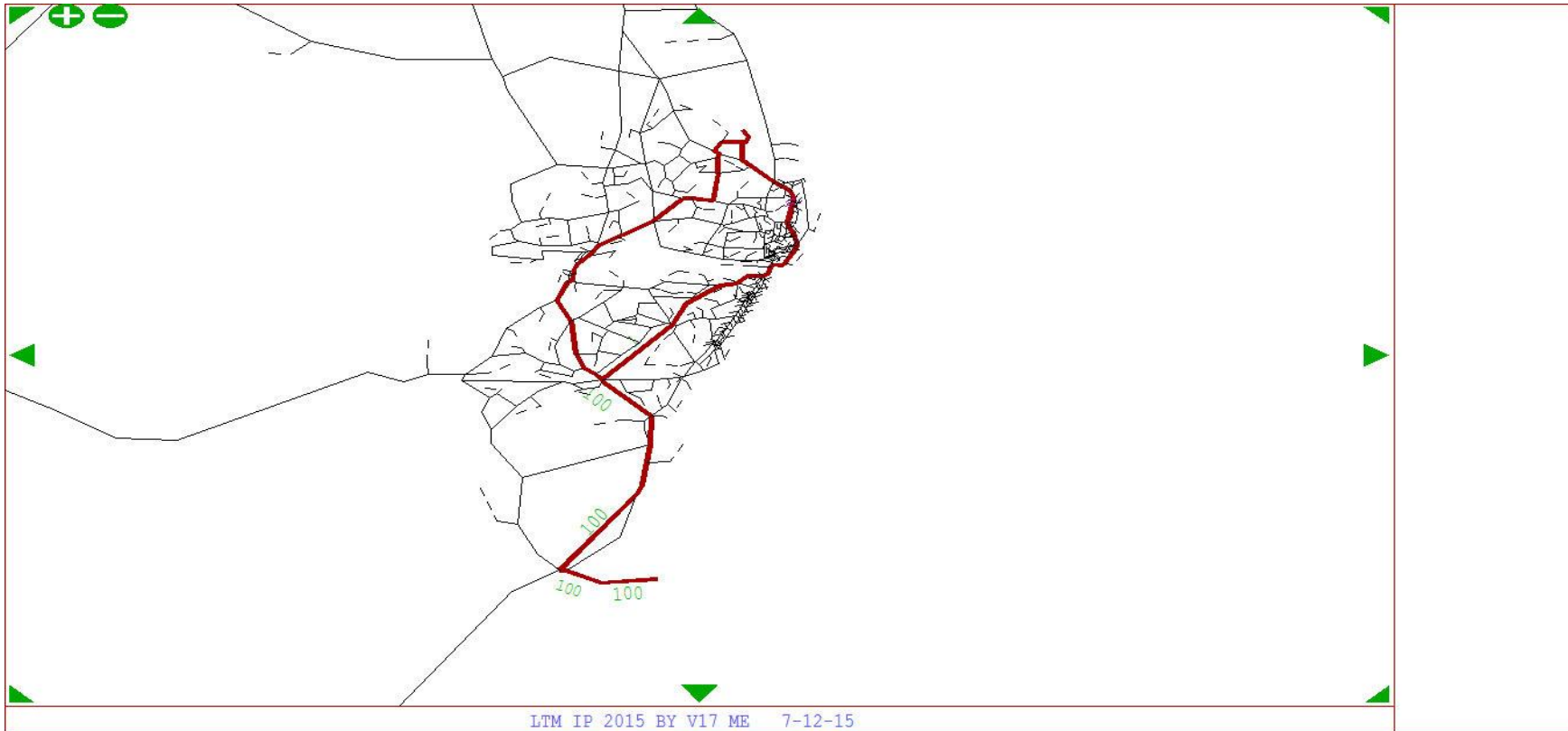
From Zone 149 To Zone 101 - User Class 1



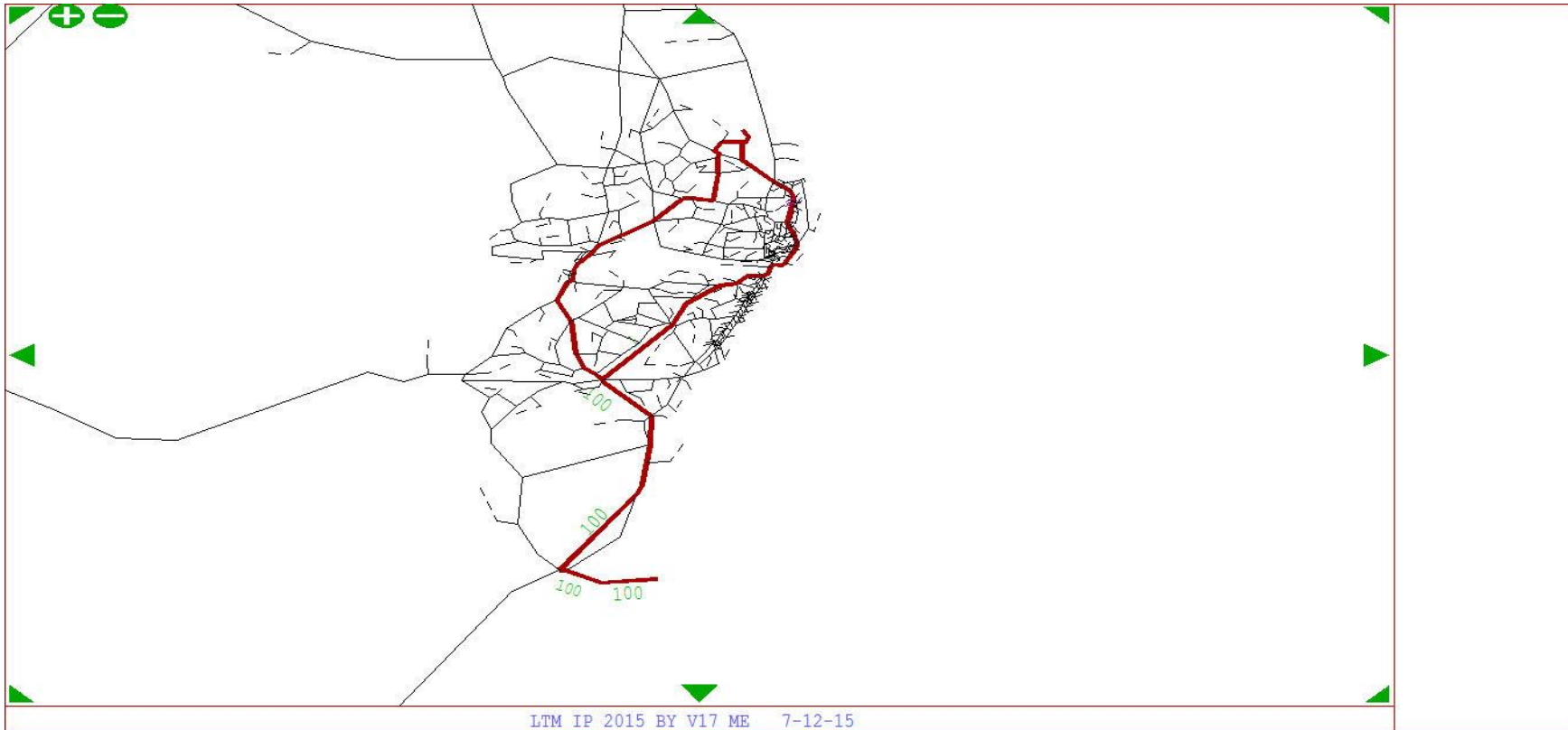
From Zone 149 To Zone 101 - User Class 3



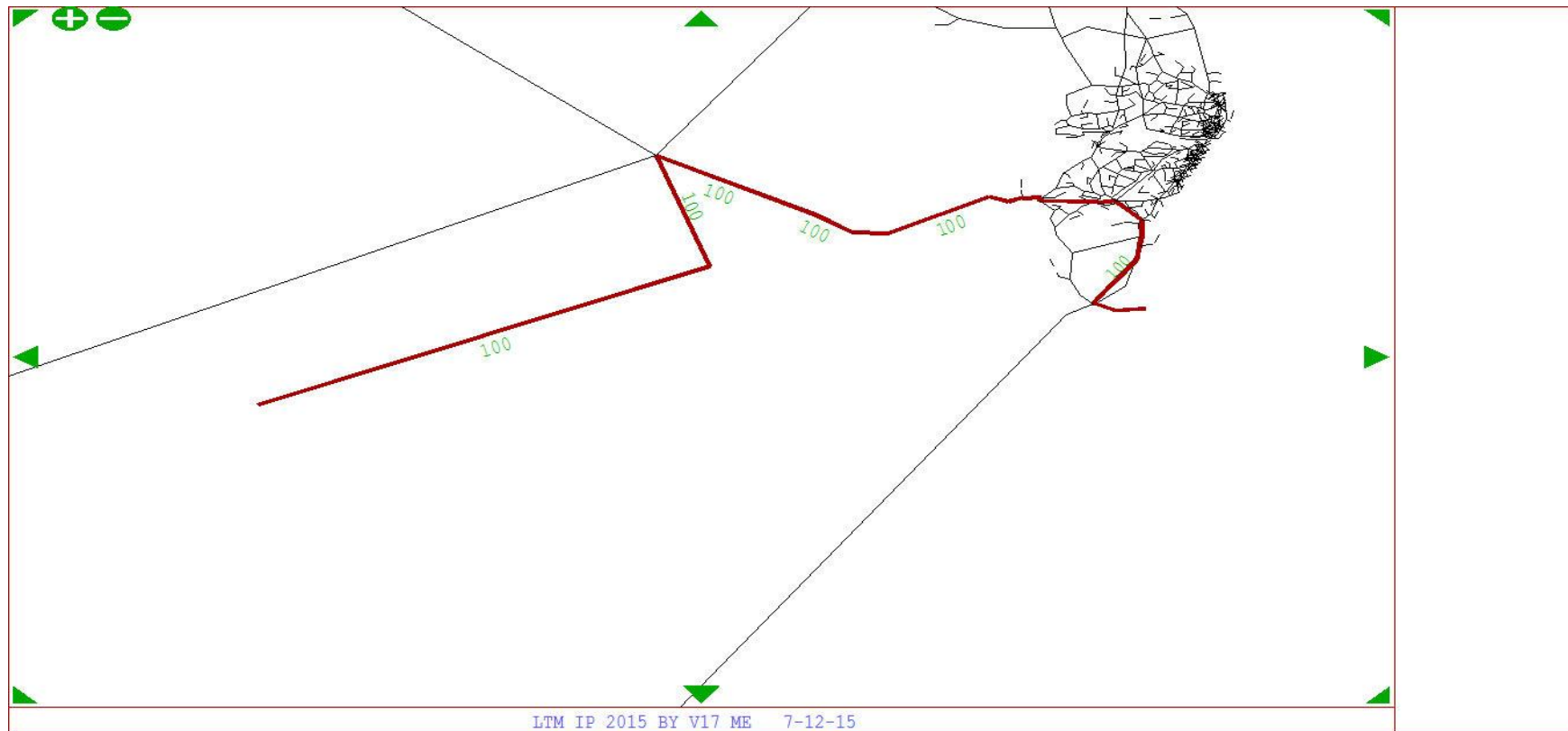
From Zone 149 To Zone 101 - User Class 4



From Zone 149 To Zone 101 - User Class 5

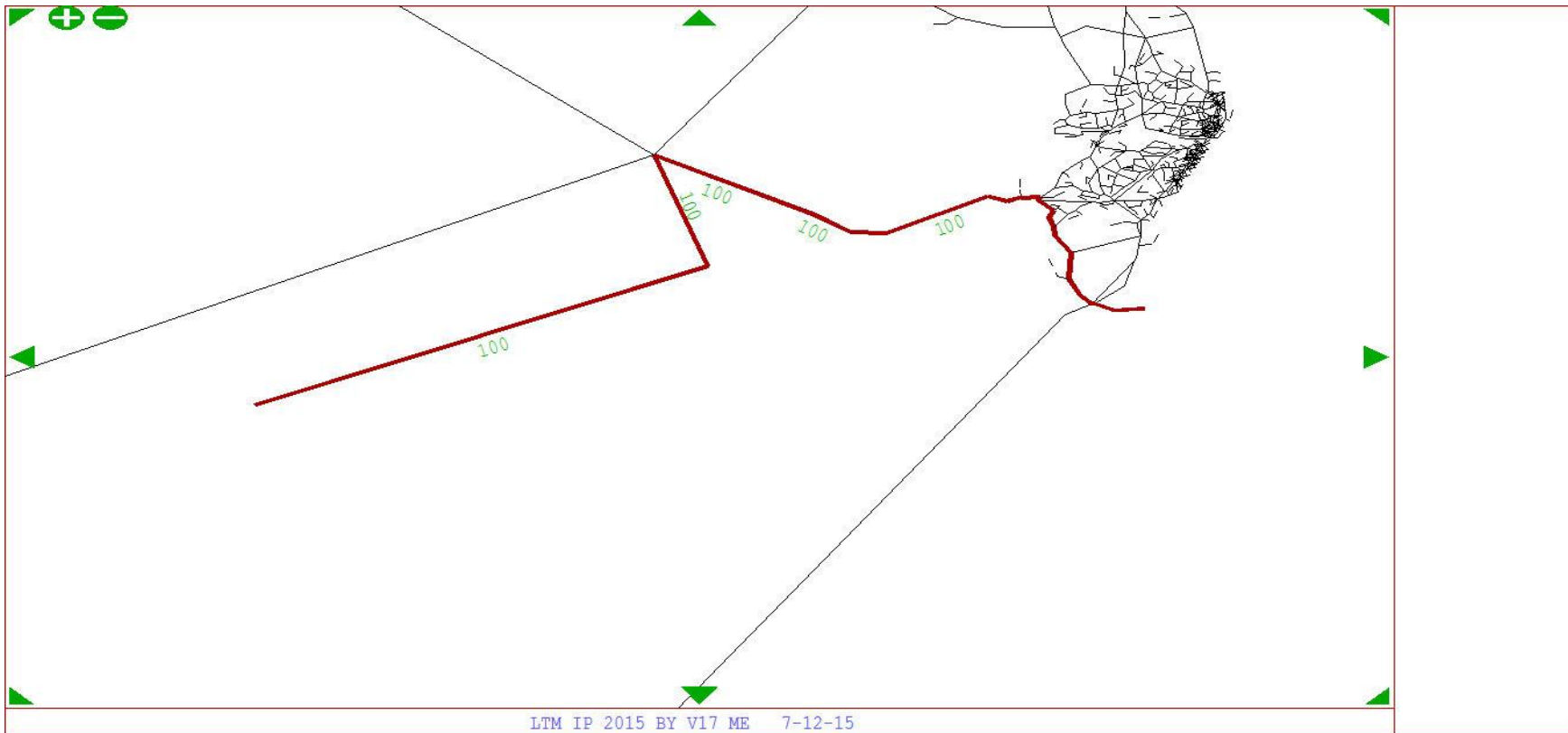


From Zone 149 To Zone 104 - User Class 1

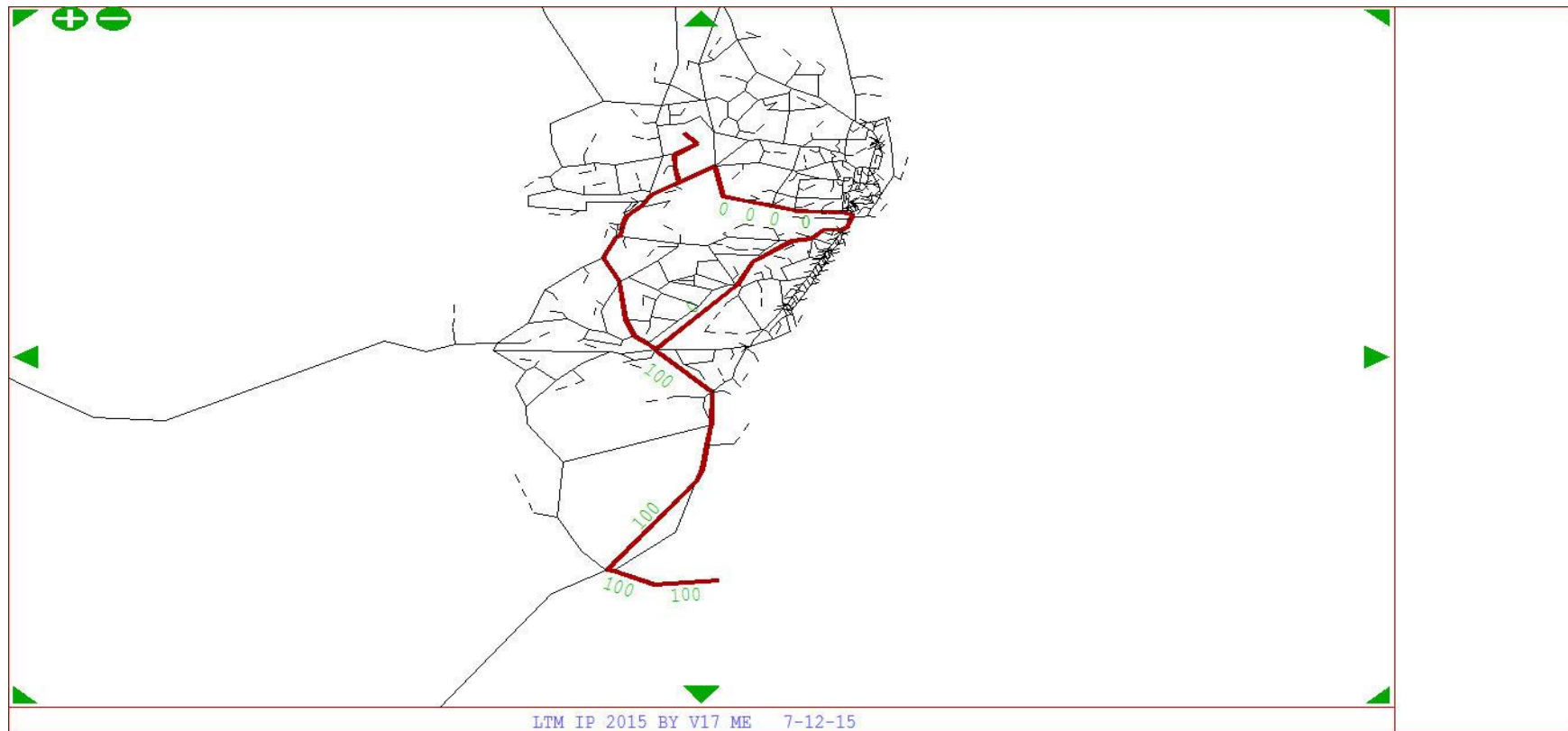


LTM IP 2015 BY V17 ME 7-12-15

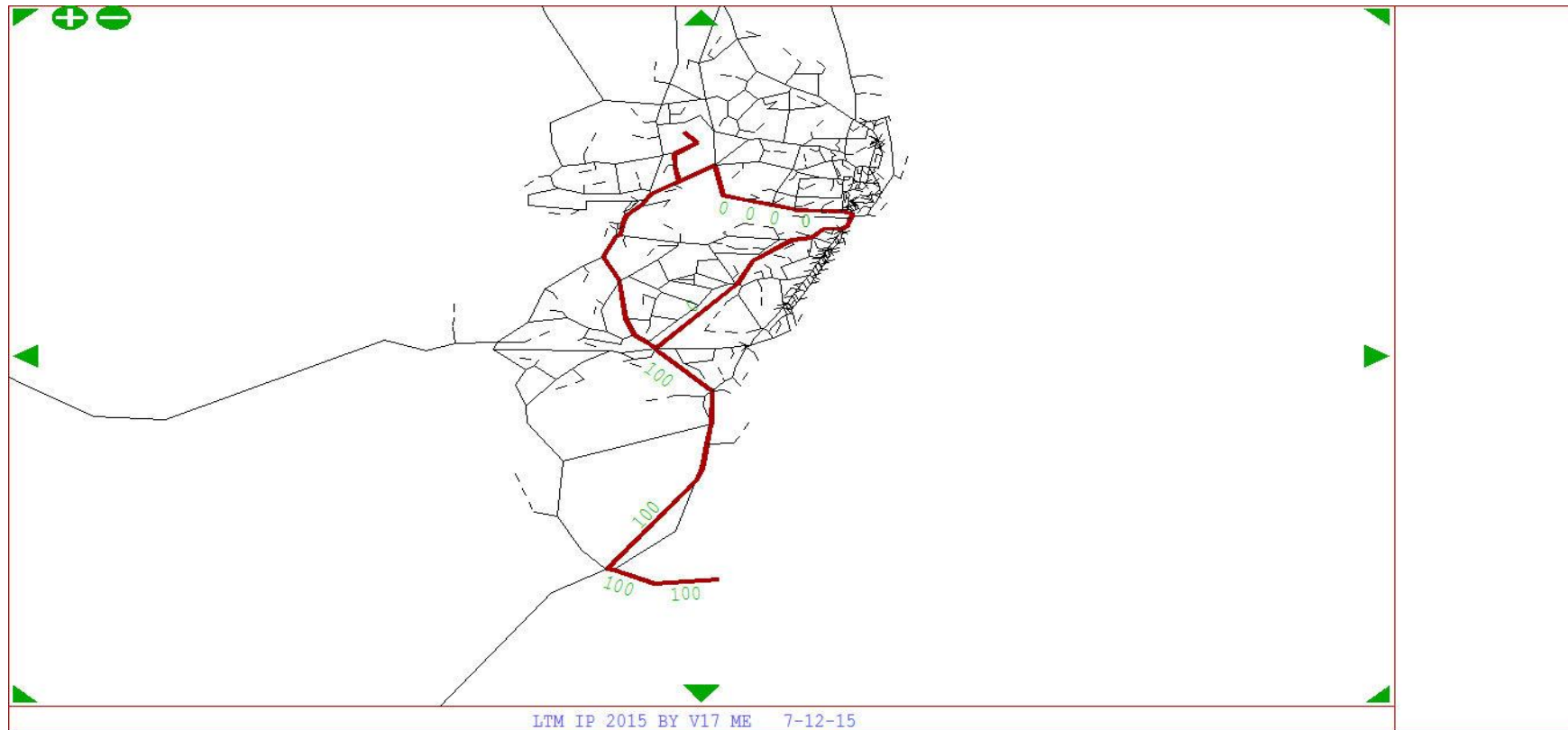
From Zone 149 To Zone 104 - User Class 5



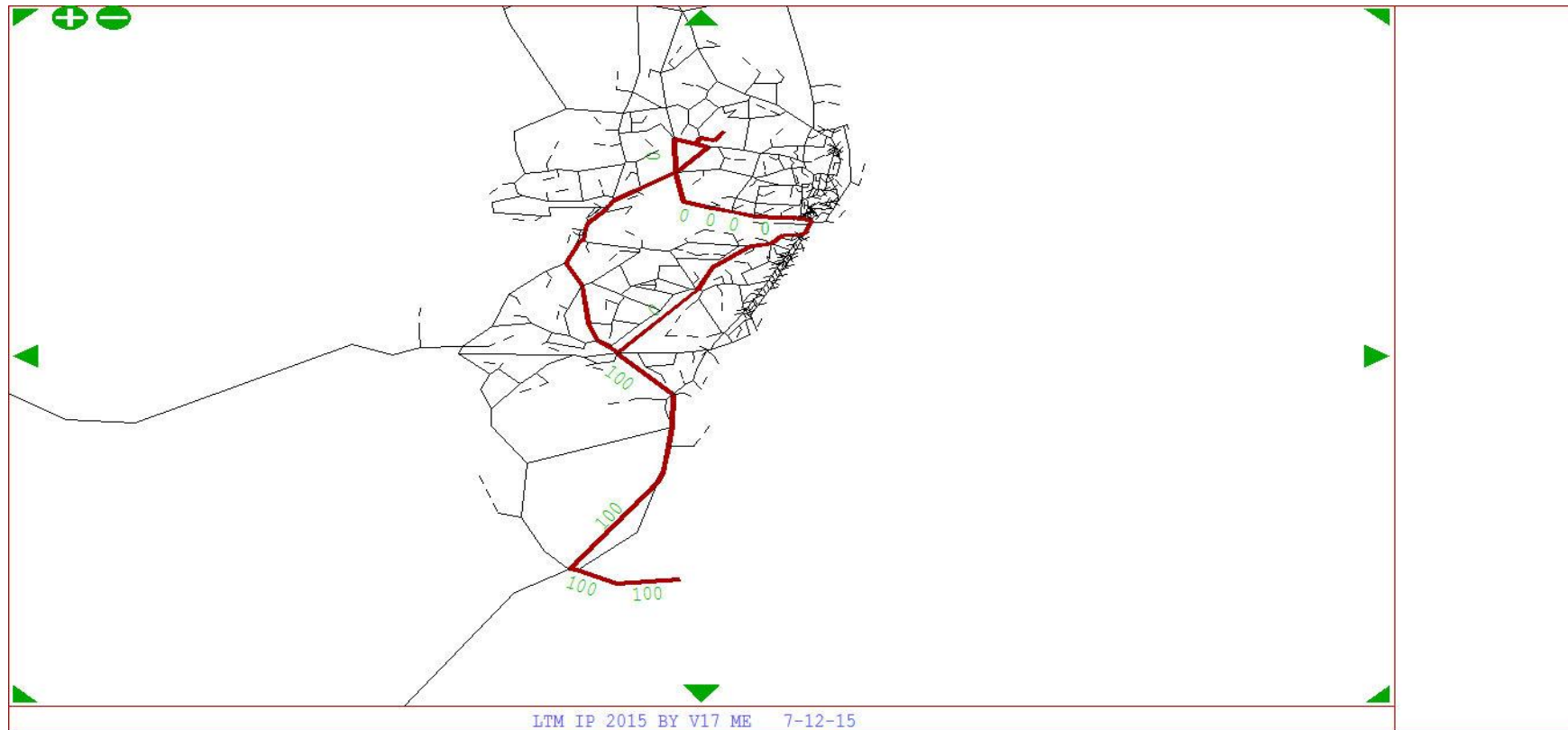
From Zone 149 To Zone 113 - User Class 4



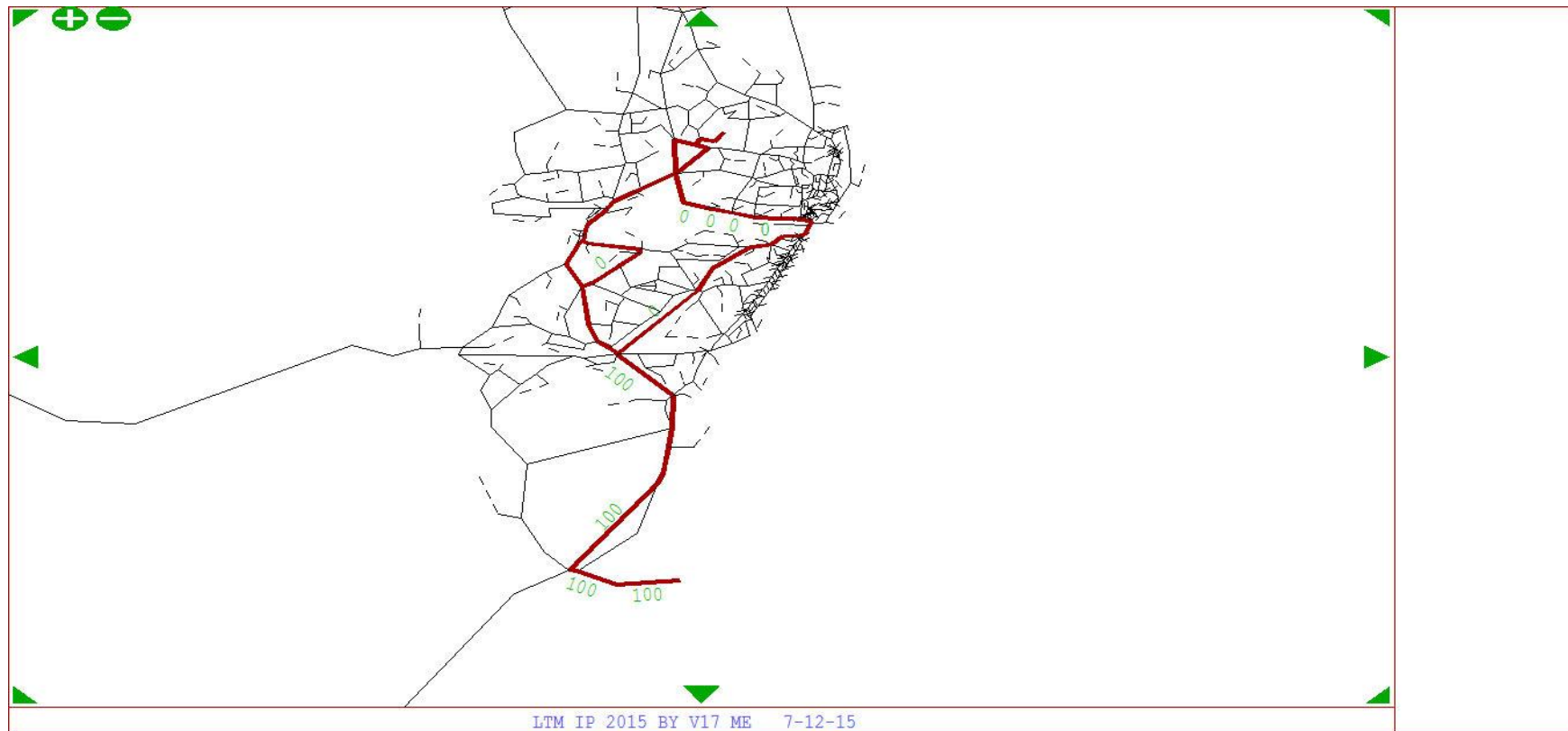
From Zone 149 To Zone 113 - User Class 5



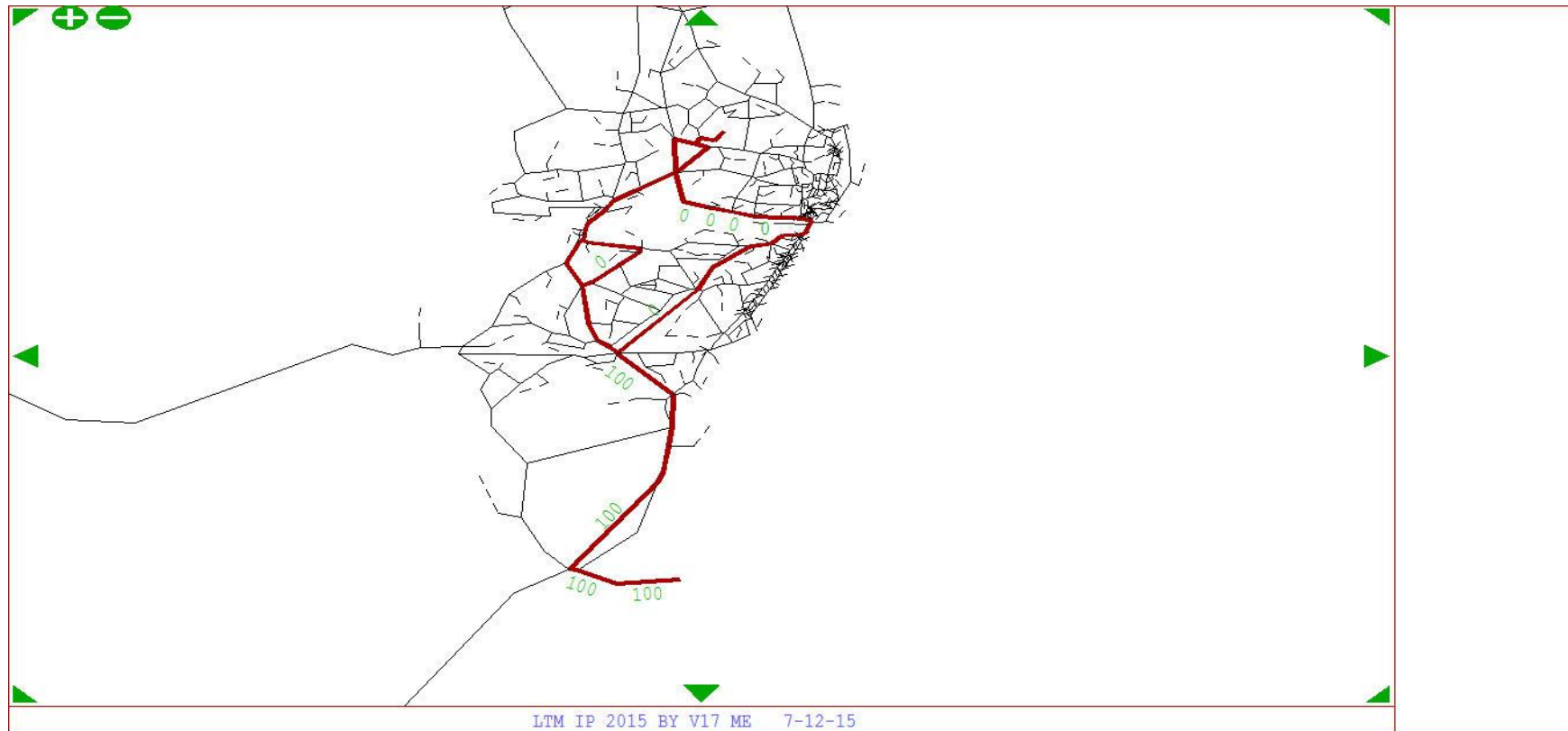
From Zone 149 To Zone 114 - User Class 1



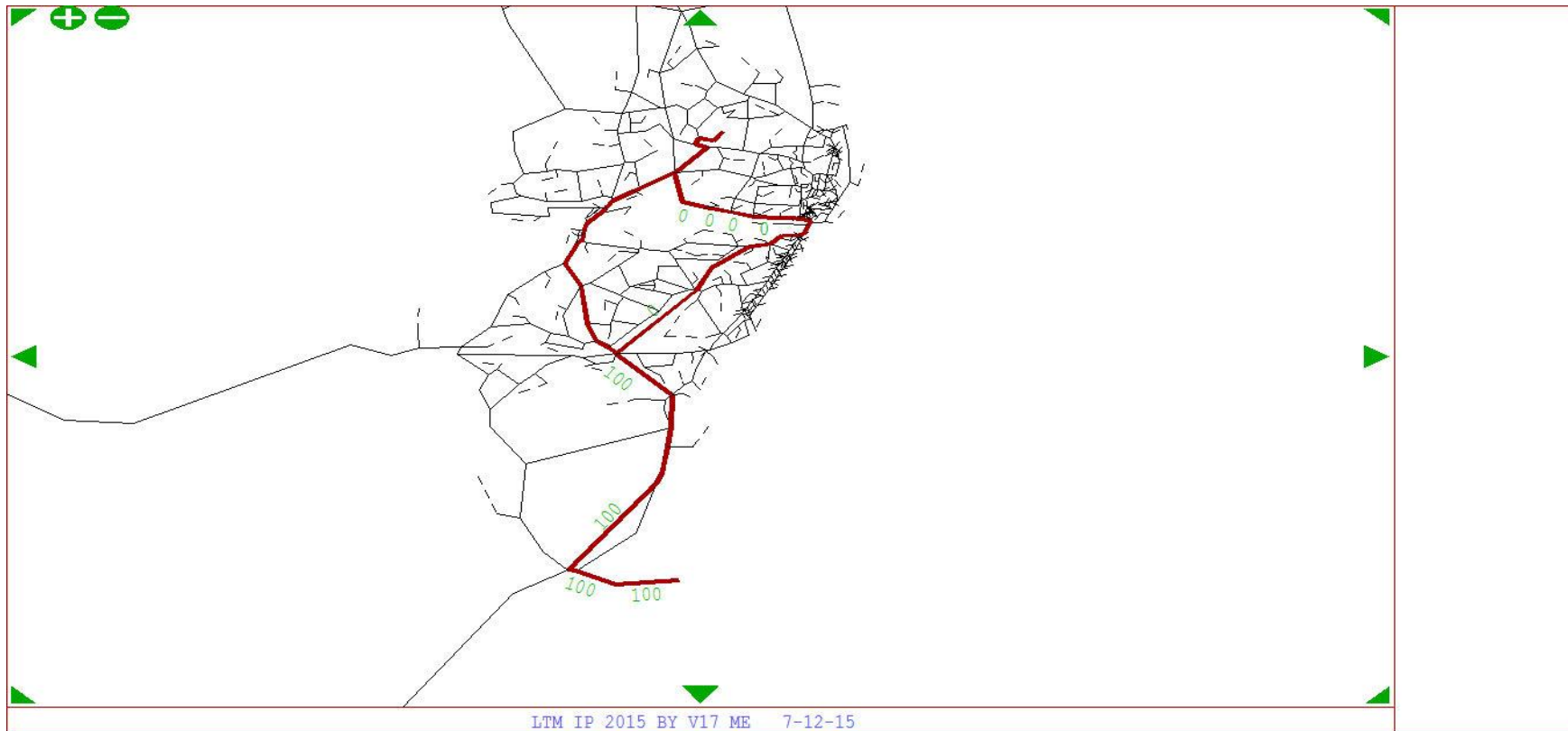
From Zone 149 To Zone 114 - User Class 2



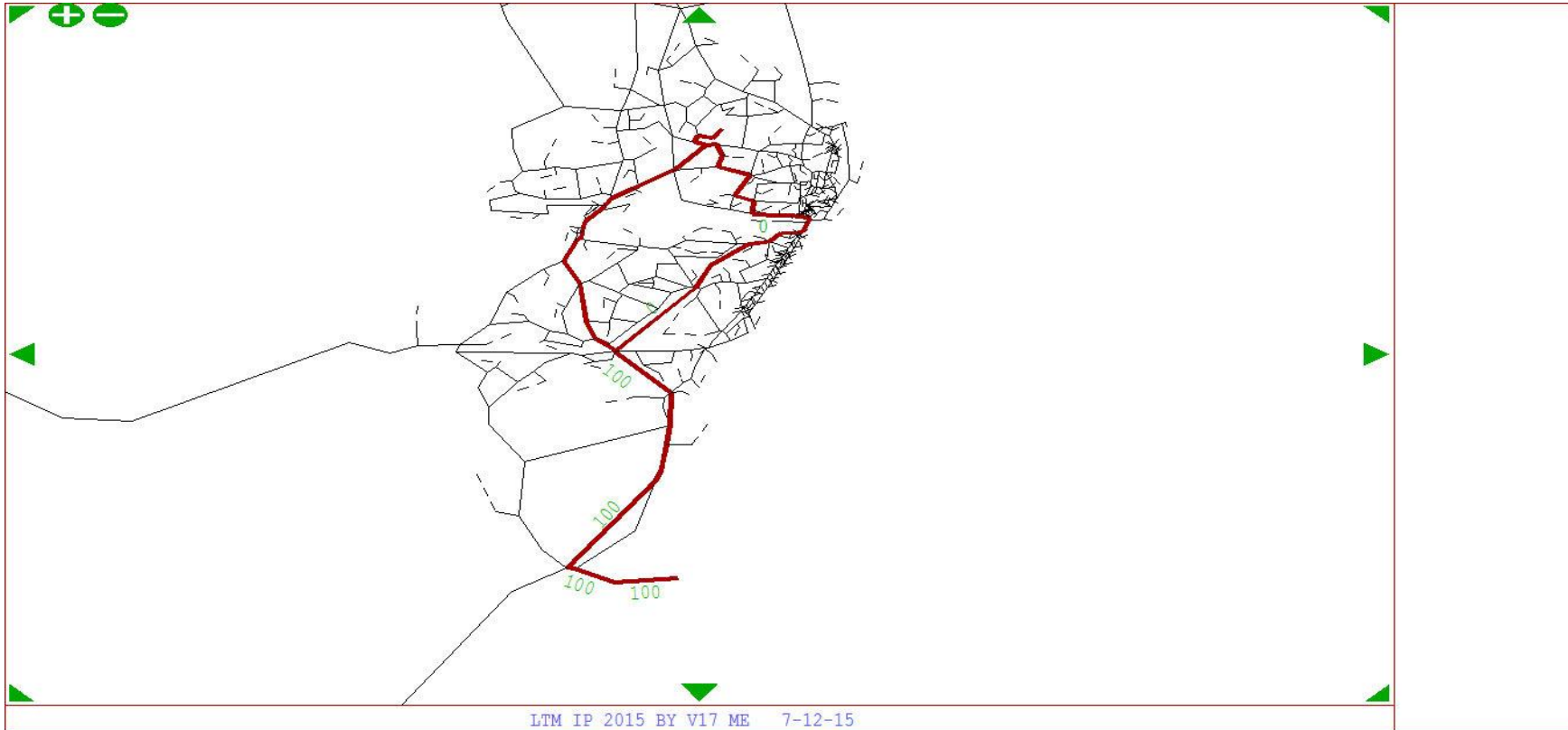
From Zone 149 To Zone 114 - User Class 3



From Zone 149 To Zone 114 - User Class 4

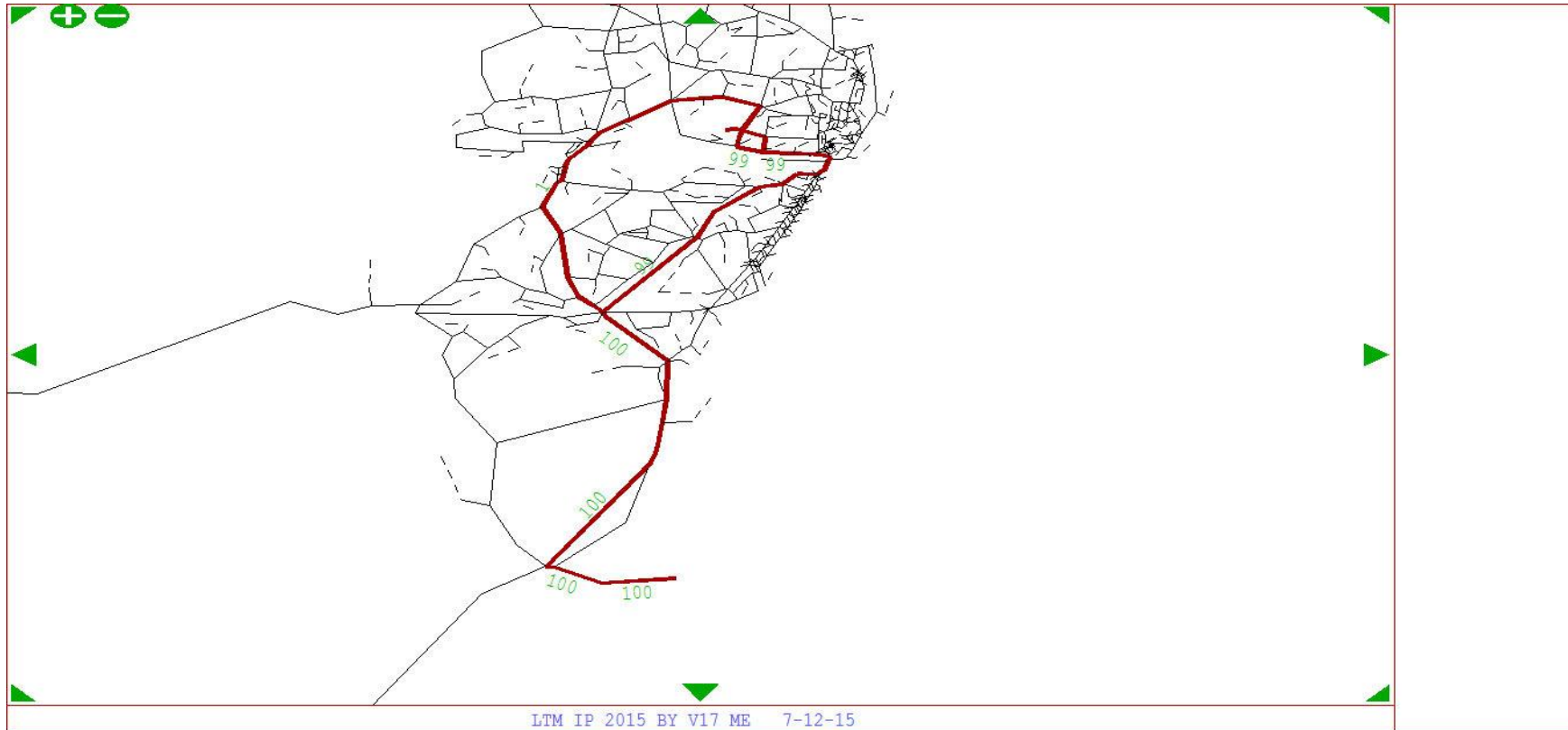


From Zone 149 To Zone 114 - User Class 5

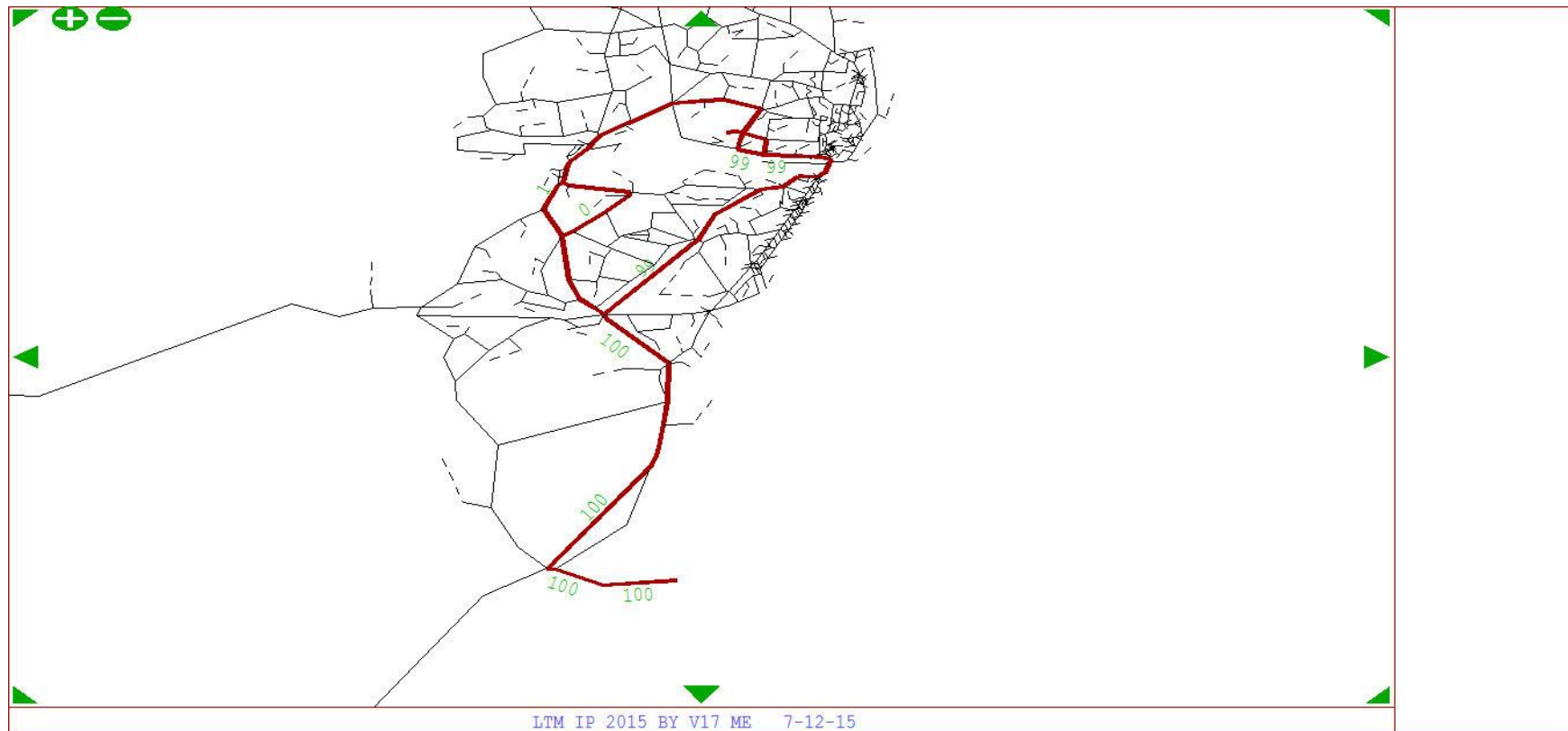


LTM IP 2015 BY V17 ME 7-12-15

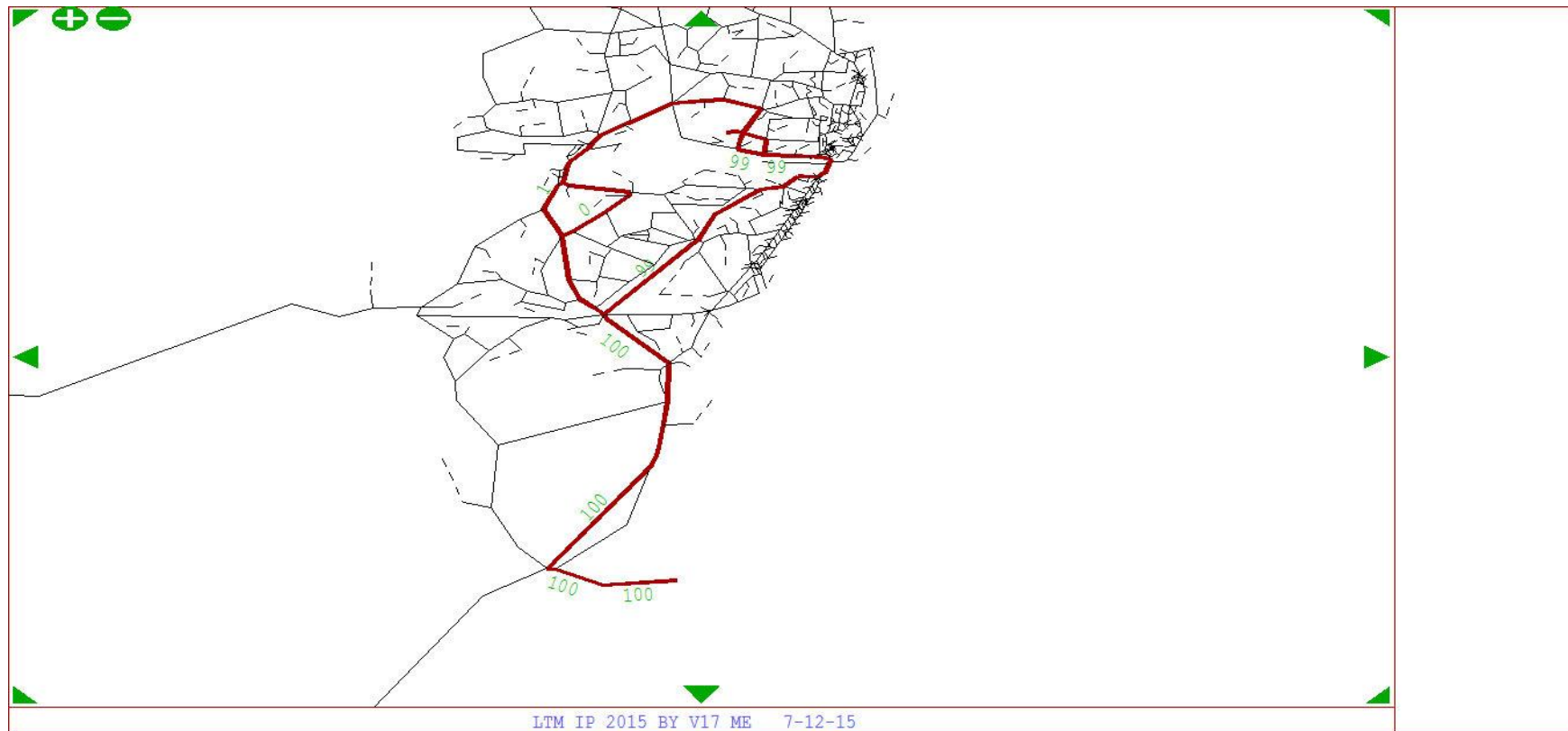
From Zone 149 To Zone 120 - User Class 1



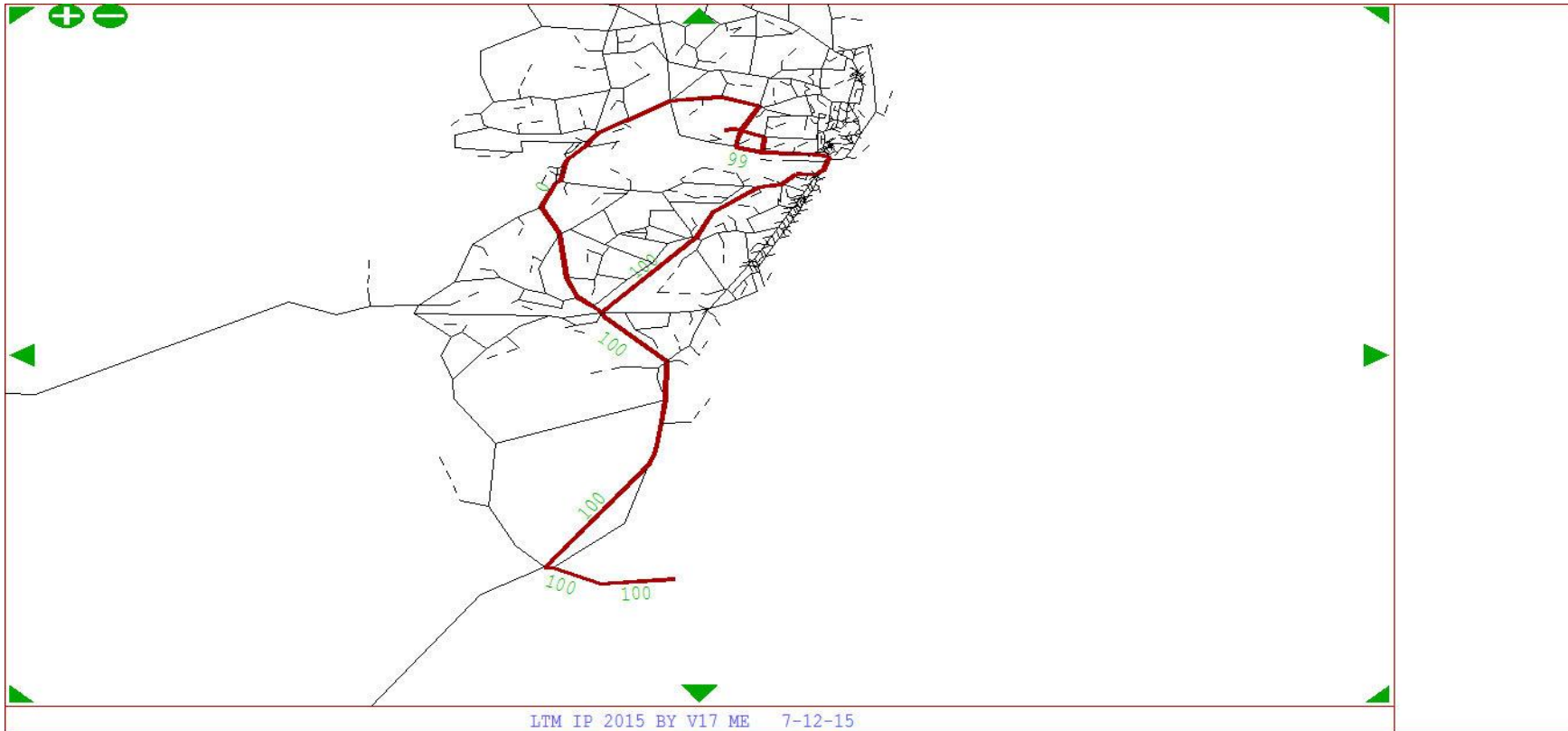
From Zone 149 To Zone 120 - User Class 2



From Zone 149 To Zone 120 - User Class 3

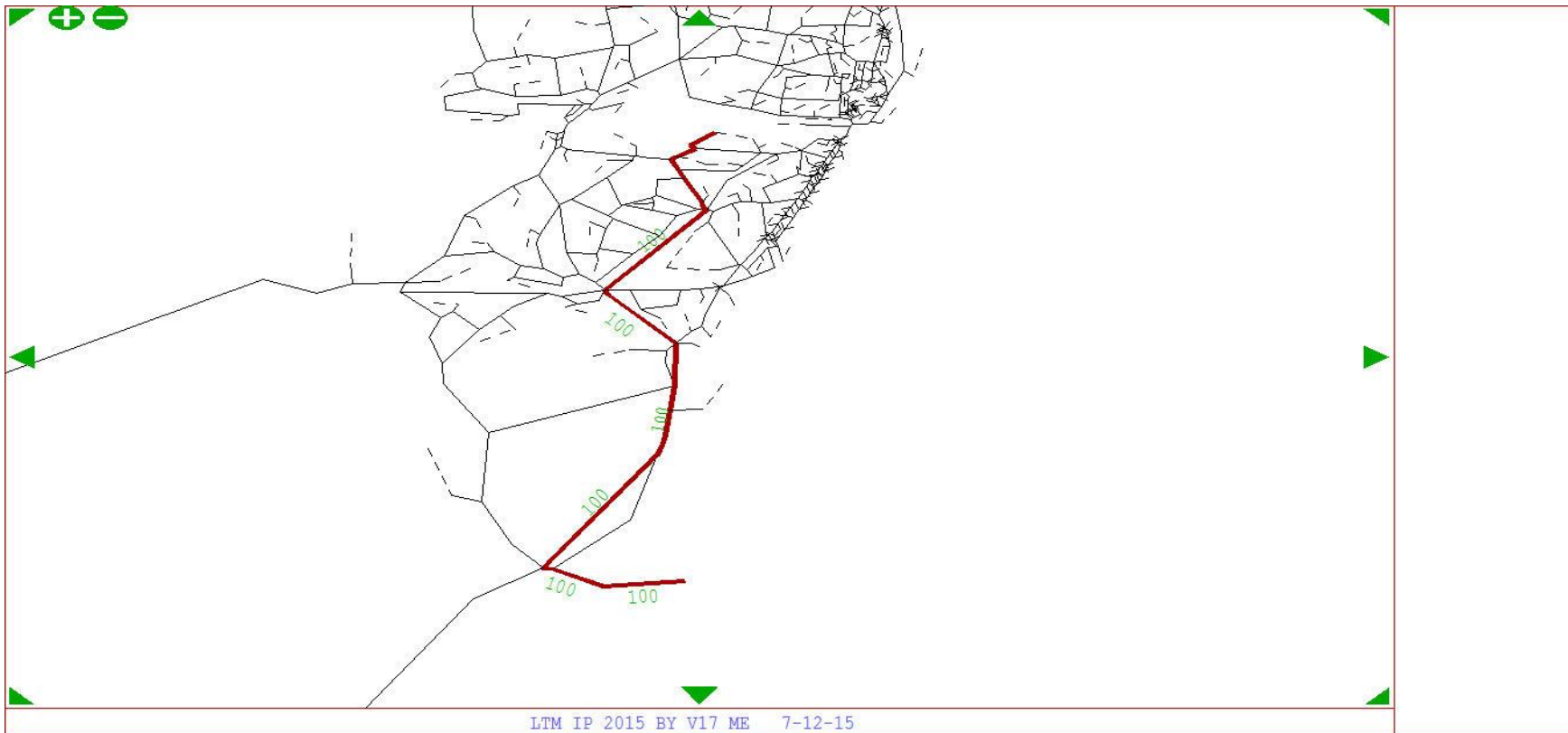


From Zone 149 To Zone 120 - User Class 4



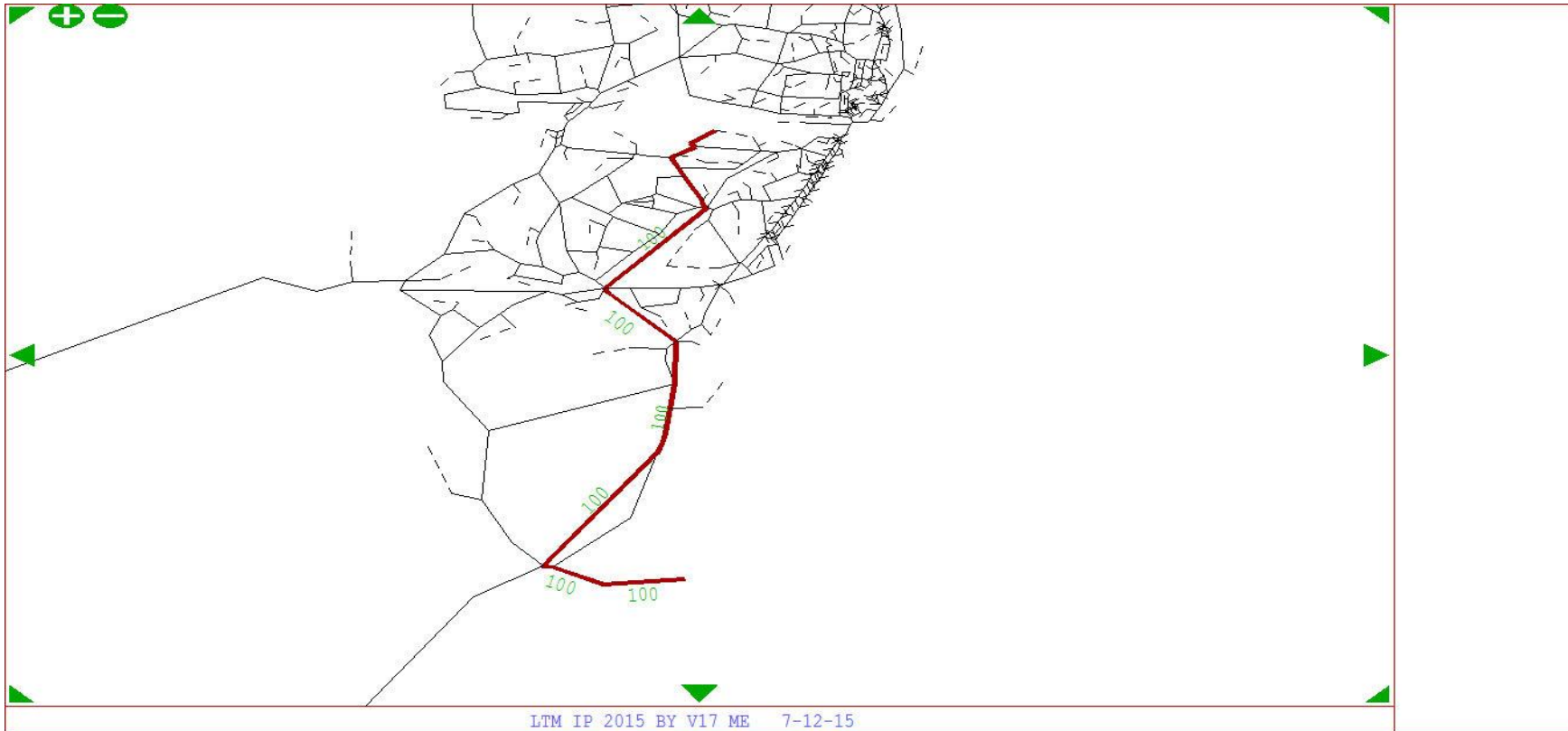
LTM IP 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 122 - User Class 2



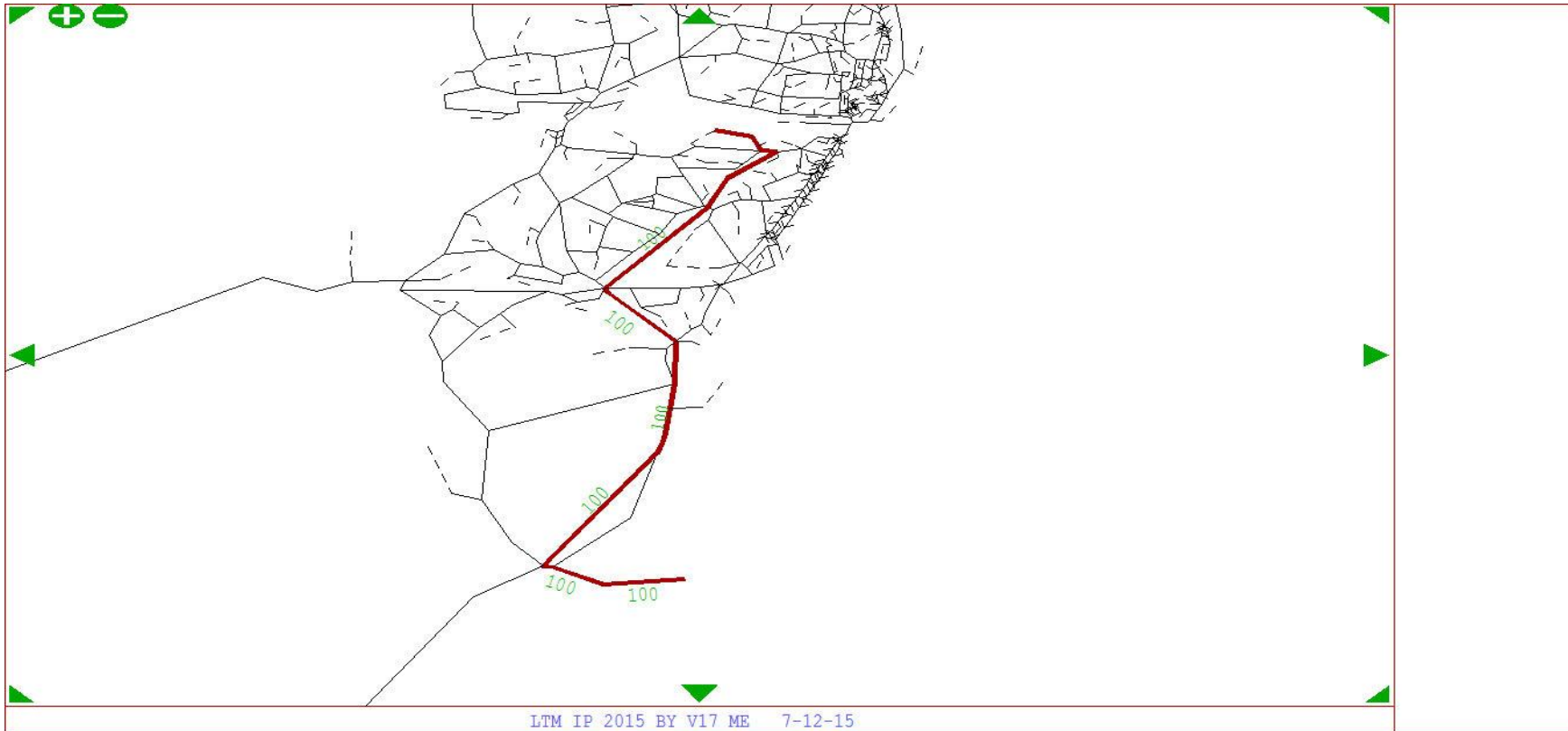
LTM IP 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 122 - User Class 3



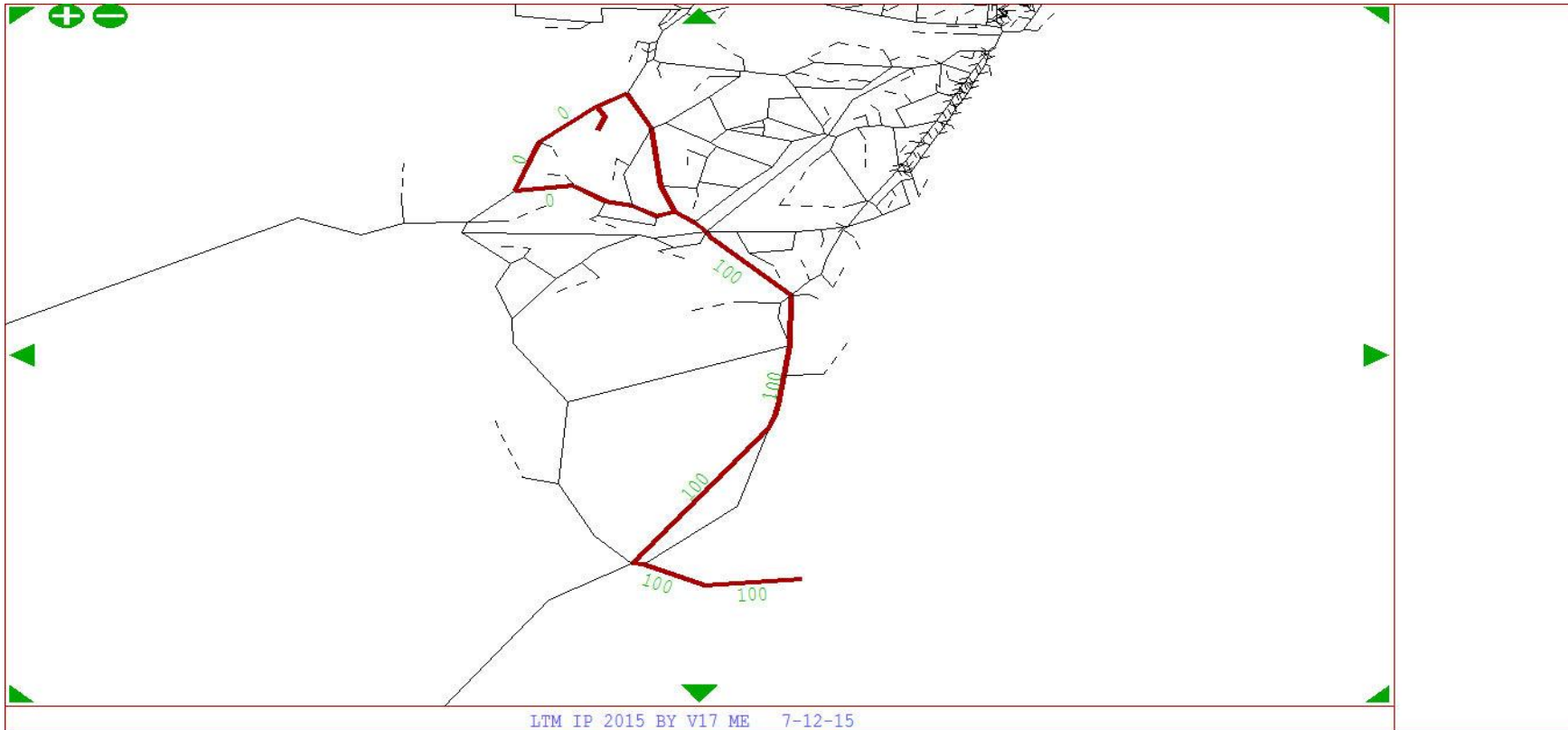
LTM IP 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 122 - User Class 5



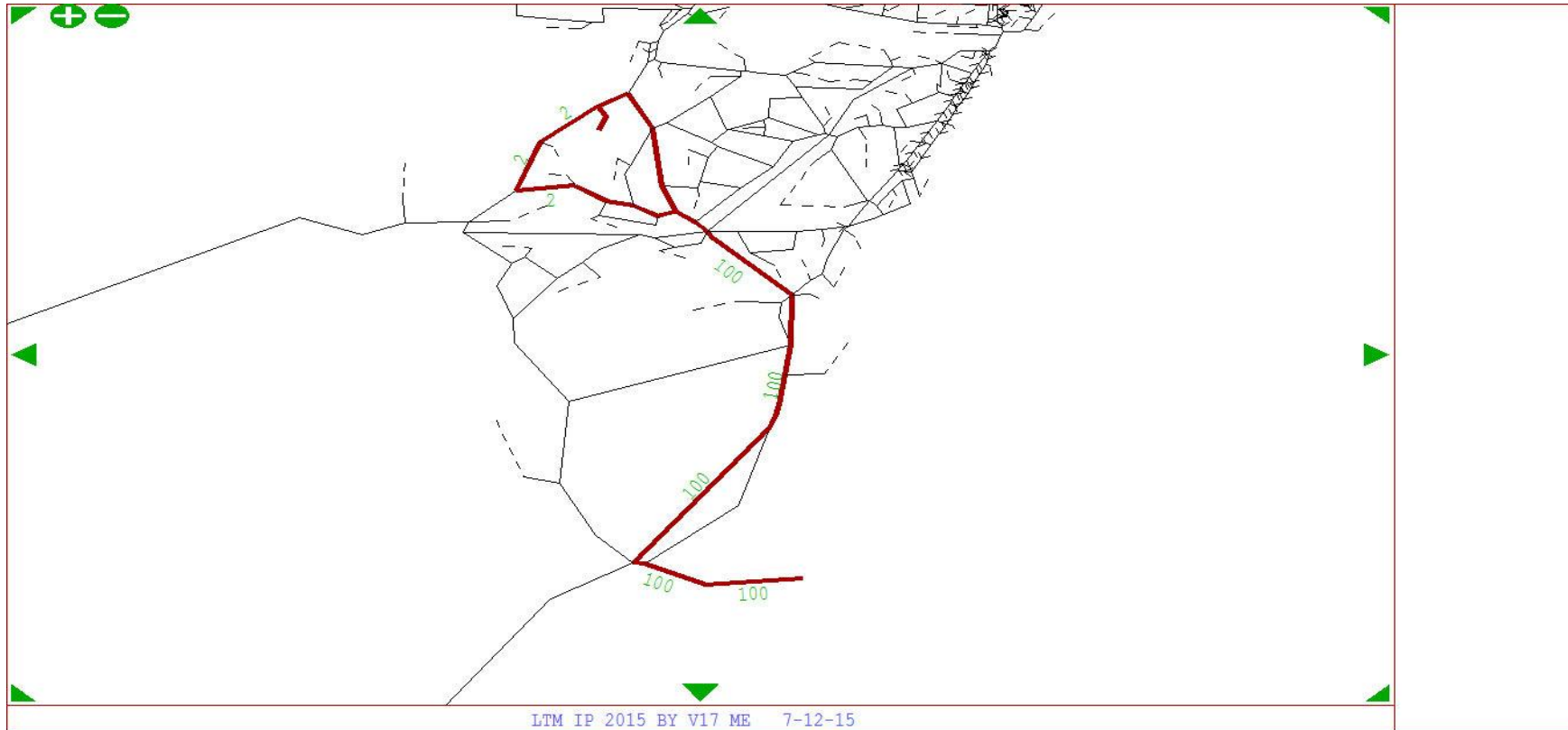
LTM IP 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 128 - User Class 1

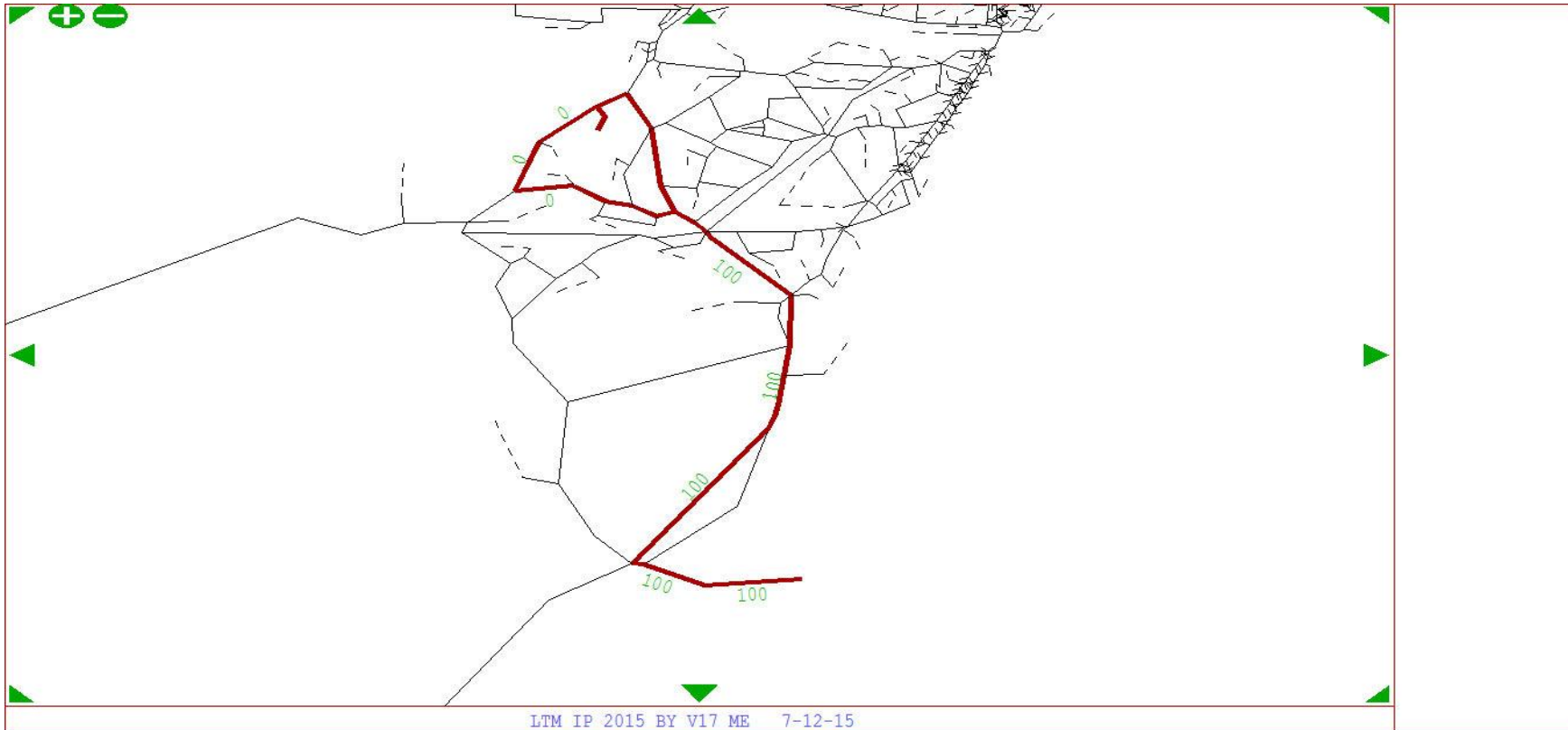


LTM IP 2015 BY V17 ME 7-12-15

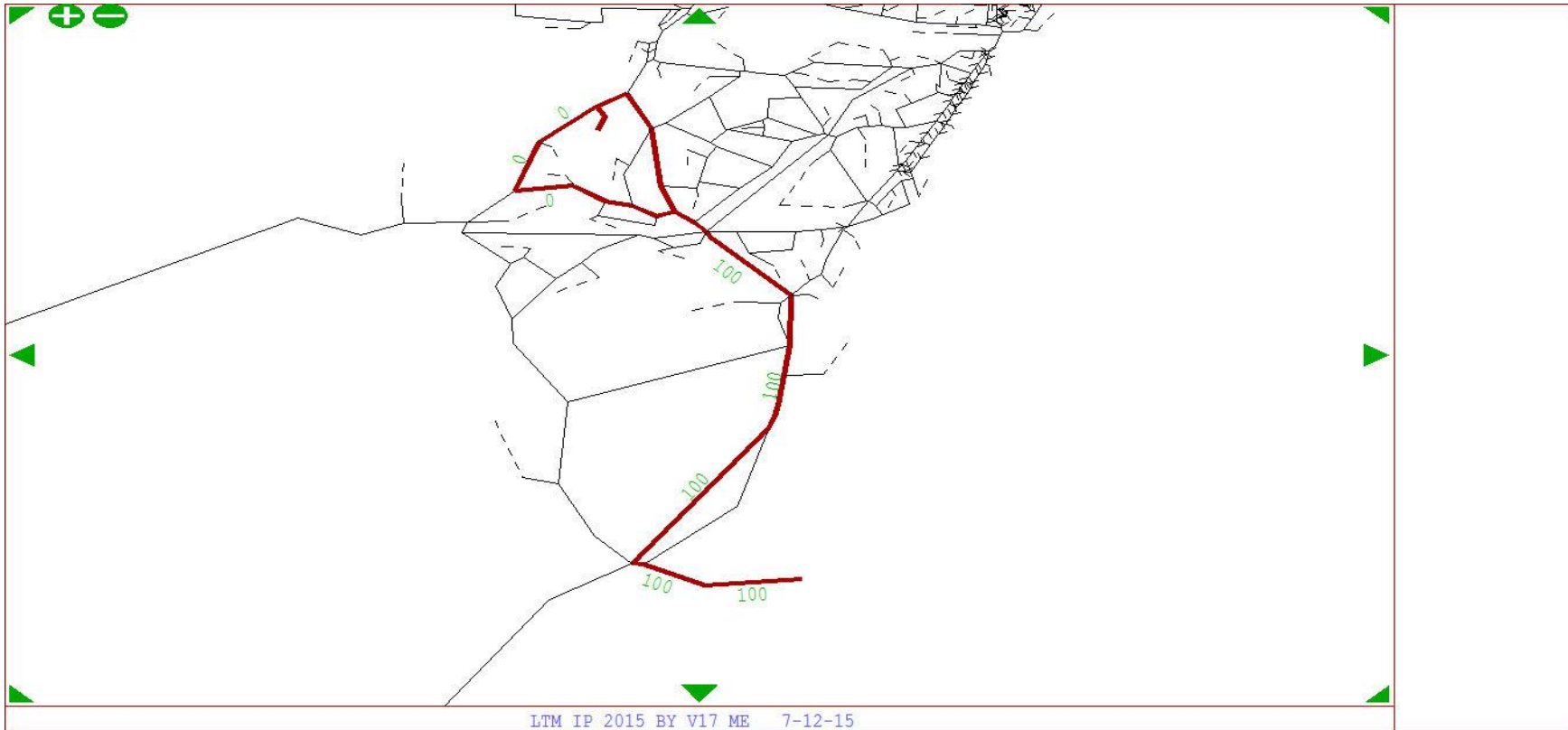
From Zone 149 To Zone 128 - User Class 2



From Zone 149 To Zone 128 - User Class 3

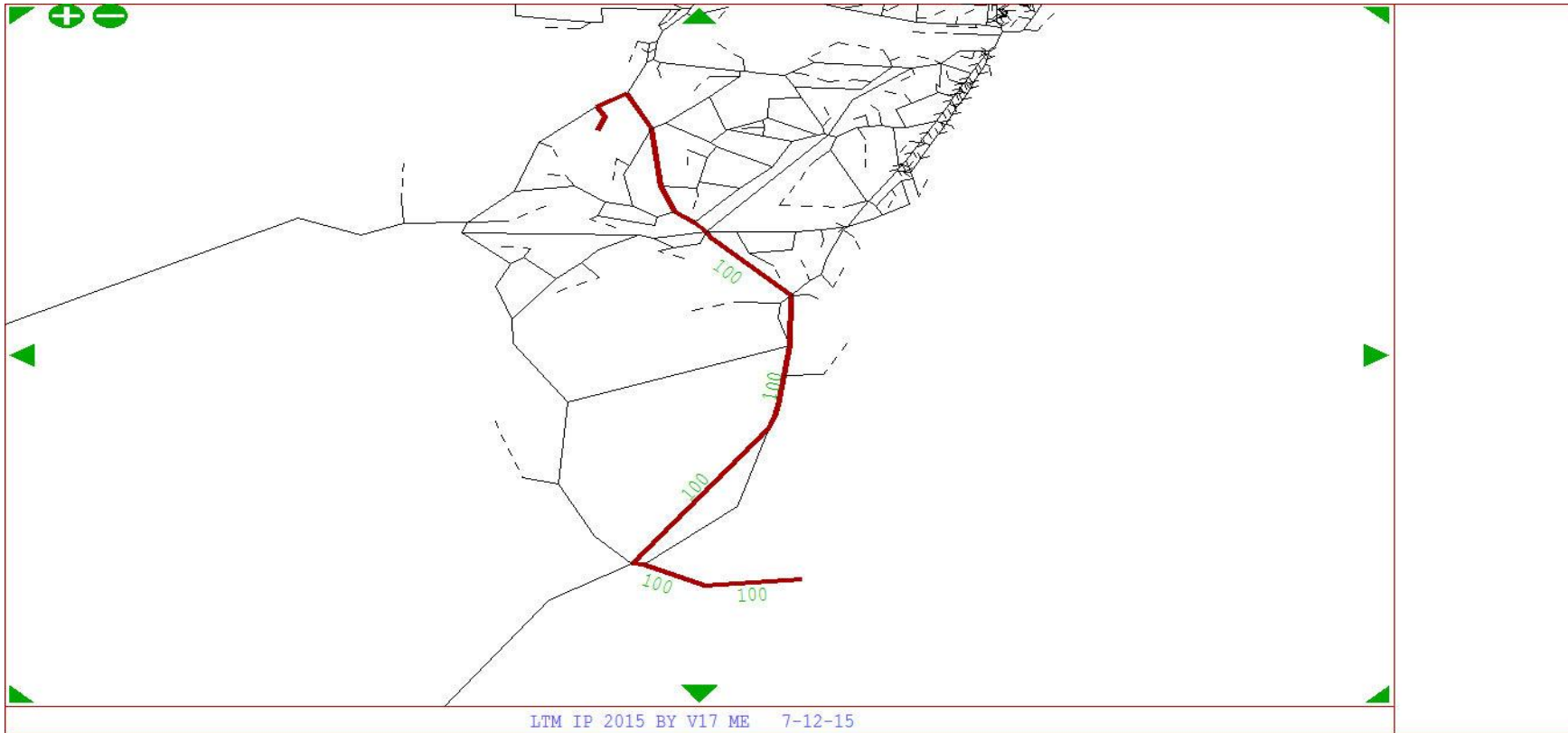


From Zone 149 To Zone 128 - User Class 4



LTM IP 2015 BY V17 ME 7-12-15

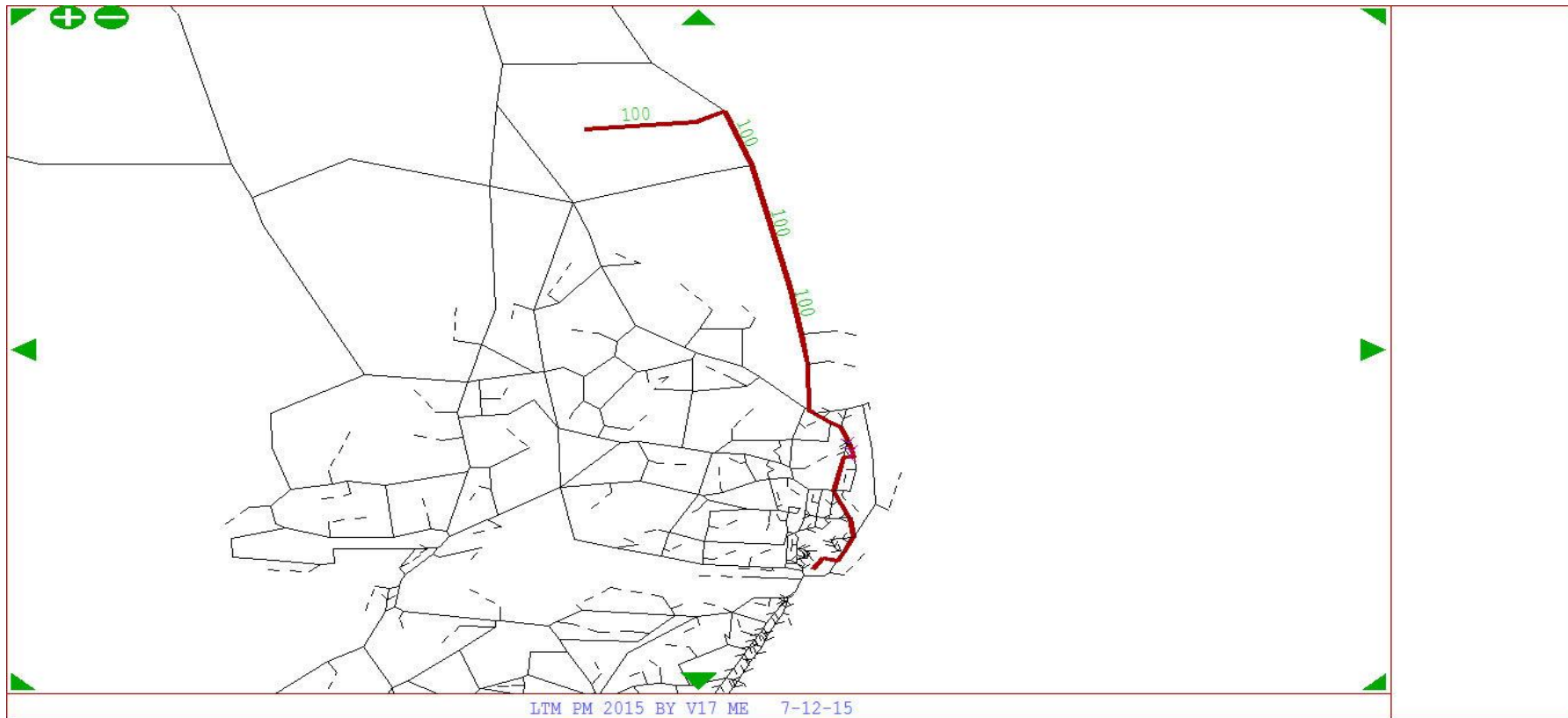
From Zone 149 To Zone 128 - User Class 5



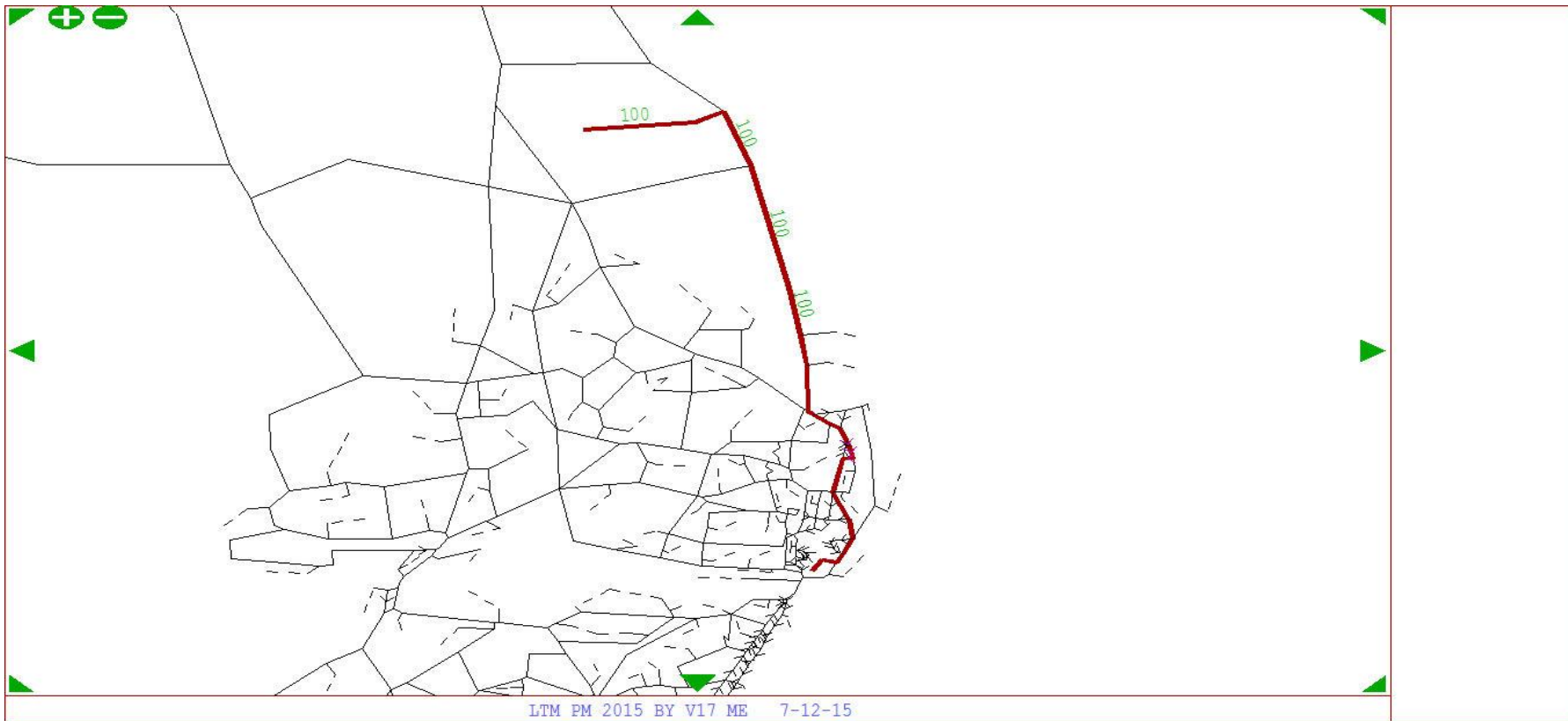
LTM IP 2015 BY V17 ME 7-12-15

Lowestoft: OD Tree Plots - PM Peak

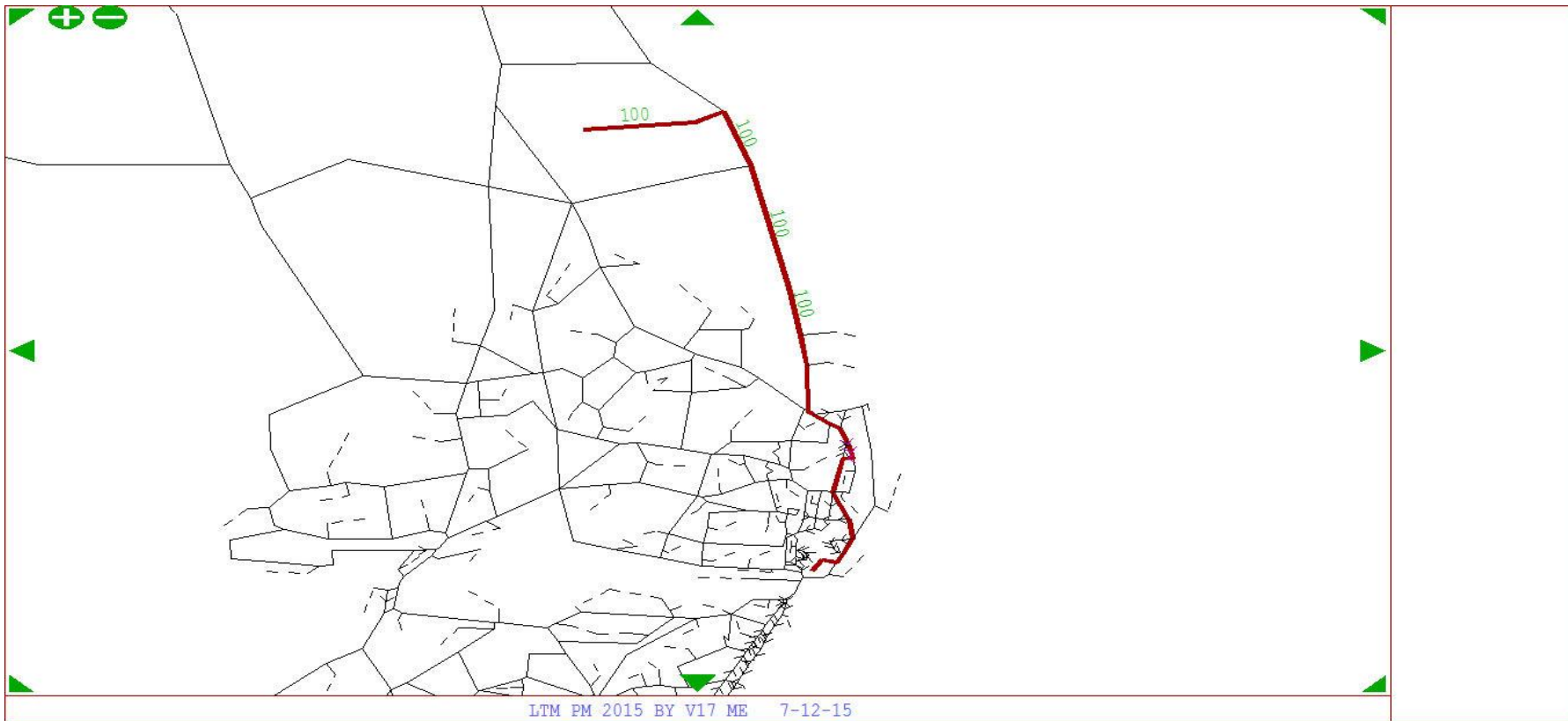
From Zone 102 To Zone 119 - User Class 1



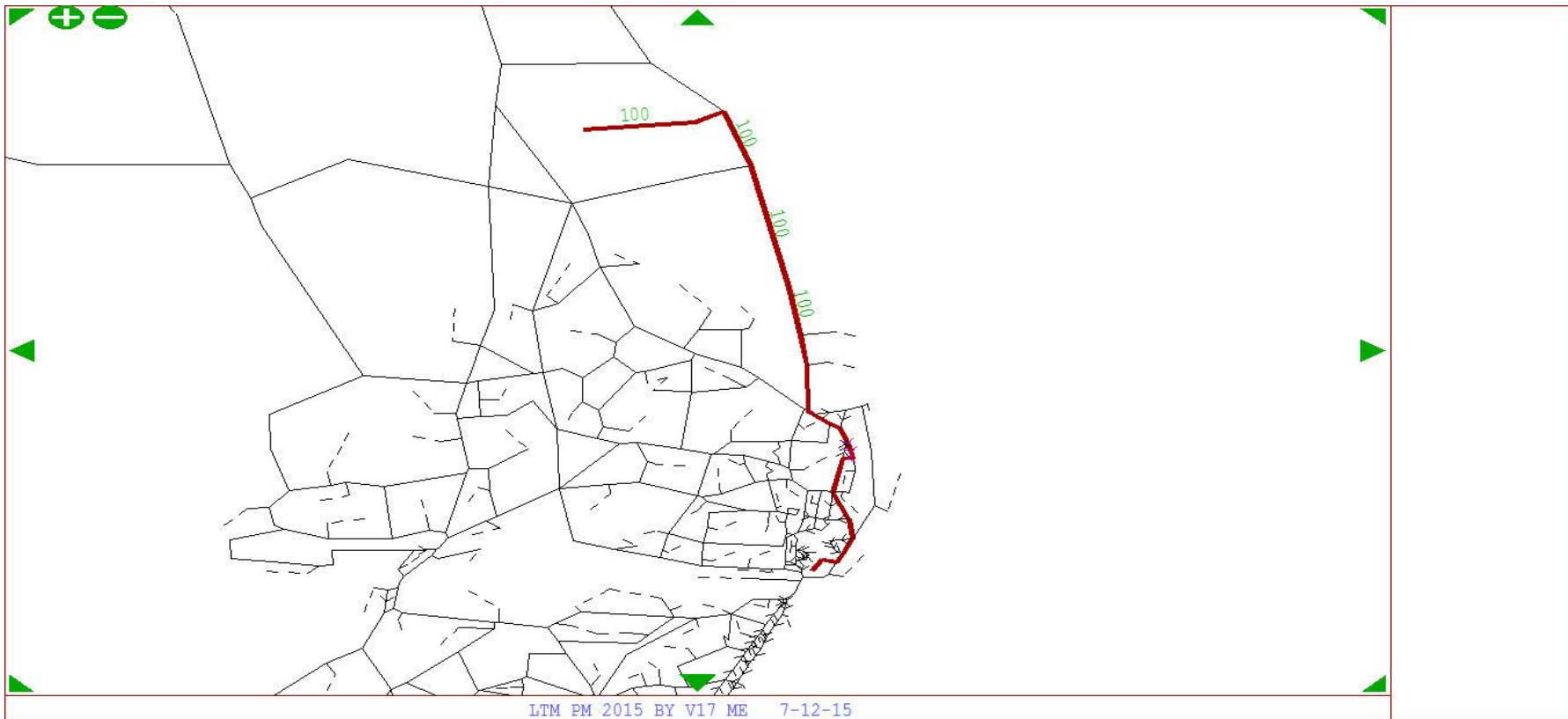
From Zone 102 To Zone 119 - User Class 2



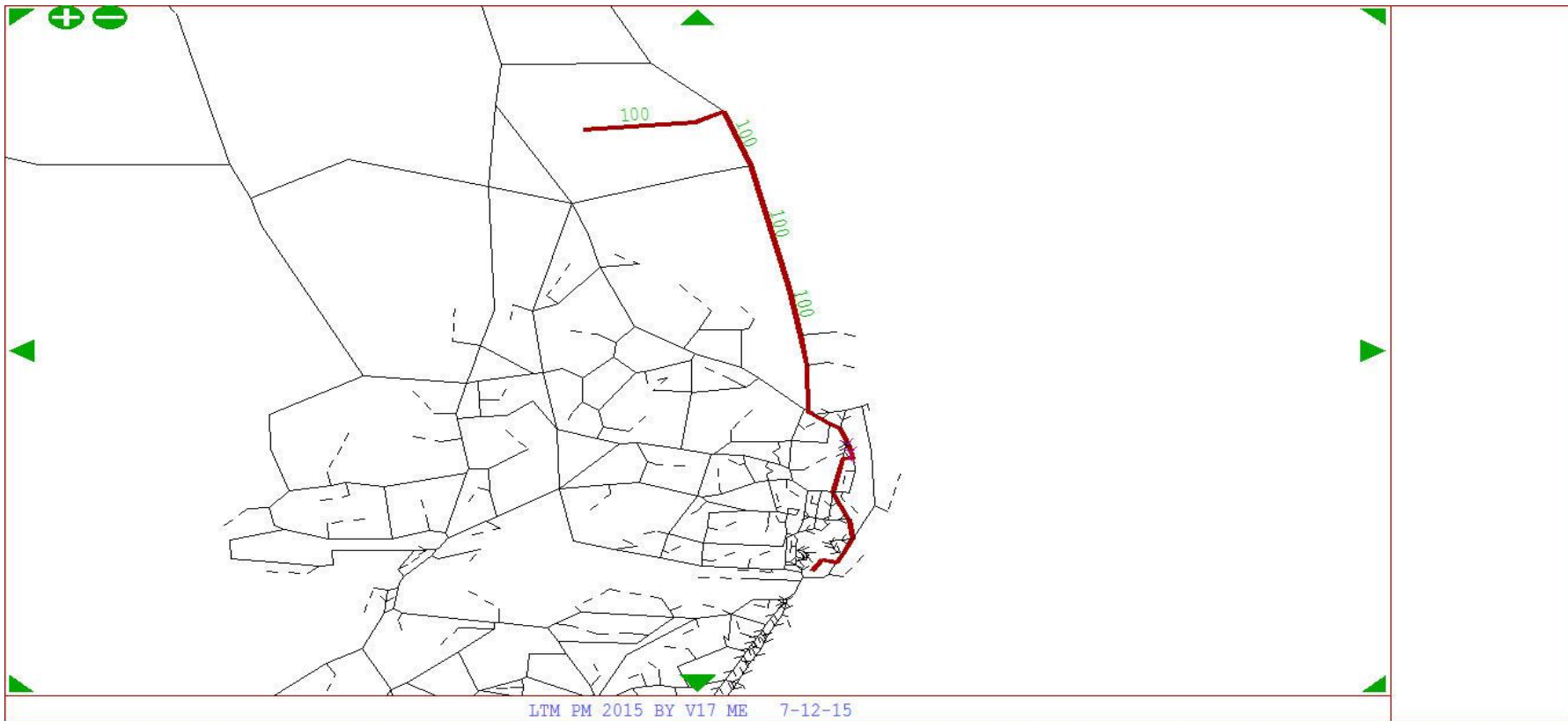
From Zone 102 To Zone 119 - User Class 3



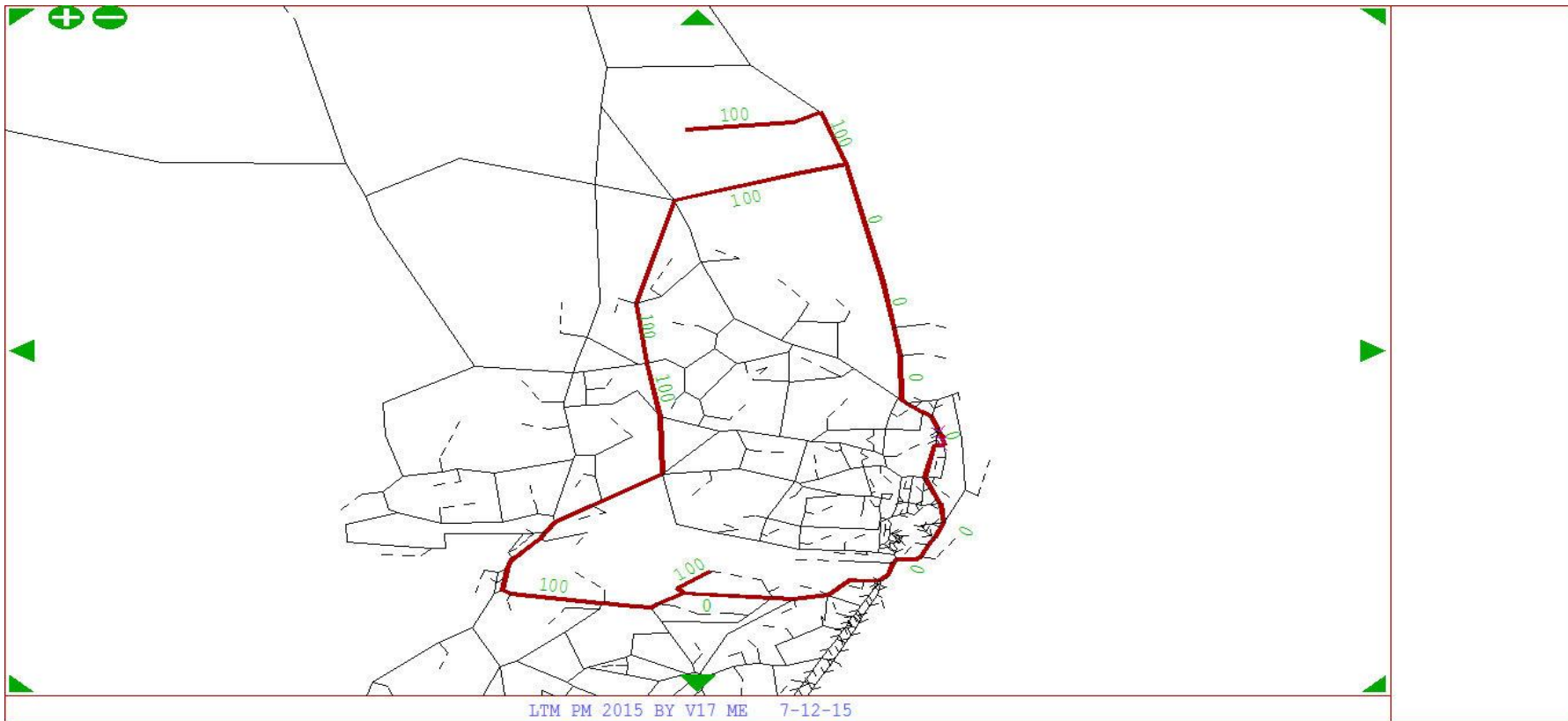
From Zone 102 To Zone 119 - User Class 4



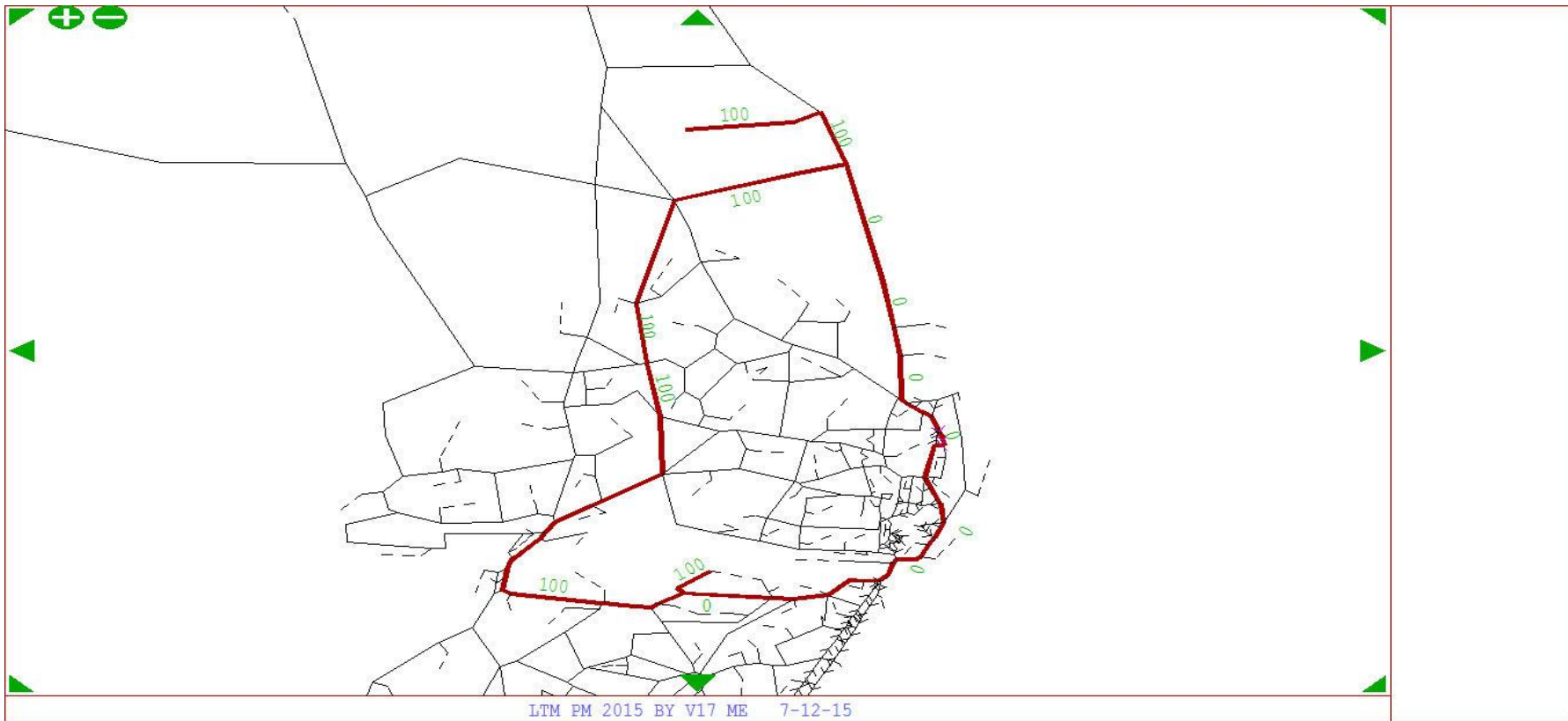
From Zone 102 To Zone 119 - User Class 5



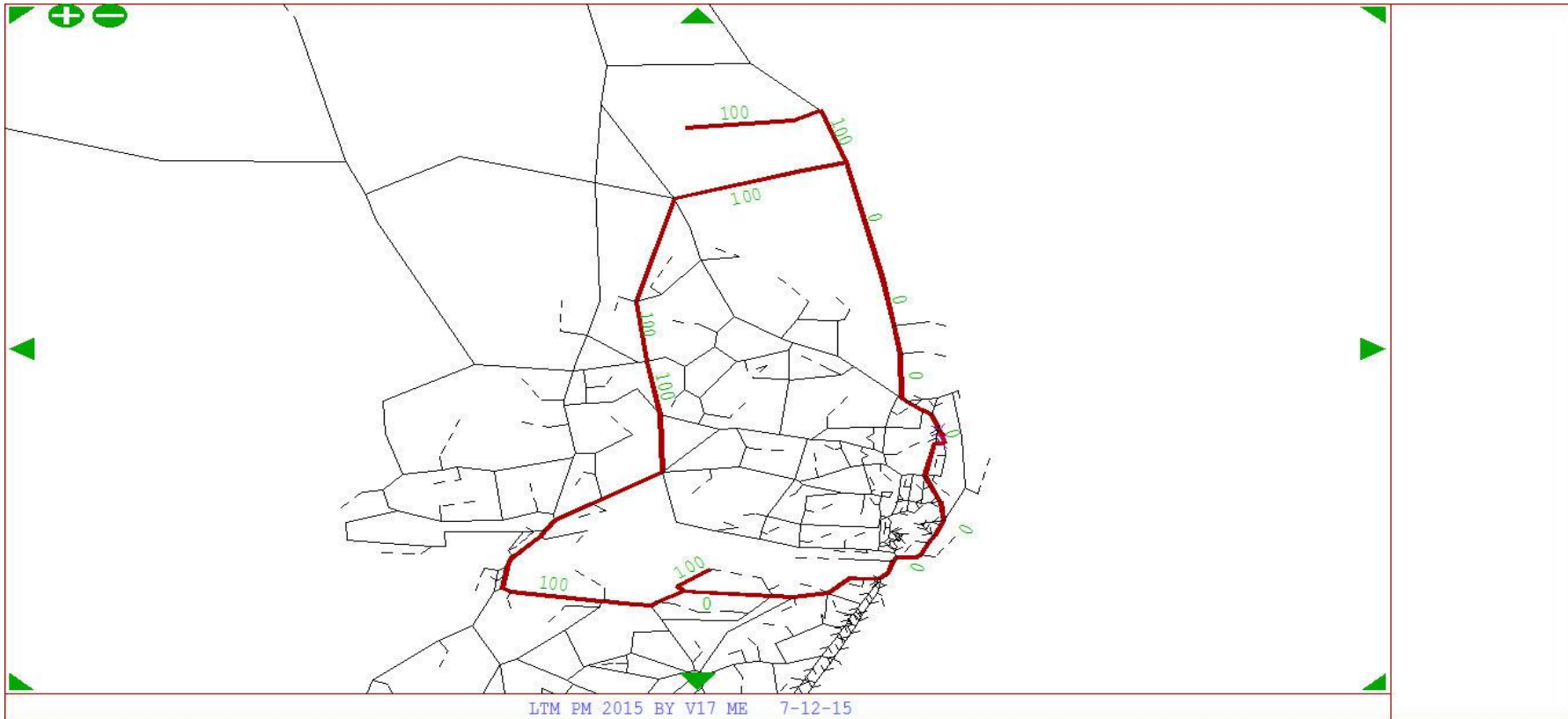
From Zone 102 To Zone 122 - User Class 1



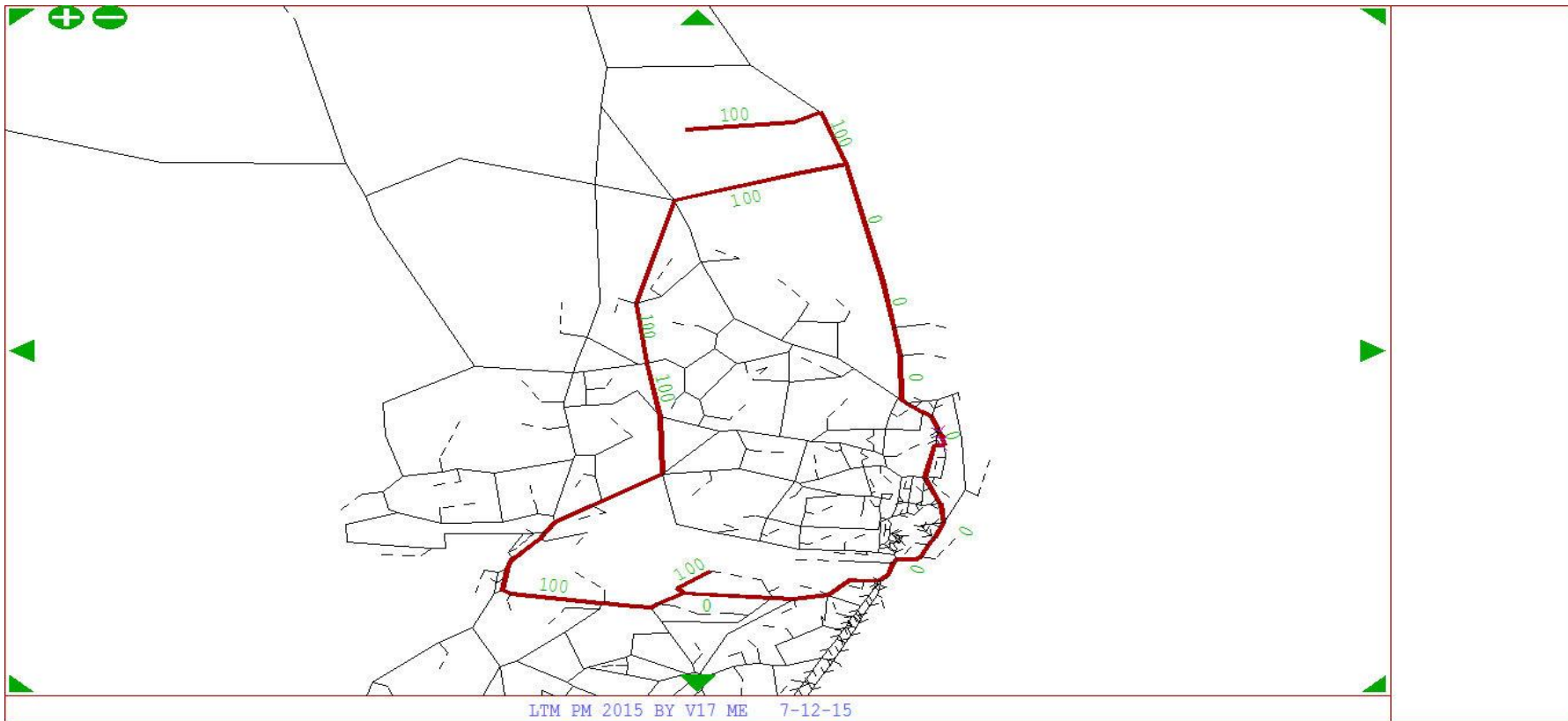
From Zone 102 To Zone 122 - User Class 2



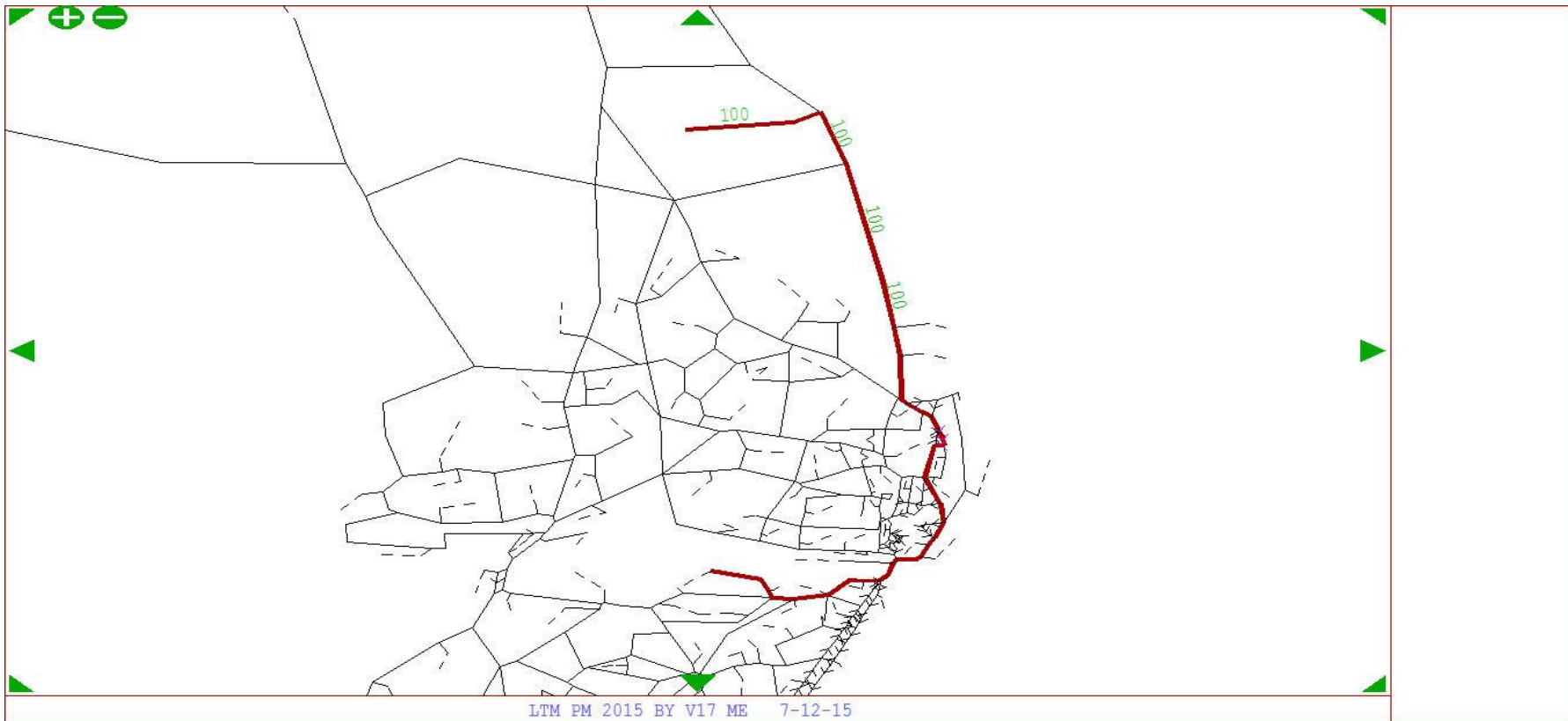
From Zone 102 To Zone 122 - User Class 3



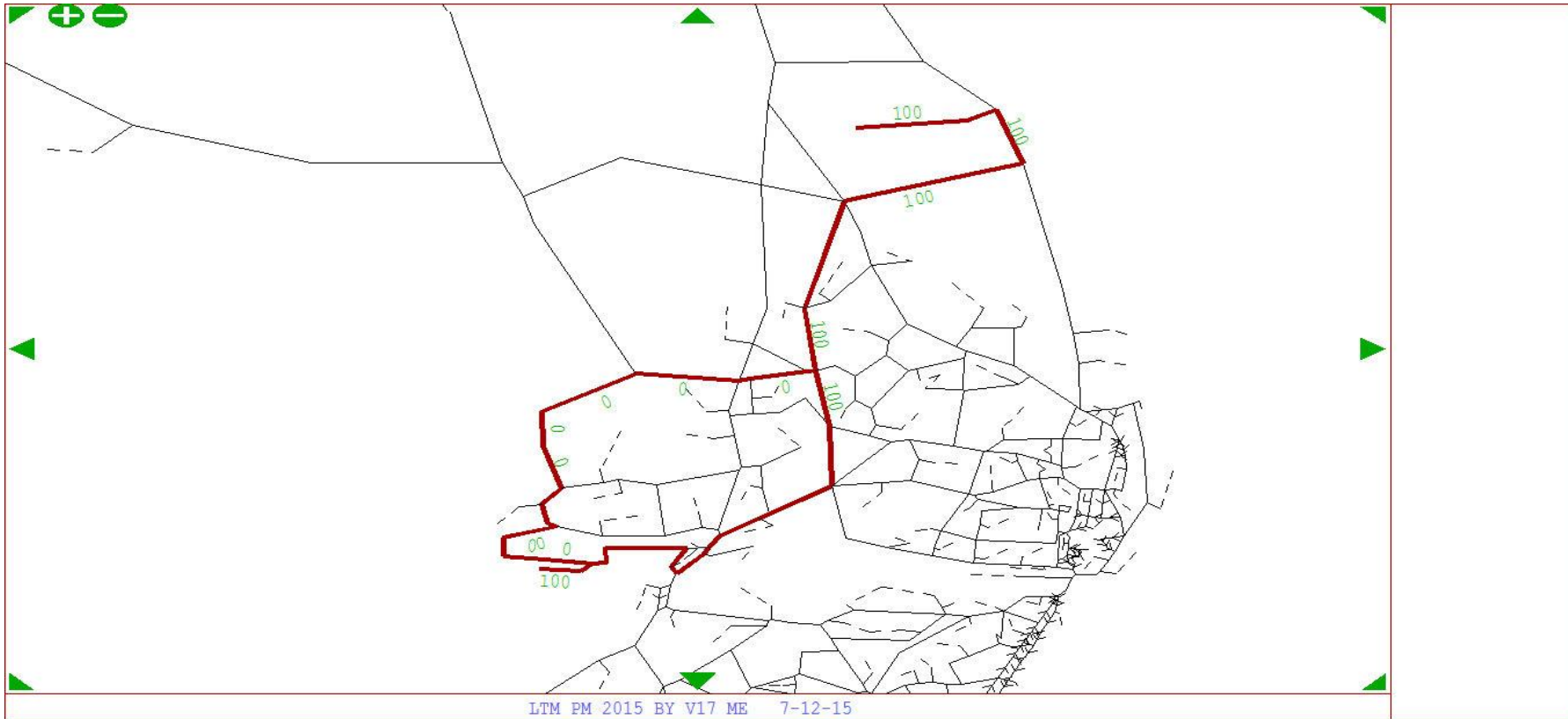
From Zone 102 To Zone 122 - User Class 4



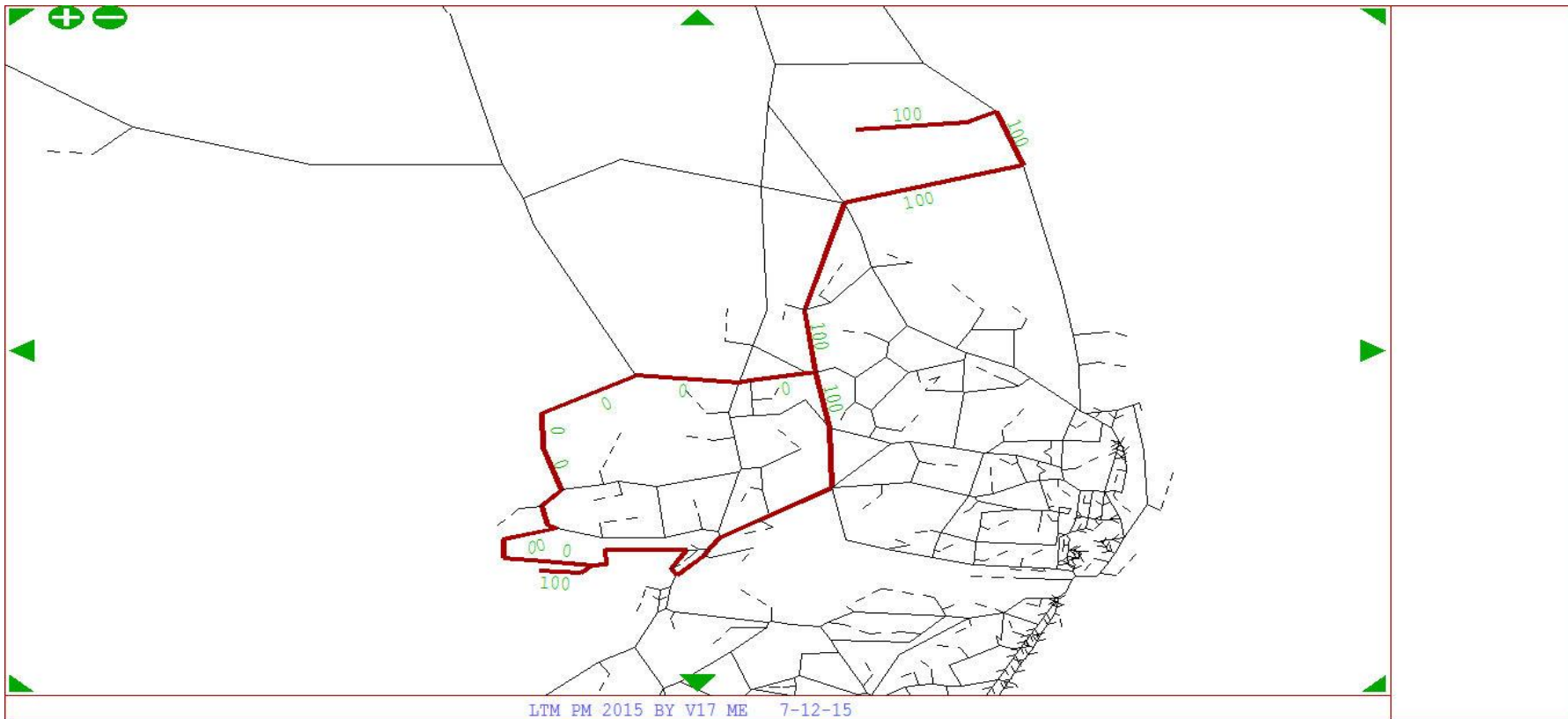
From Zone 102 To Zone 122 - User Class 5



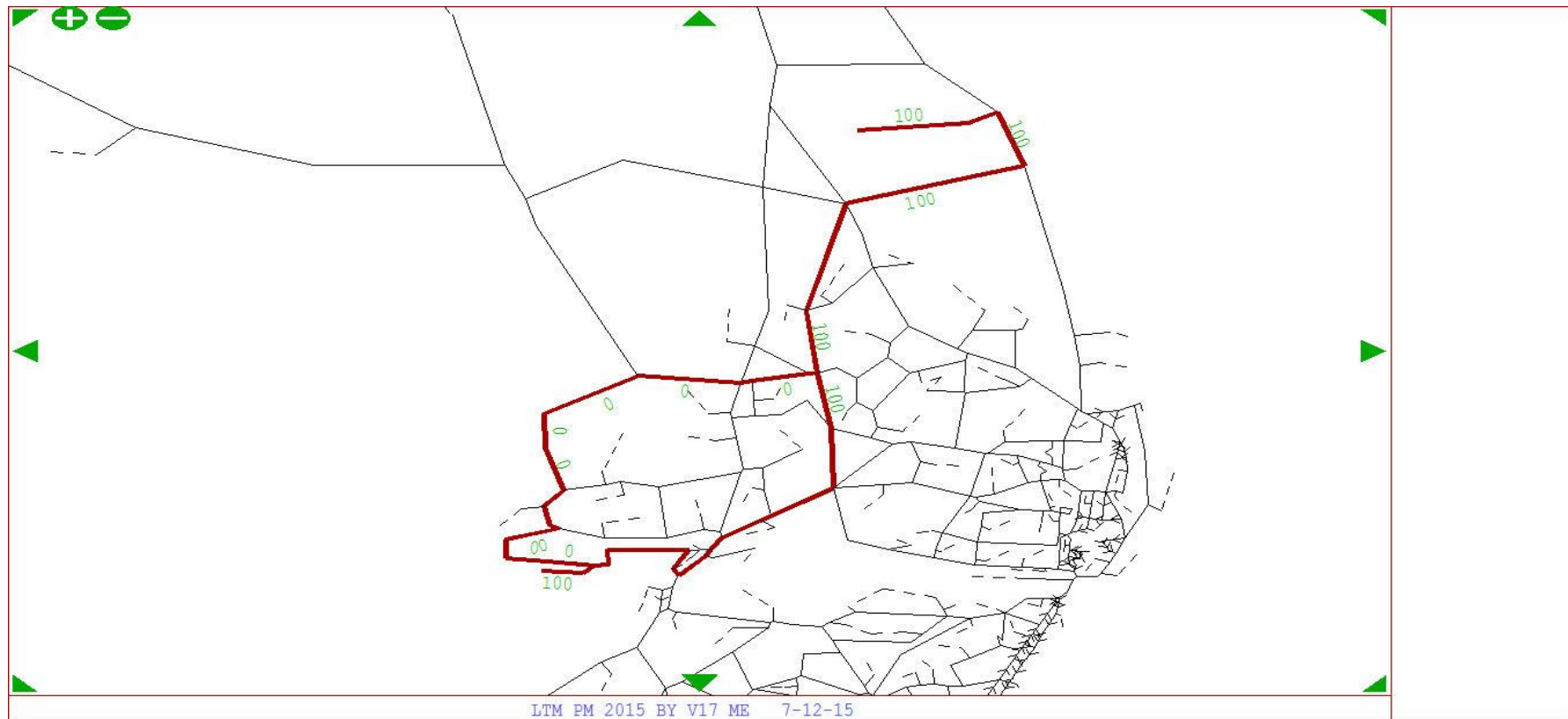
From Zone 102 To Zone 130 - User Class 1



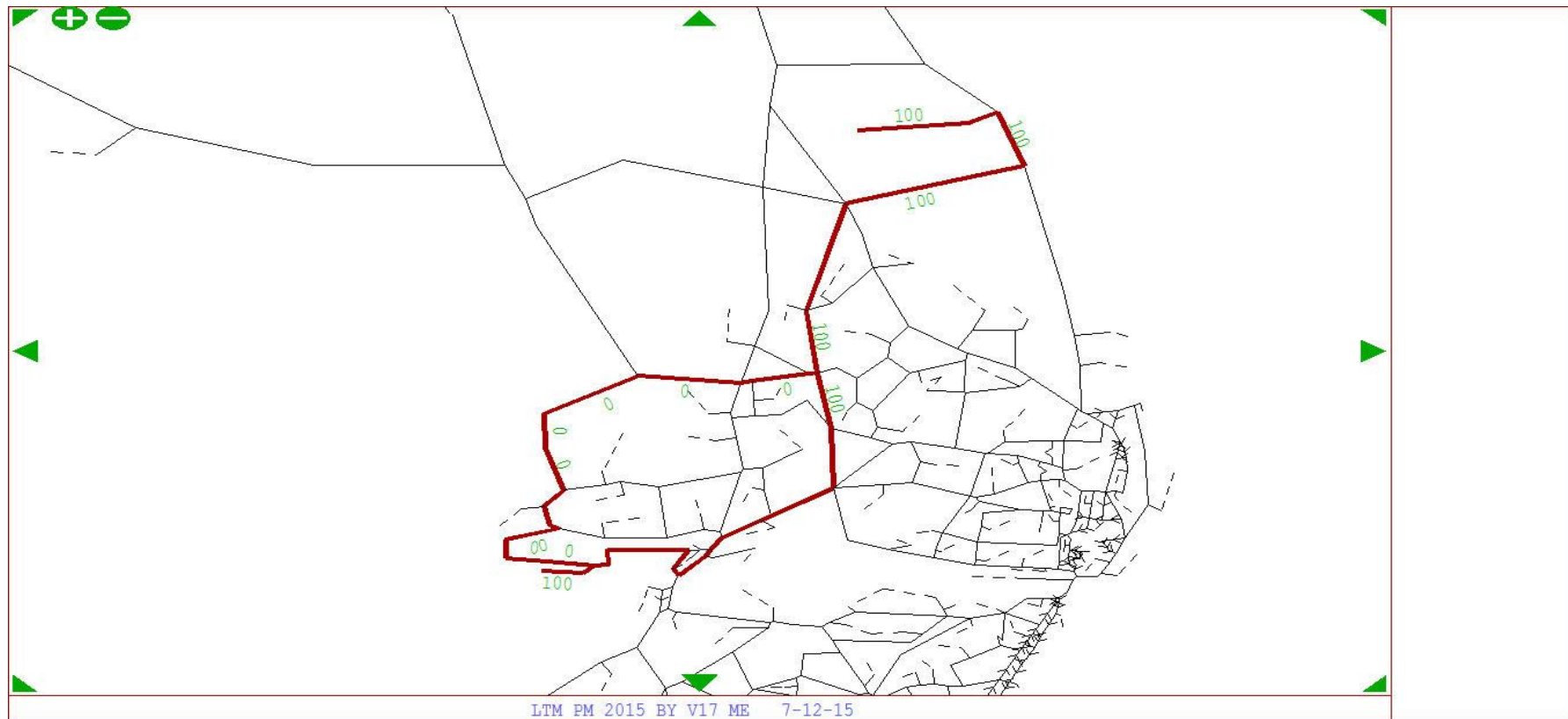
From Zone 102 To Zone 130 - User Class 2



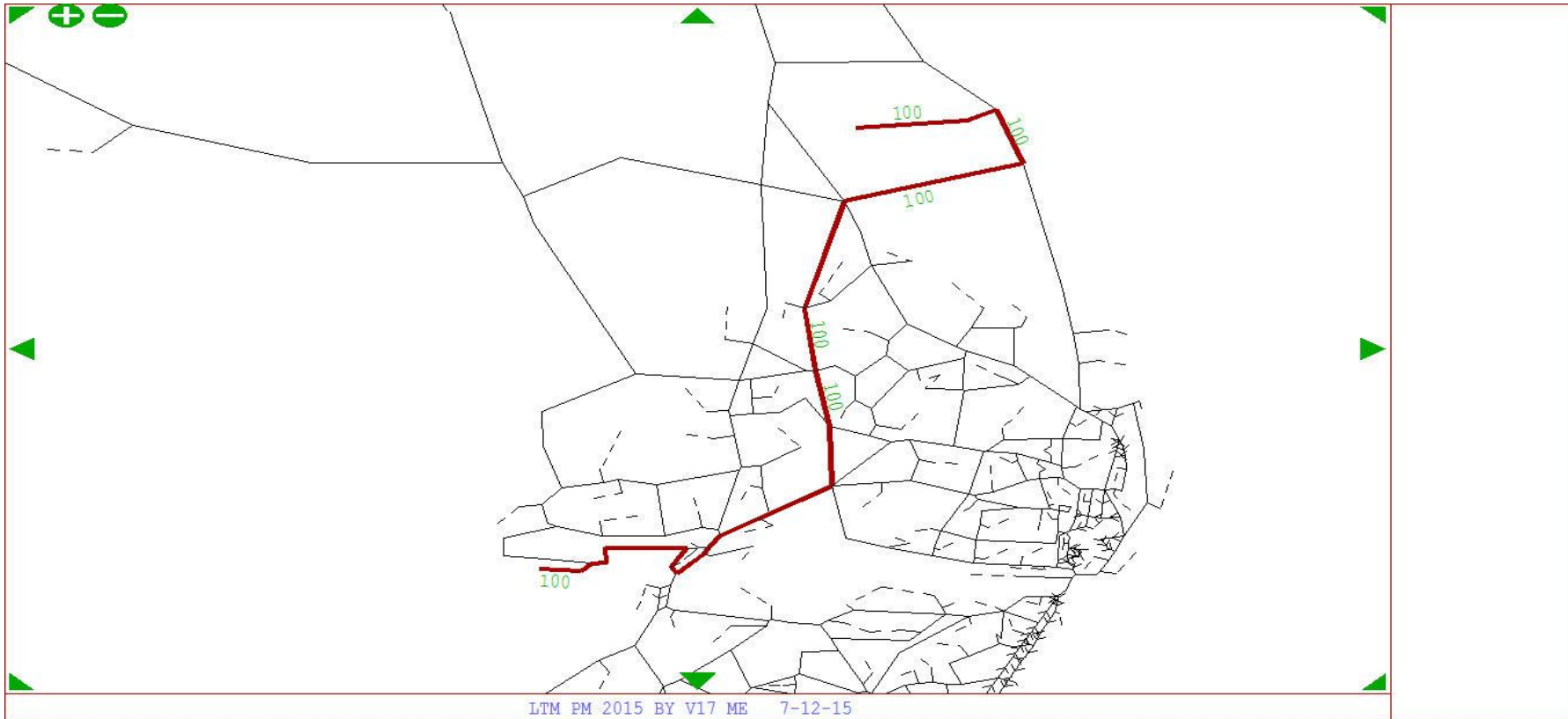
From Zone 102 To Zone 130 - User Class 3



From Zone 102 To Zone 130 - User Class 4

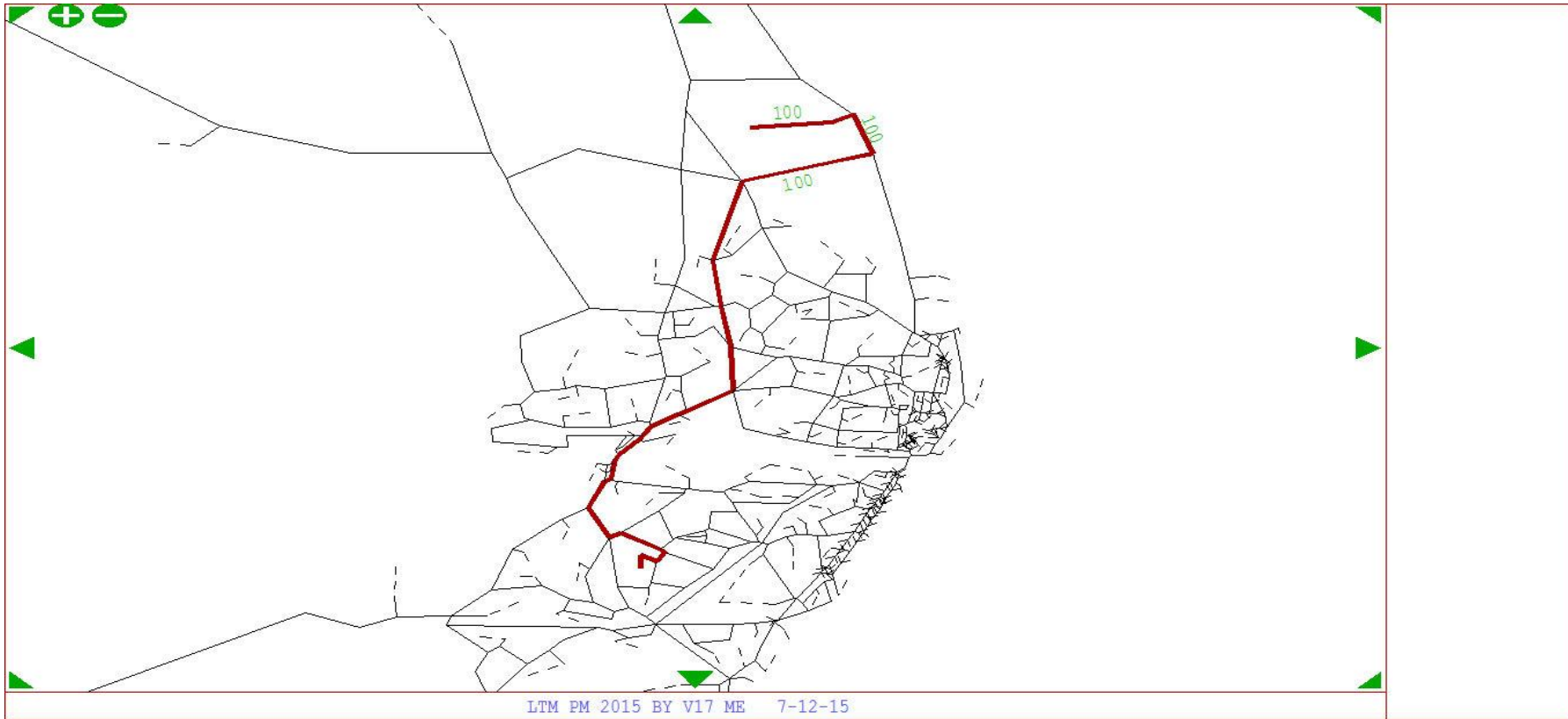


From Zone 102 To Zone 130 - User Class 5



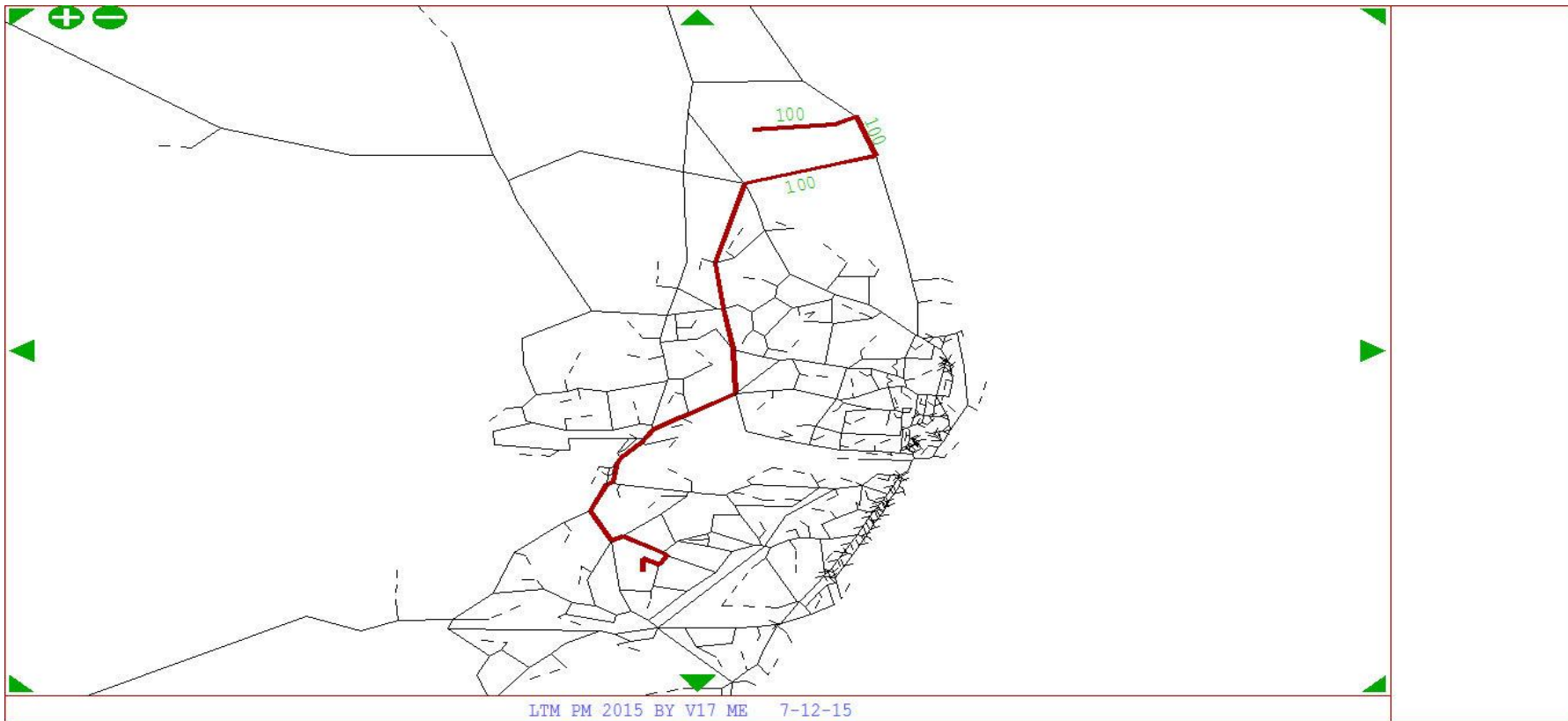
LTM PM 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 131 - User Class 1



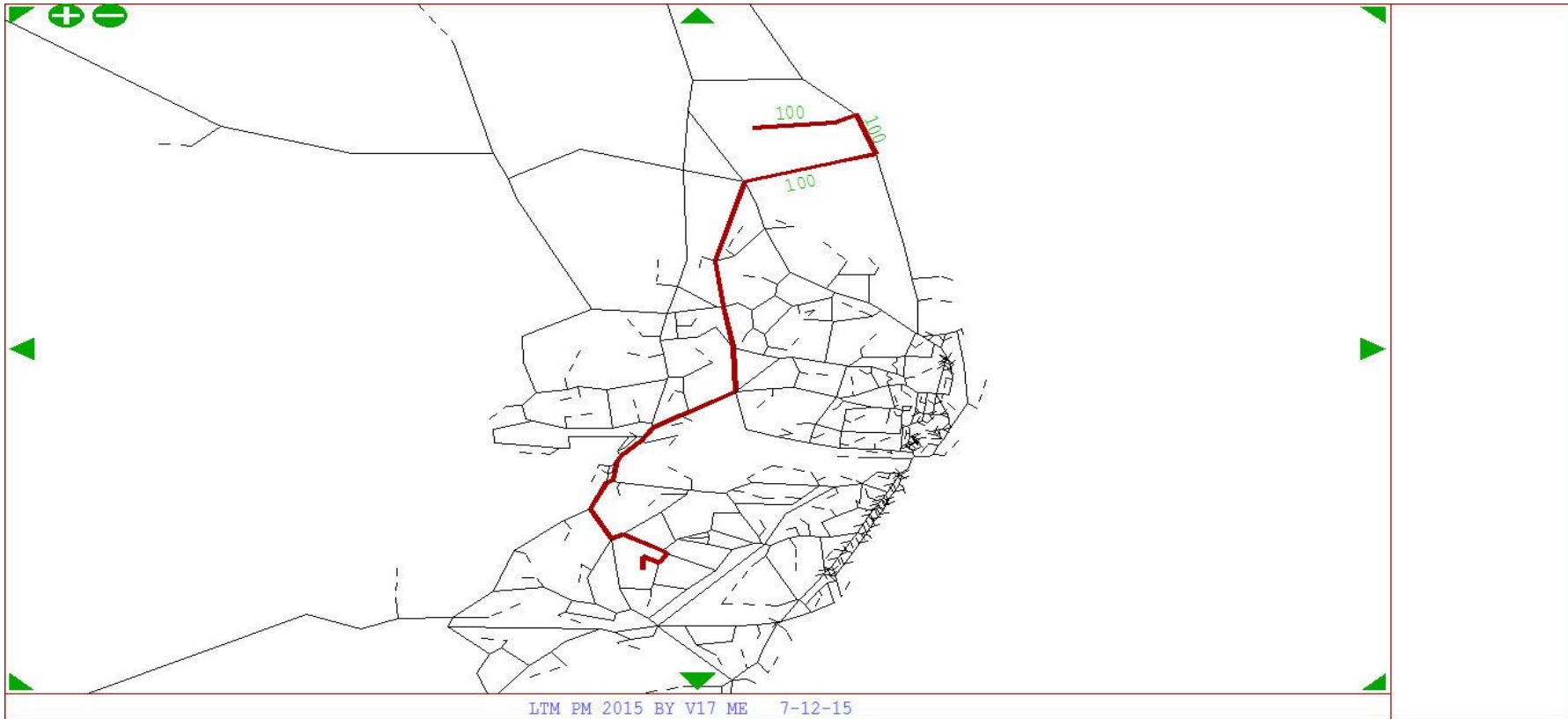
LTM PM 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 131 - User Class 2



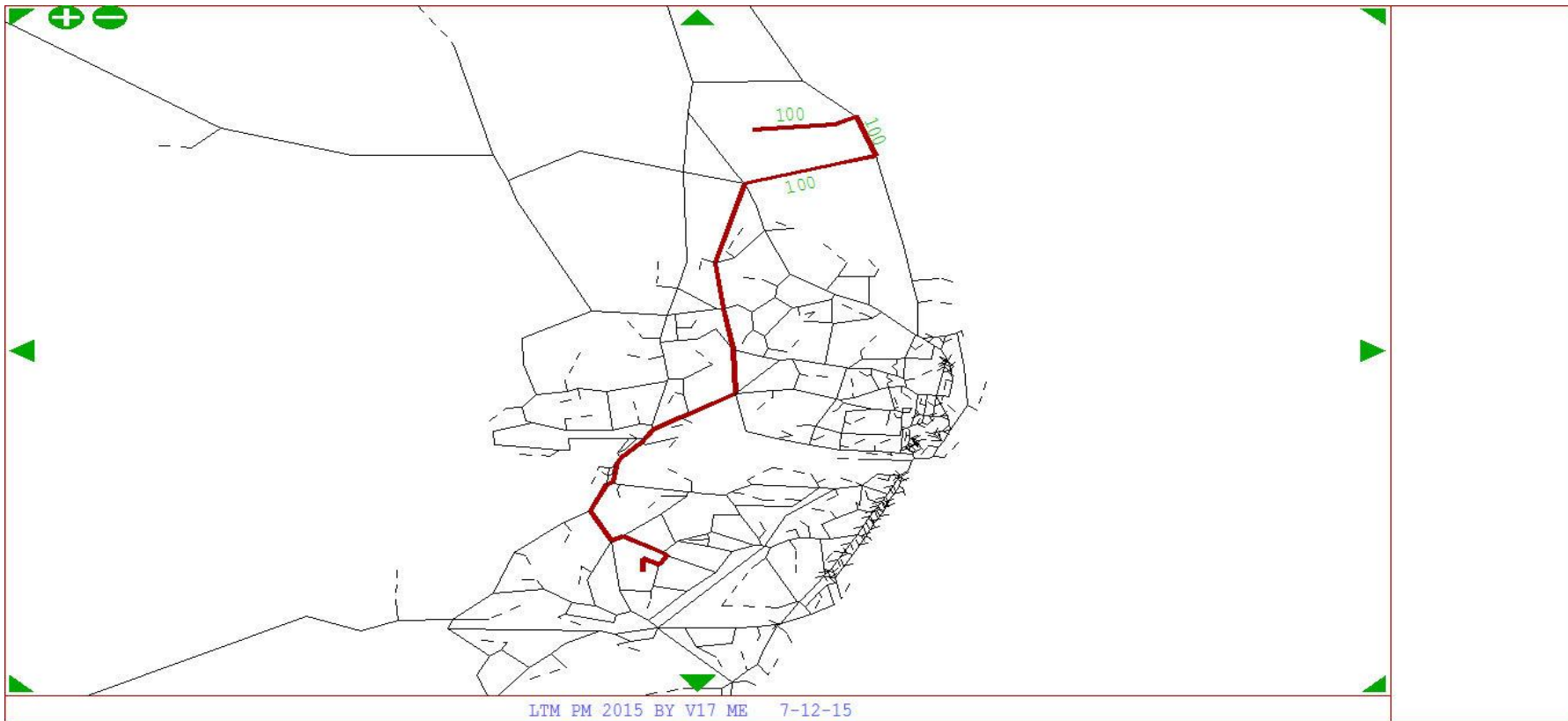
LTM PM 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 131 - User Class 3

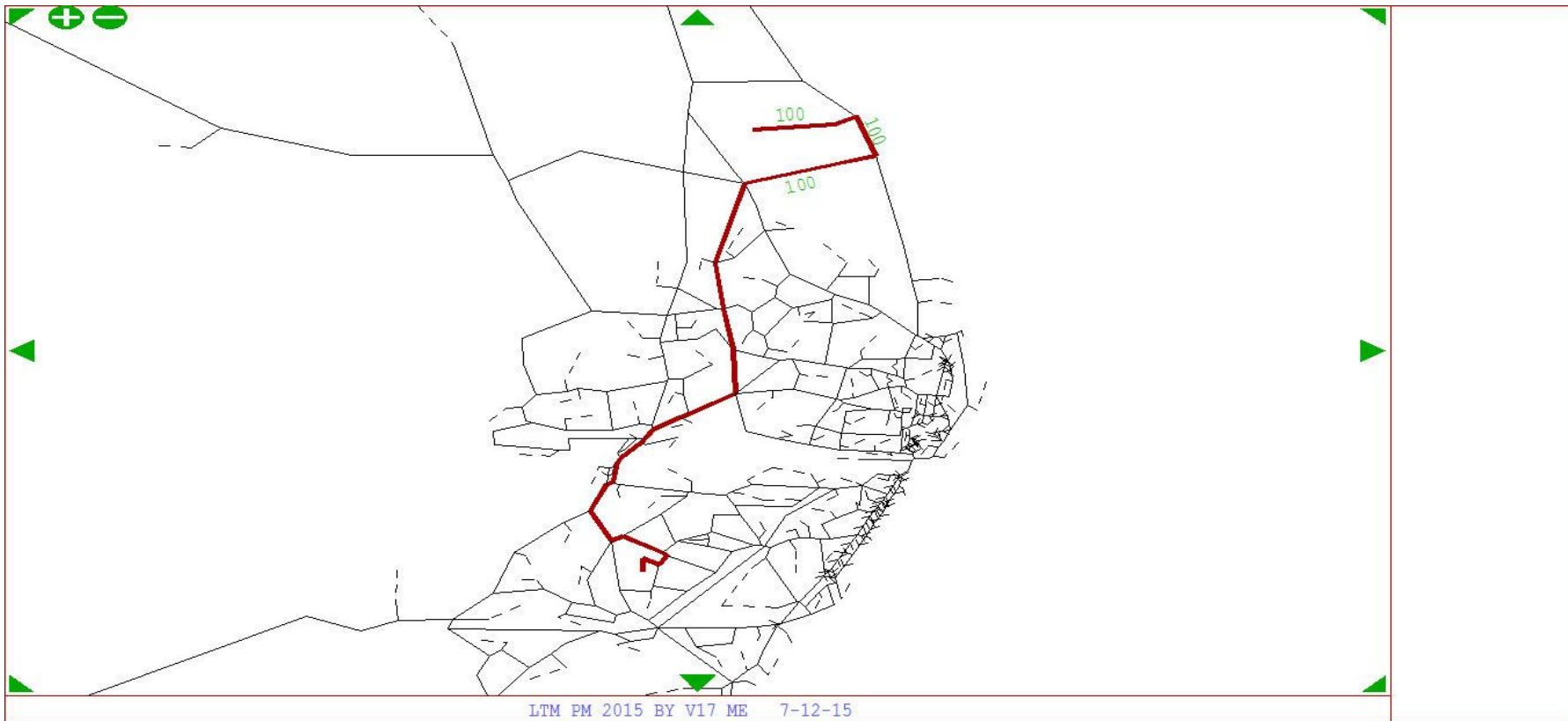


LTM PM 2015 BY V17 ME 7-12-15

From Zone 102 To Zone 131 - User Class 4

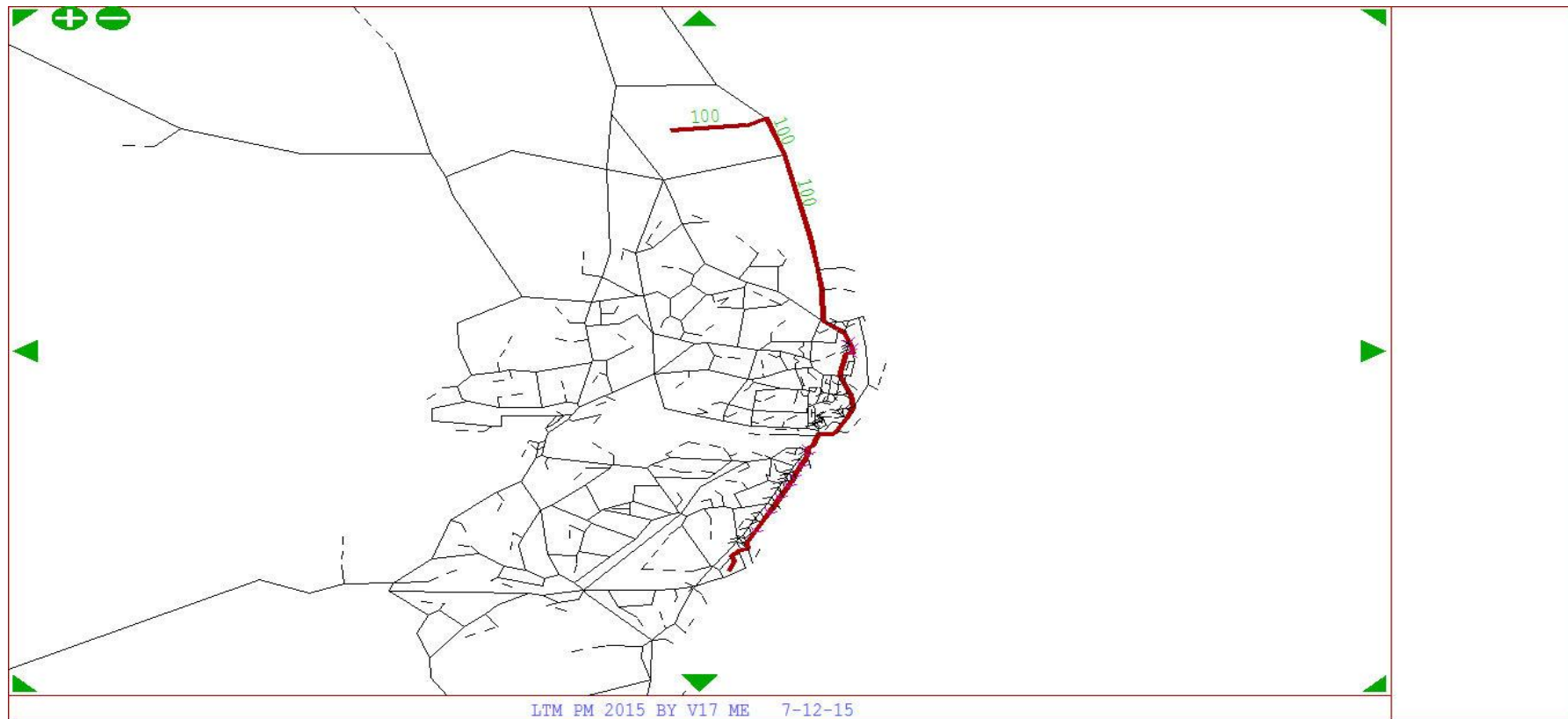


From Zone 102 To Zone 131 - User Class 5

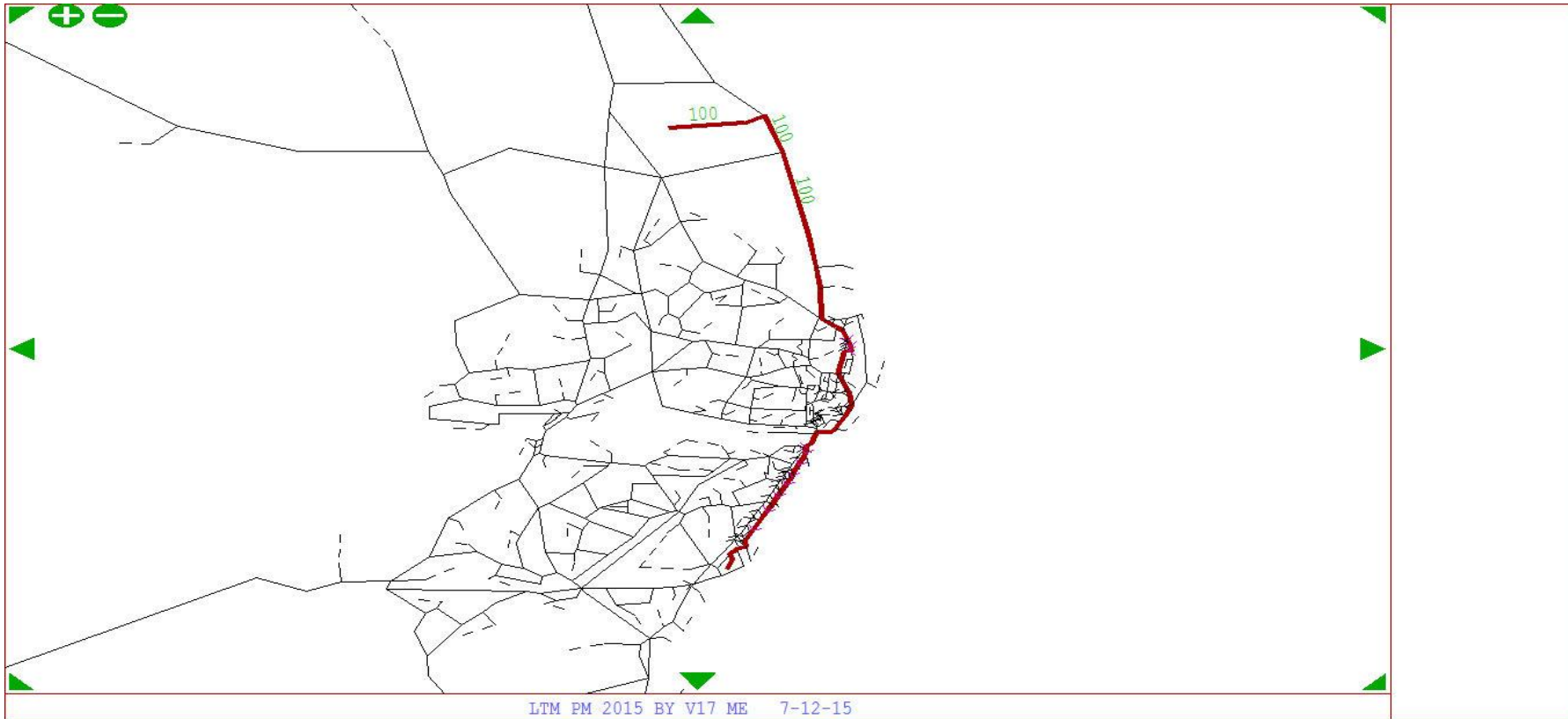


LTM PM 2015 BY V17 ME 7-12-15

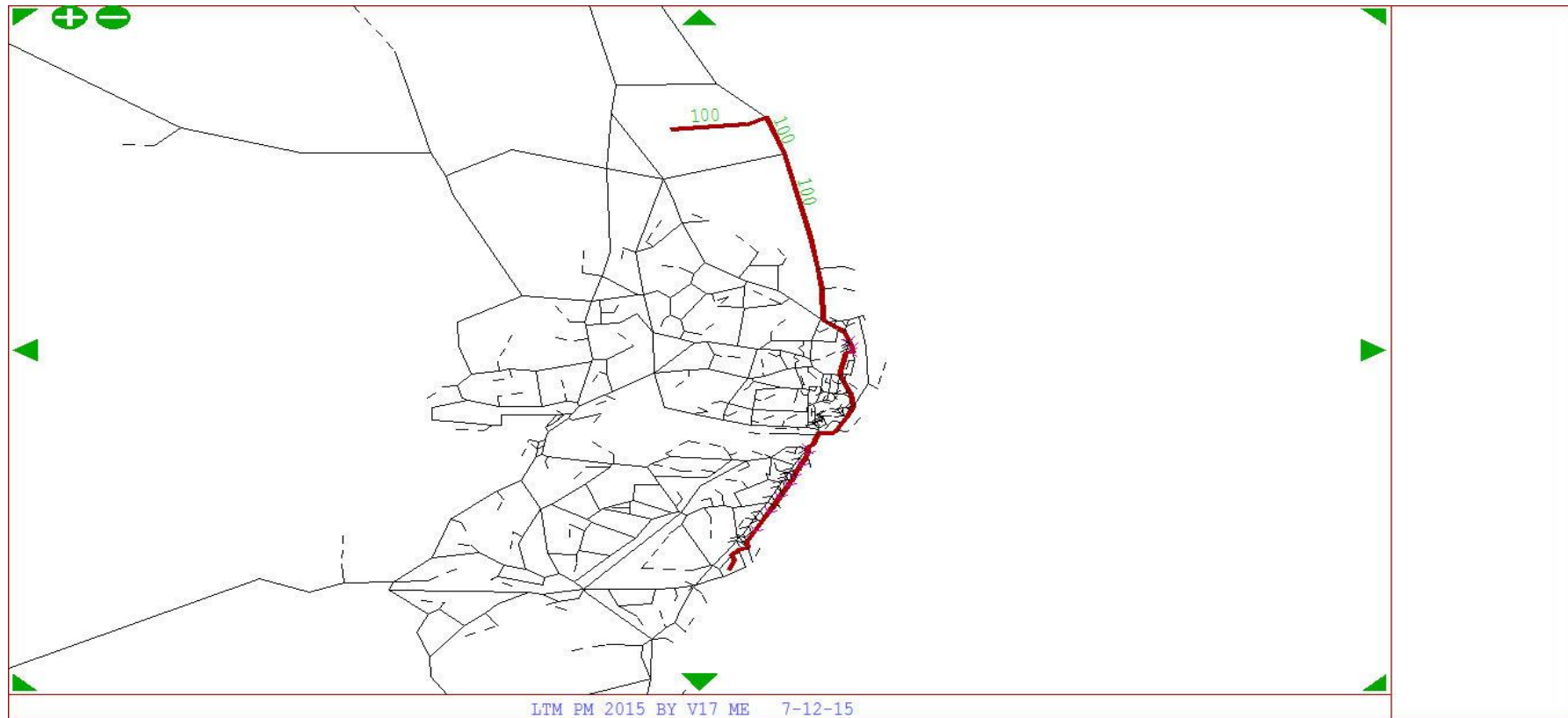
From Zone 102 To Zone 136 - User Class 1



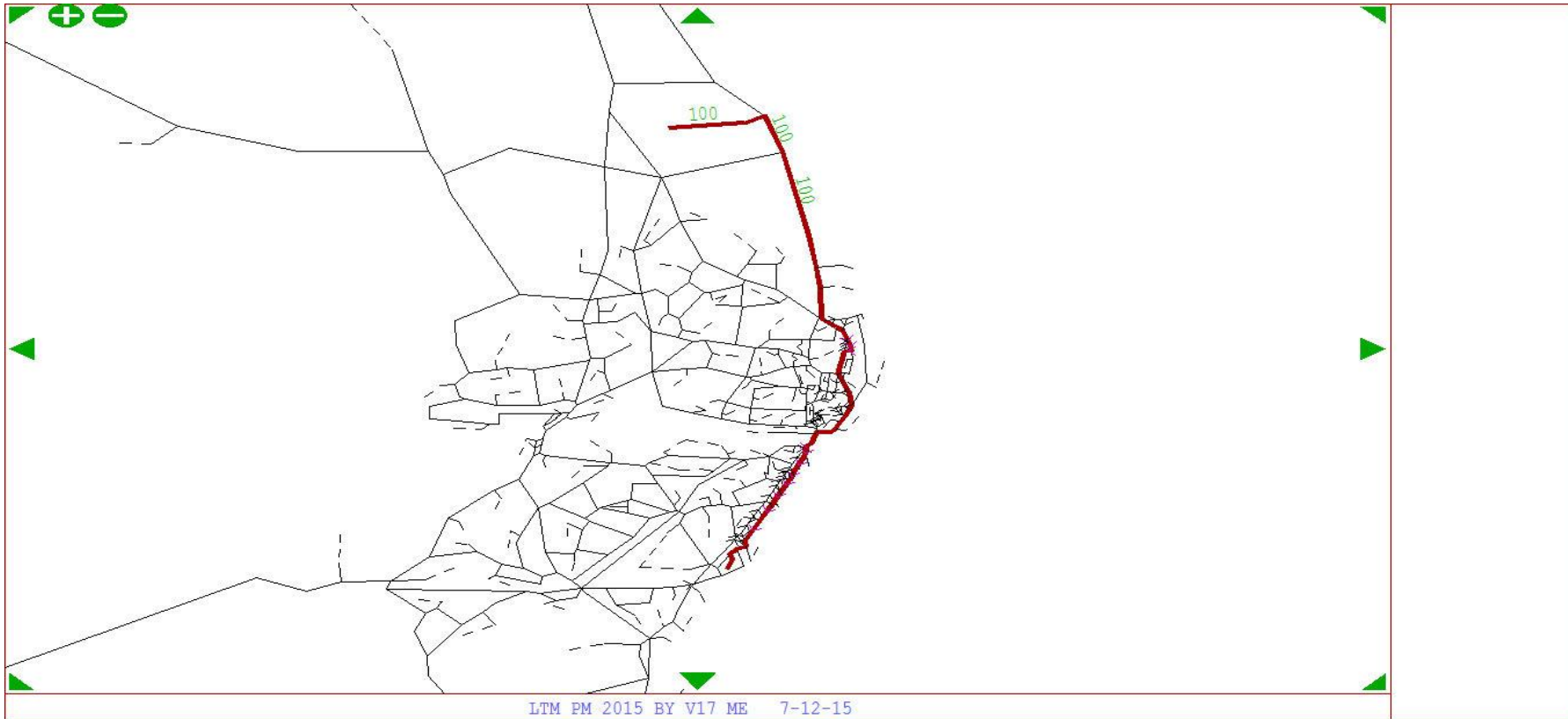
From Zone 102 To Zone 136 - User Class 2



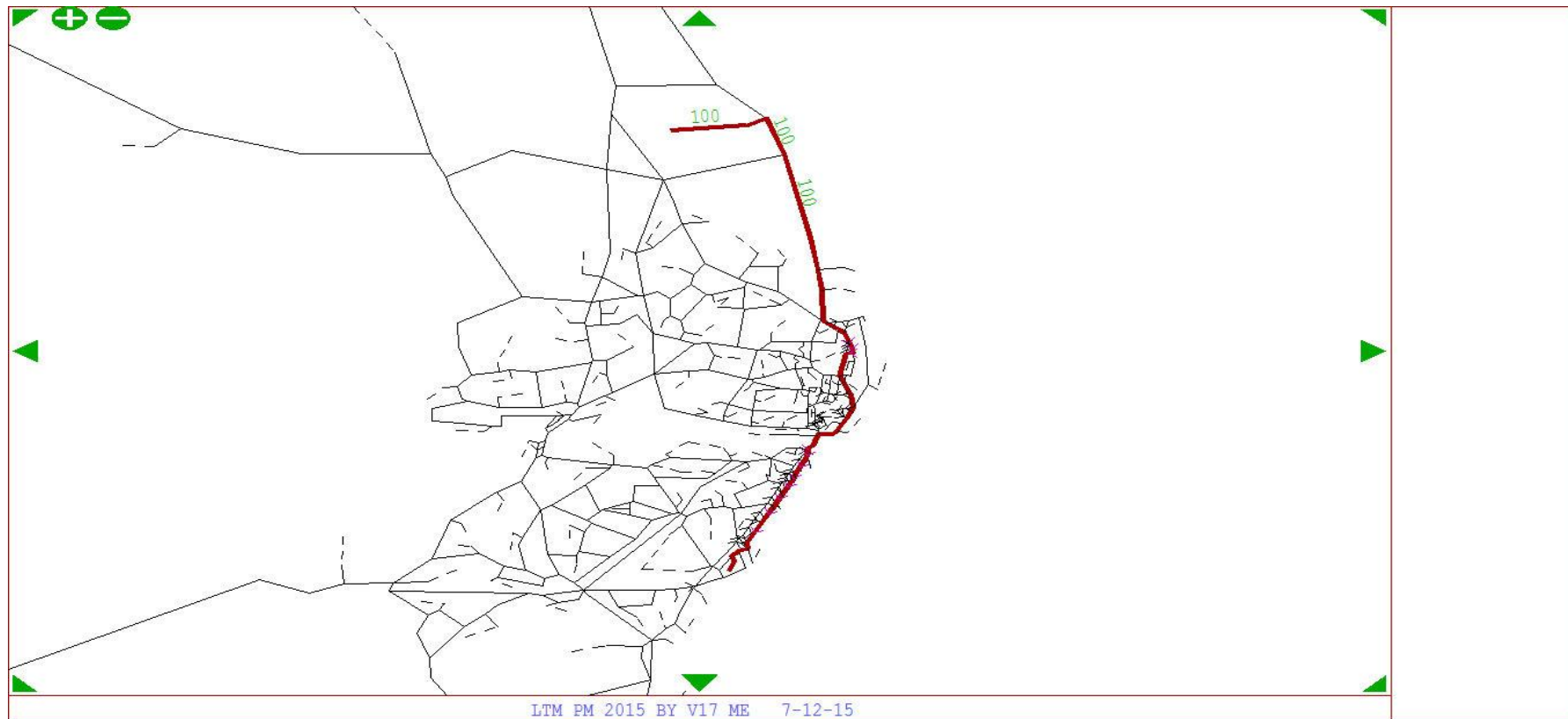
From Zone 102 To Zone 136 - User Class 3



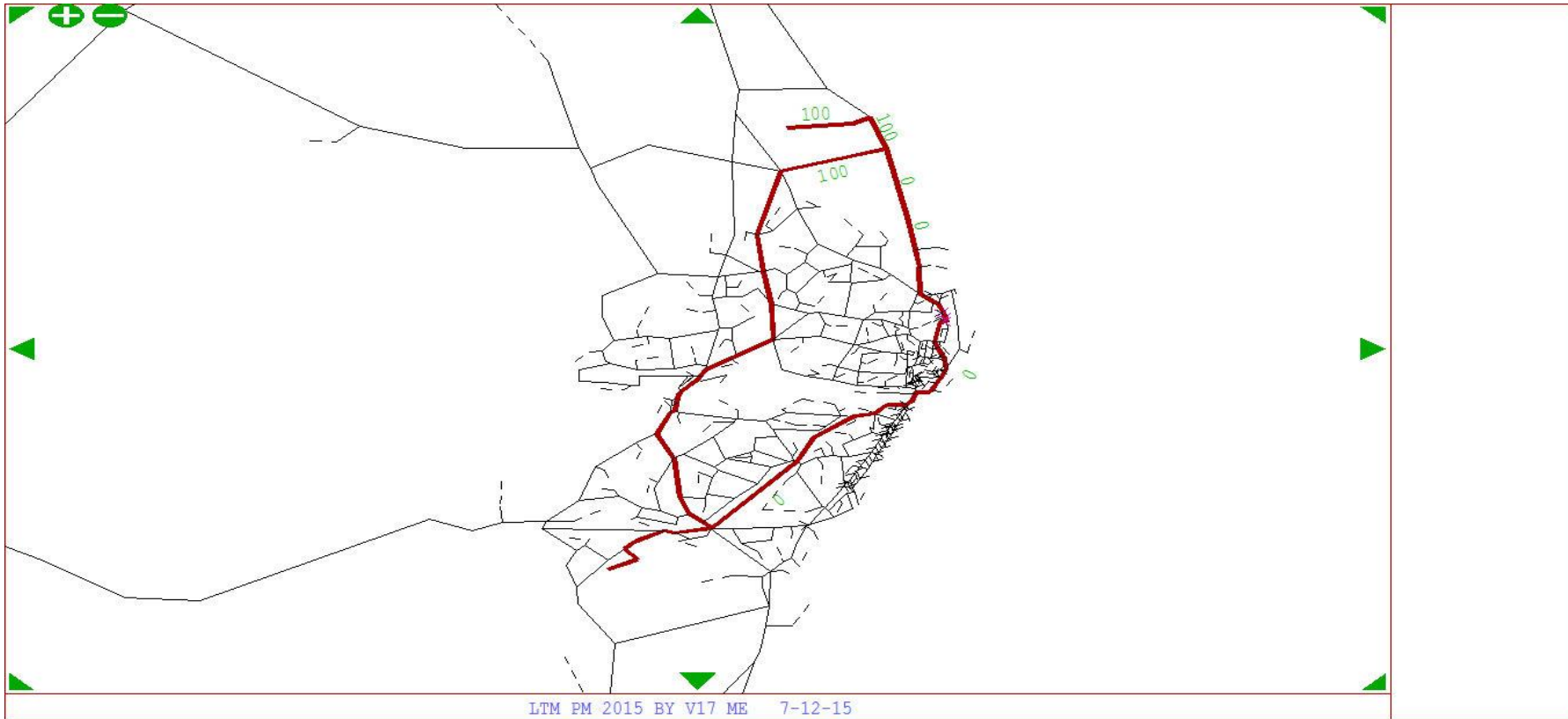
From Zone 102 To Zone 136 - User Class 4



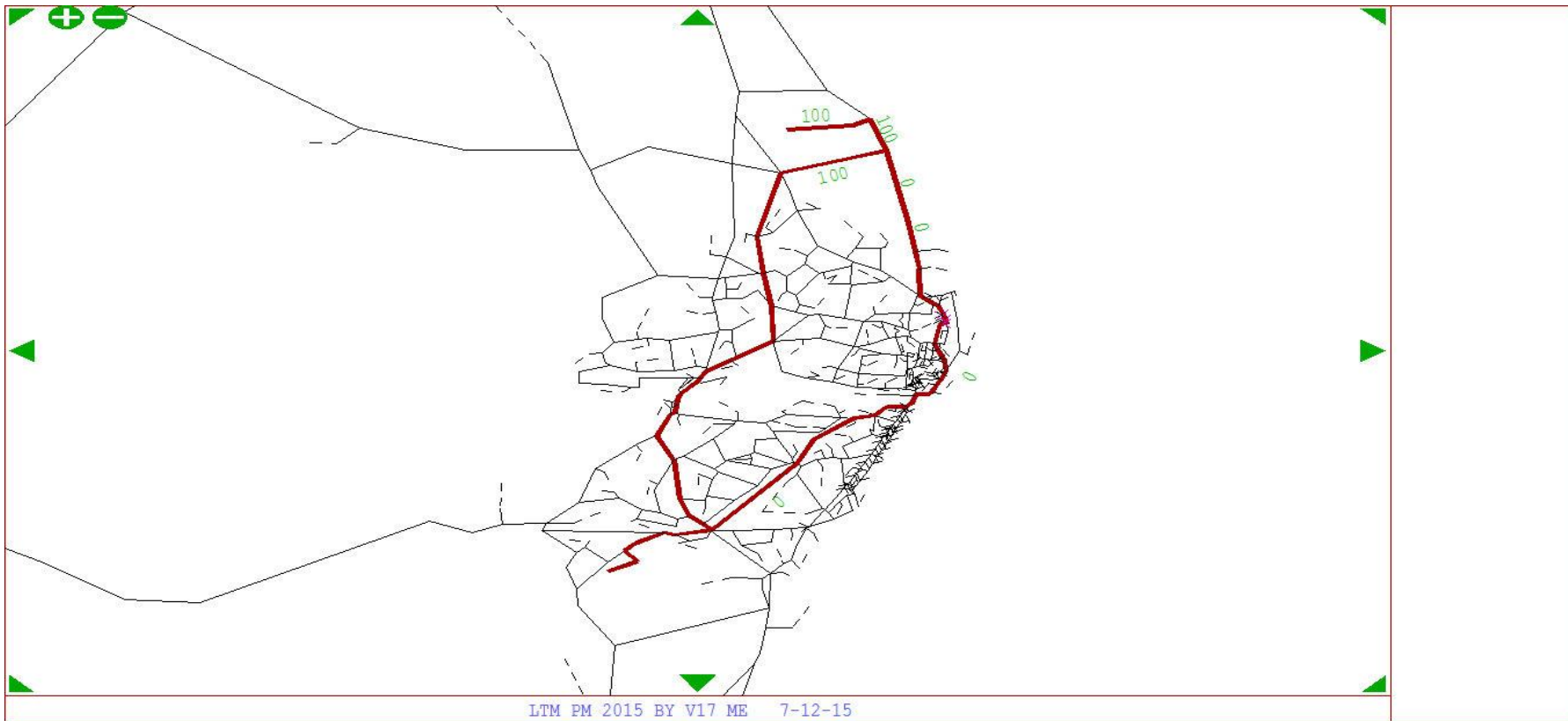
From Zone 102 To Zone 136 - User Class 5



From Zone 102 To Zone 143 - User Class 1

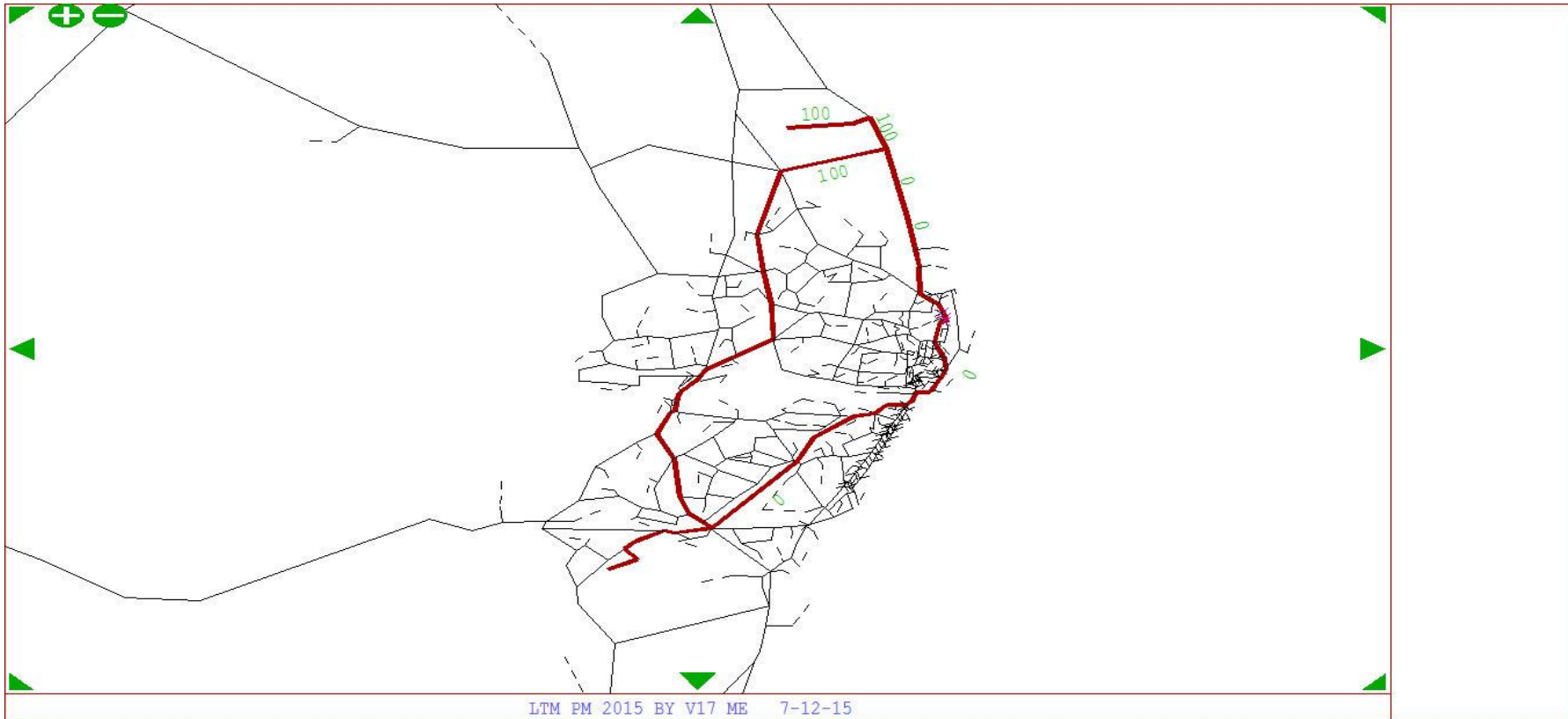


From Zone 102 To Zone 143 - User Class 2

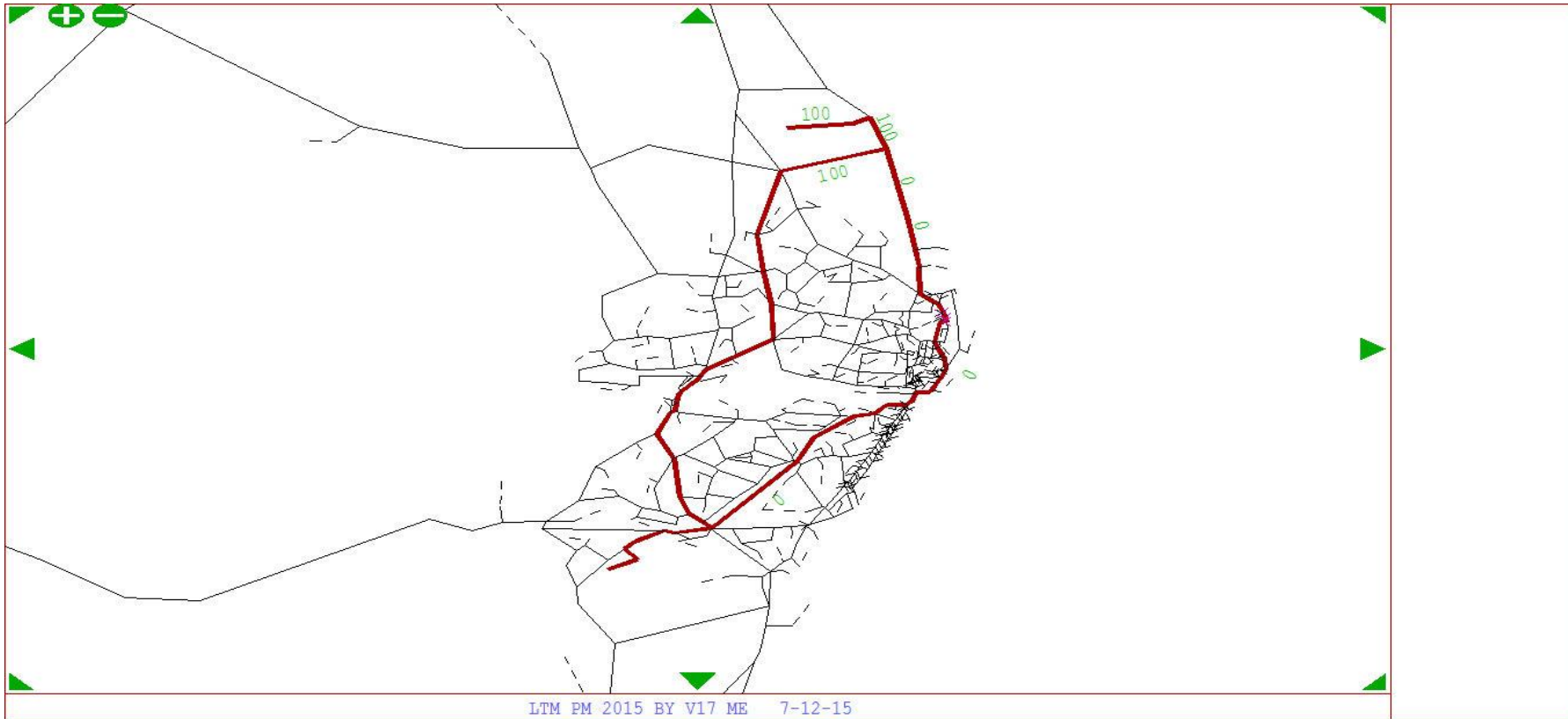


LTM PM 2015 BY V17 ME 7-12-15

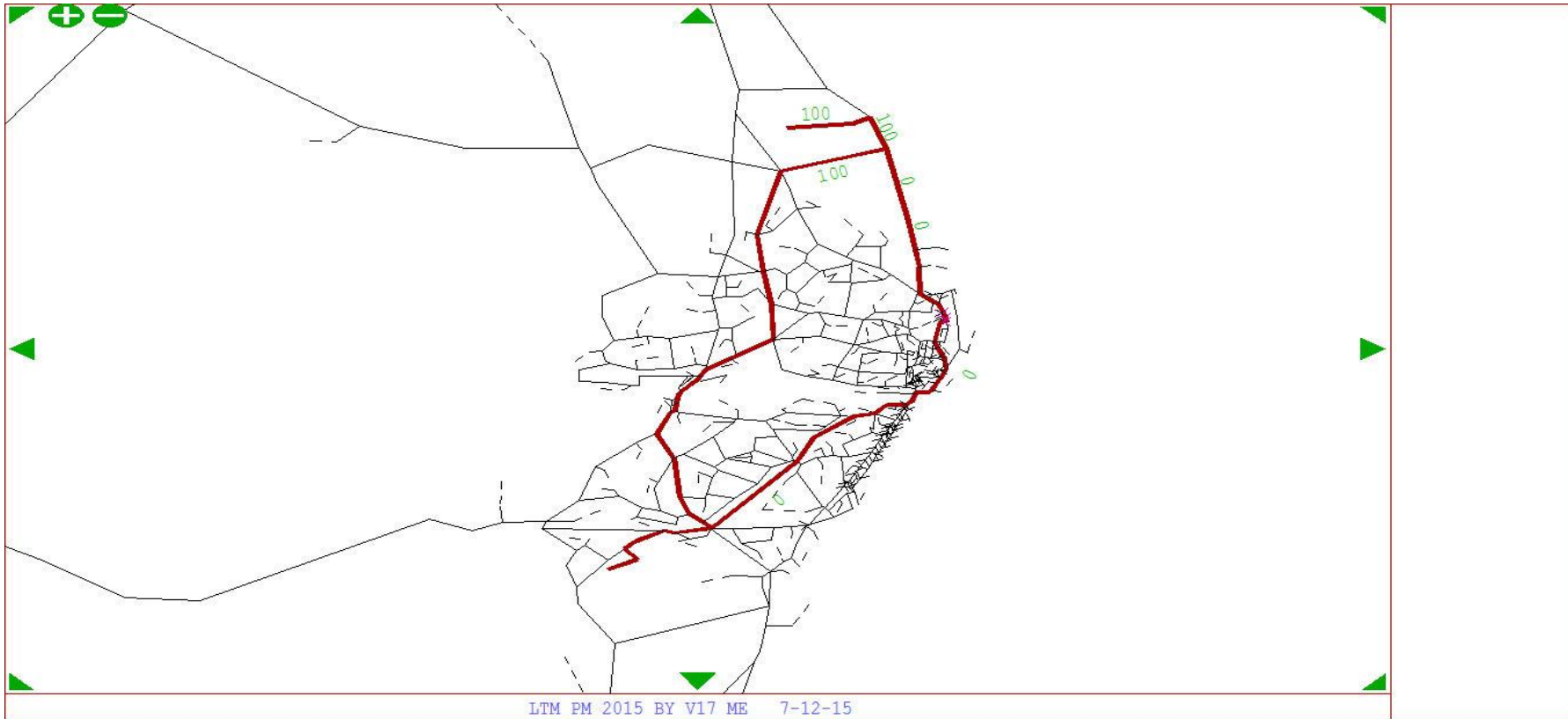
From Zone 102 To Zone 143 - User Class 3



From Zone 102 To Zone 143 - User Class 4

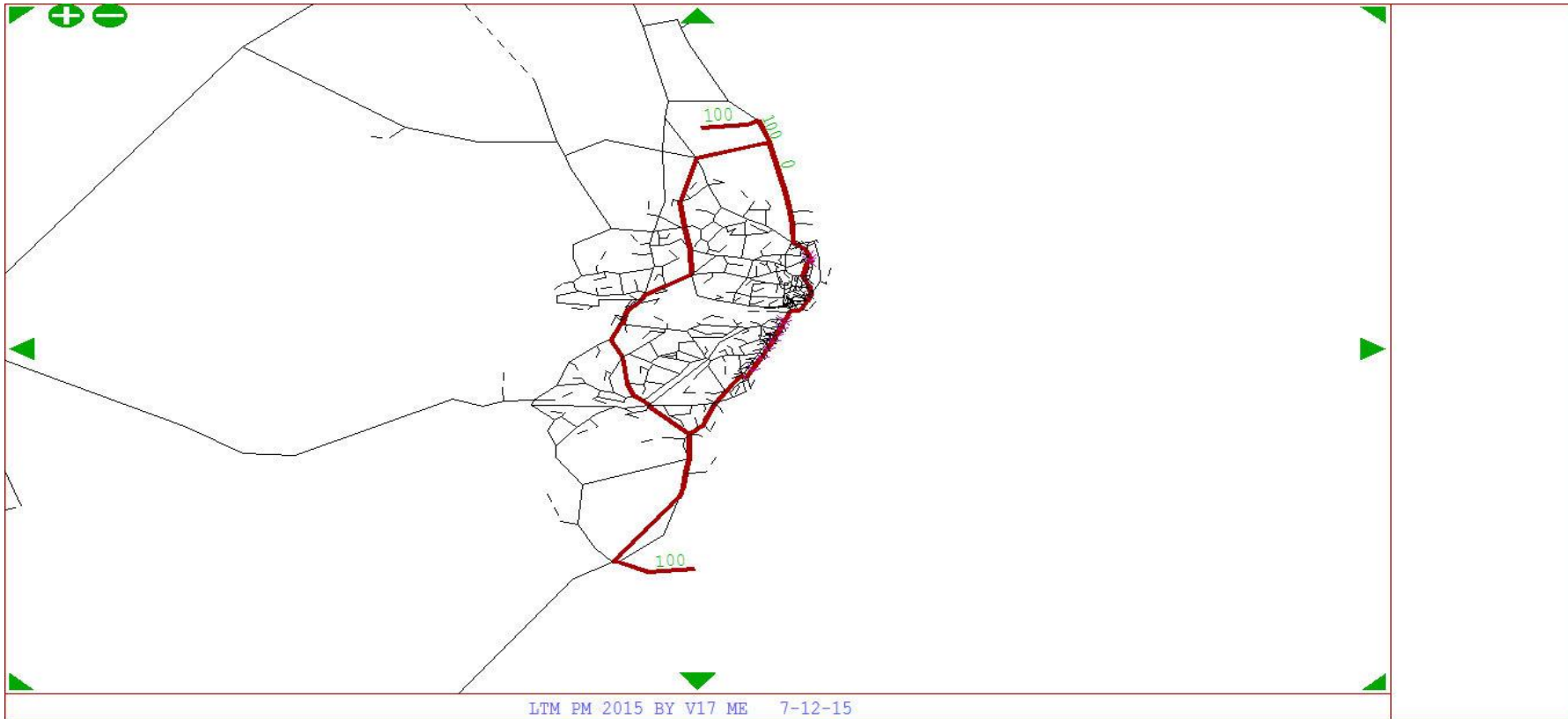


From Zone 102 To Zone 143 - User Class 5

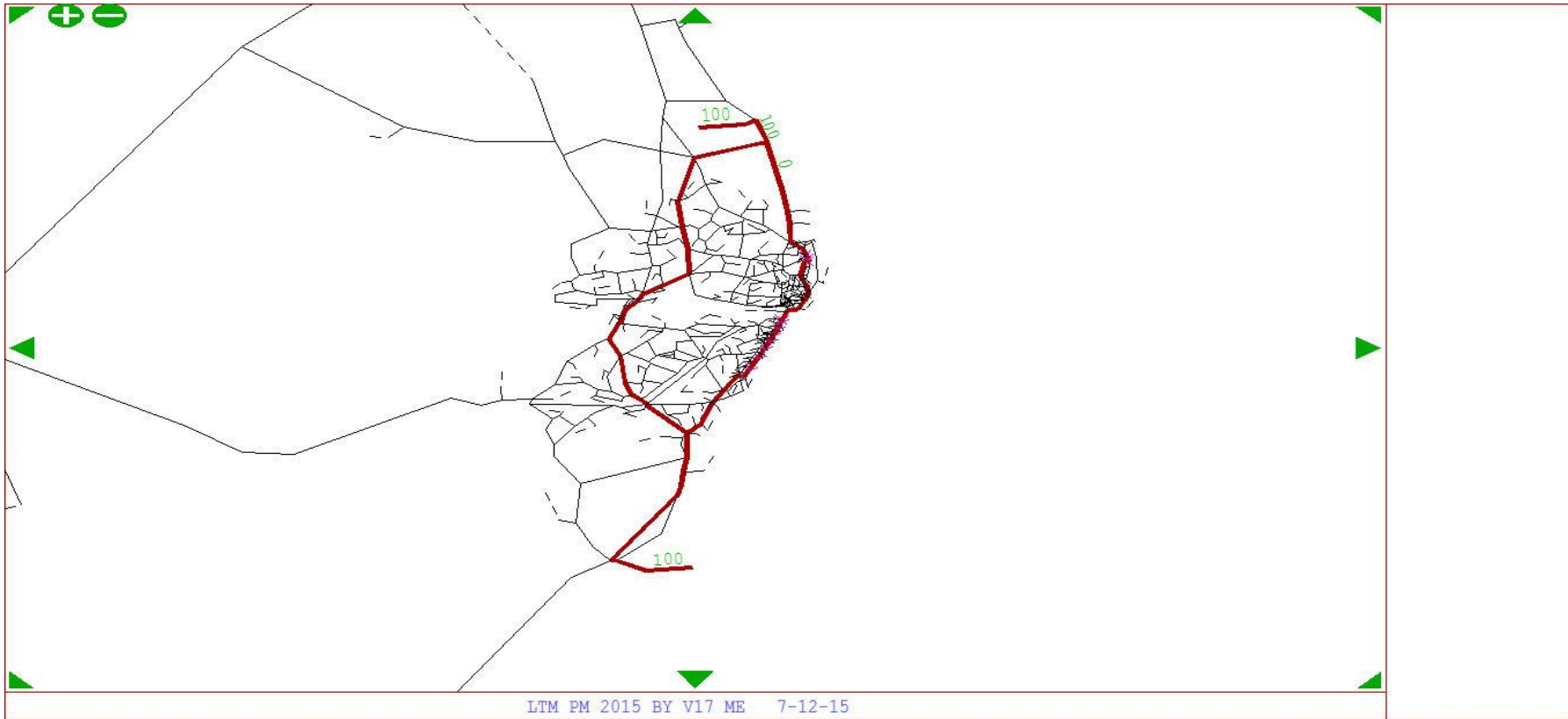


LTM PM 2015 BY V17 ME 7-12-15

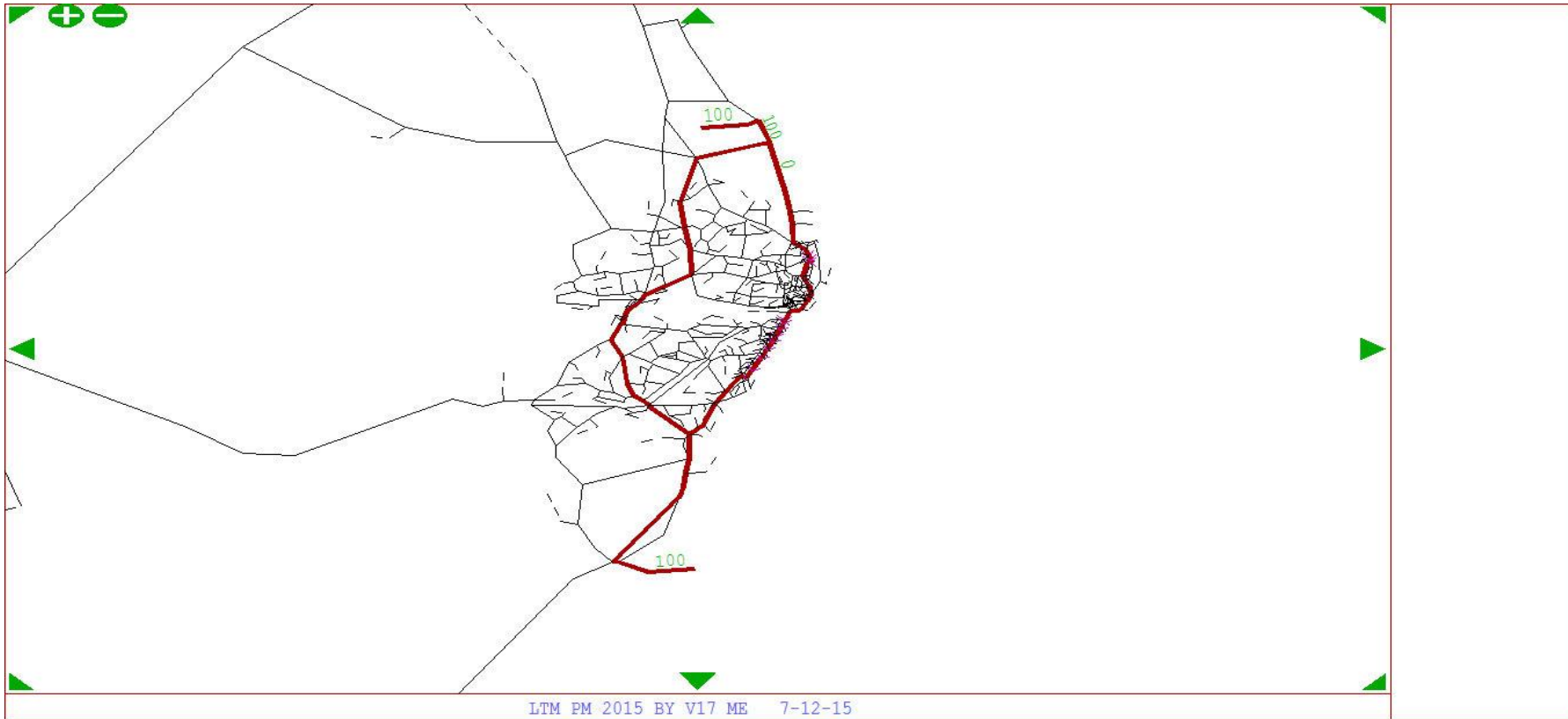
From Zone 102 To Zone 149 - User Class 1



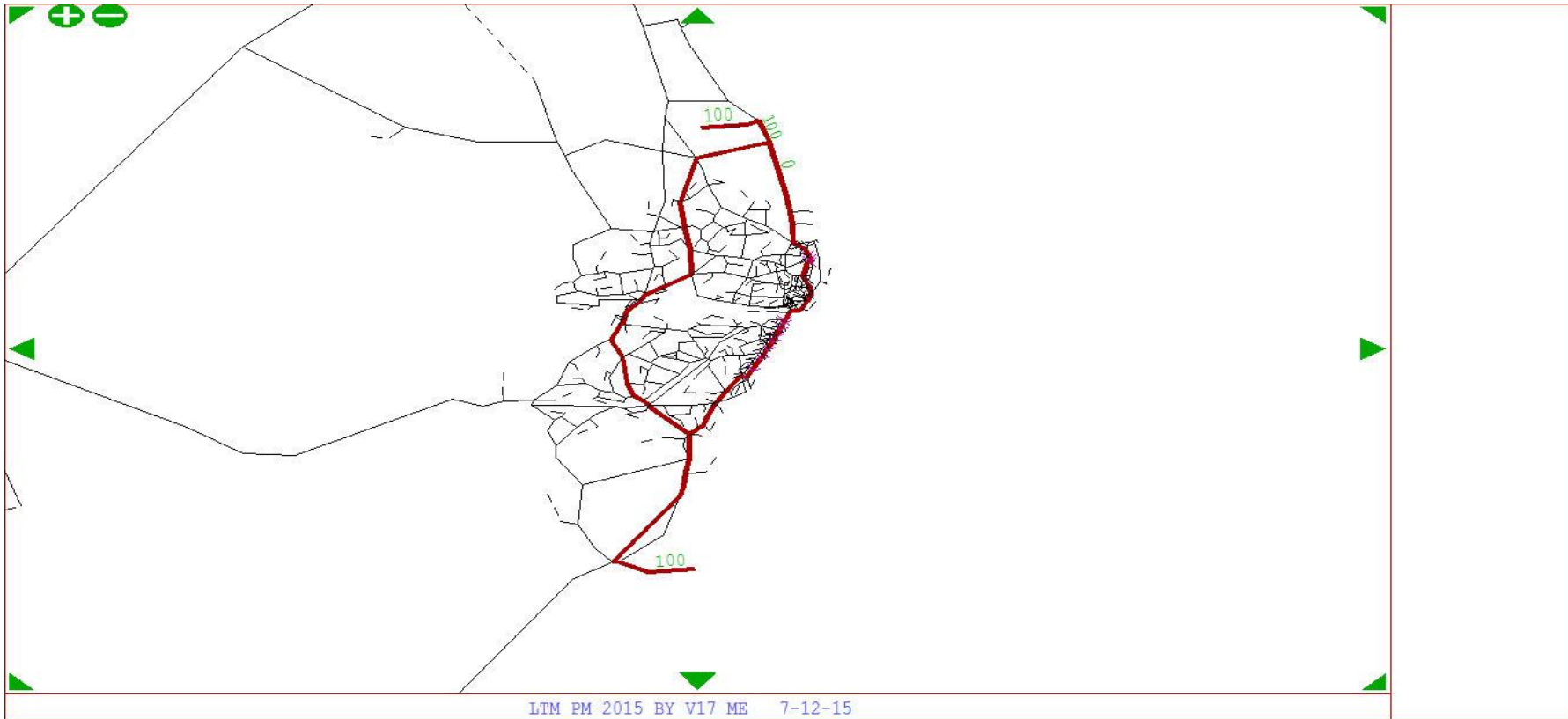
From Zone 102 To Zone 149 - User Class 2



From Zone 102 To Zone 149 - User Class 3

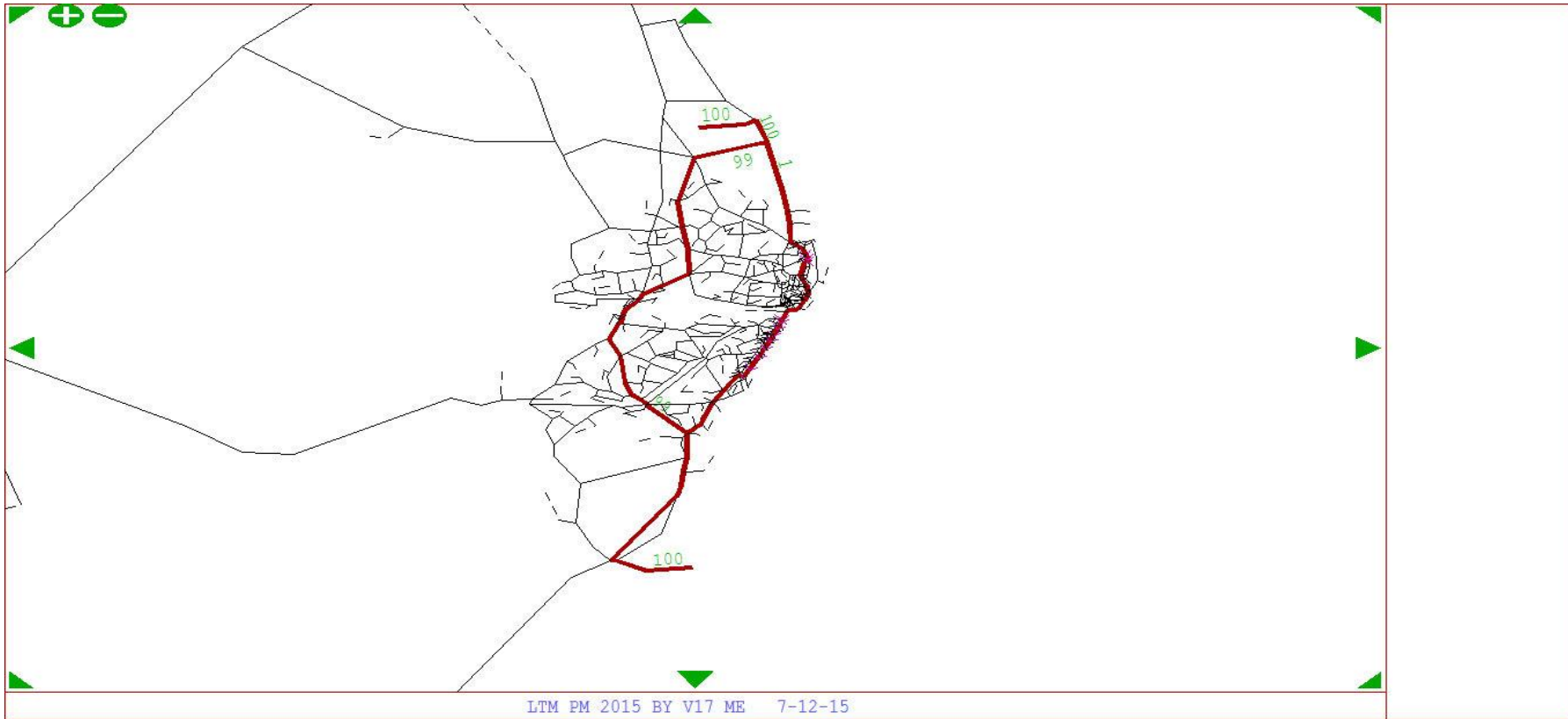


From Zone 102 To Zone 149 - User Class 4

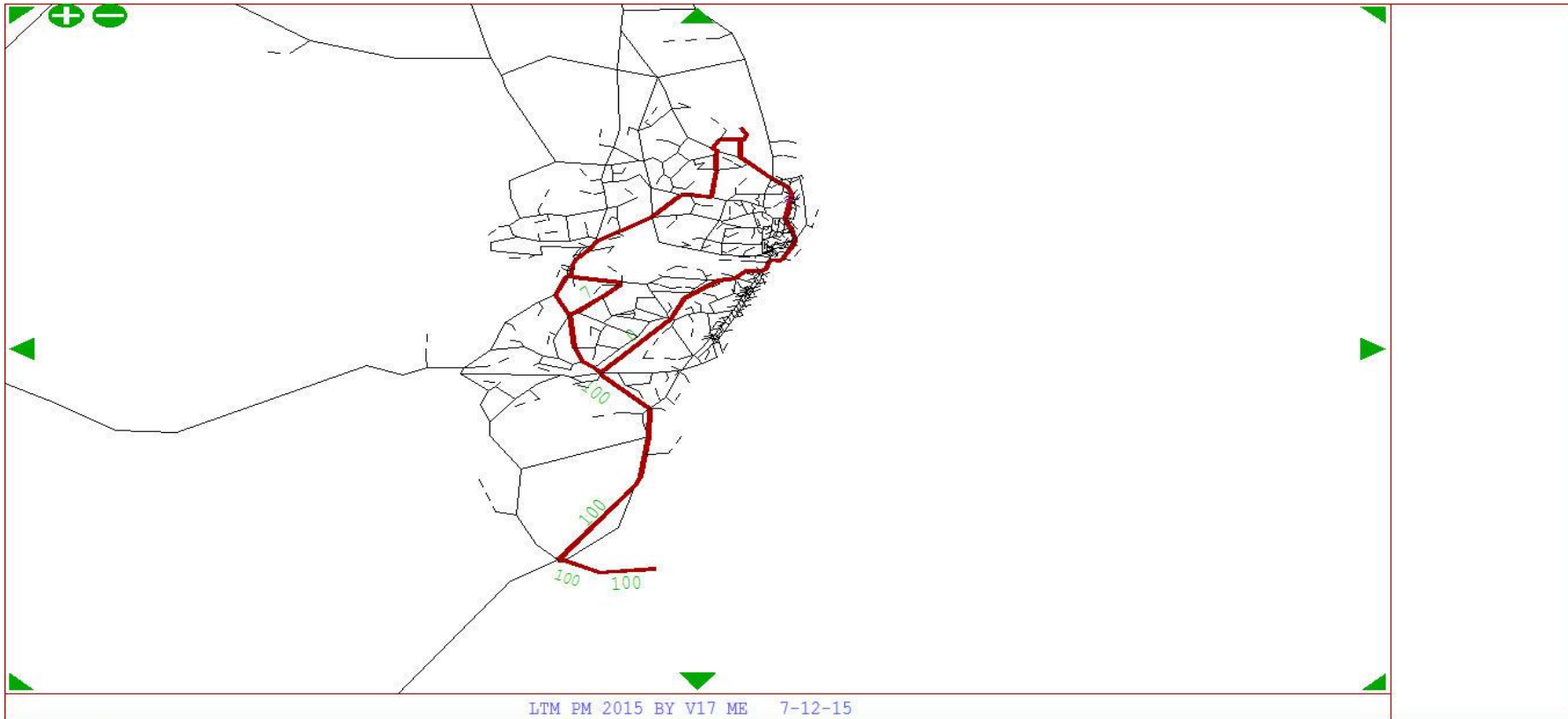


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From Zone 102 To Zone 149 - User Class 5

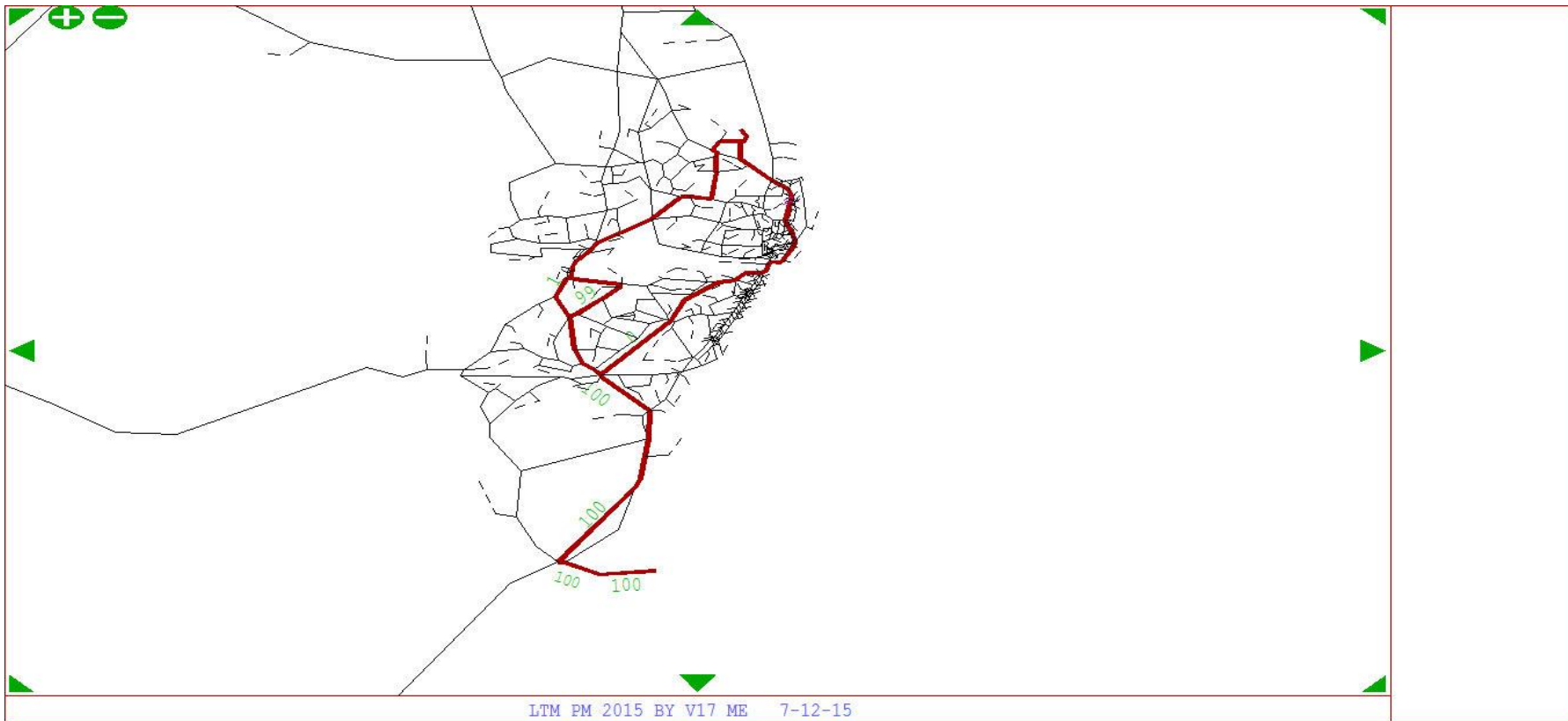


From Zone 149 To Zone 101 - User Class 1

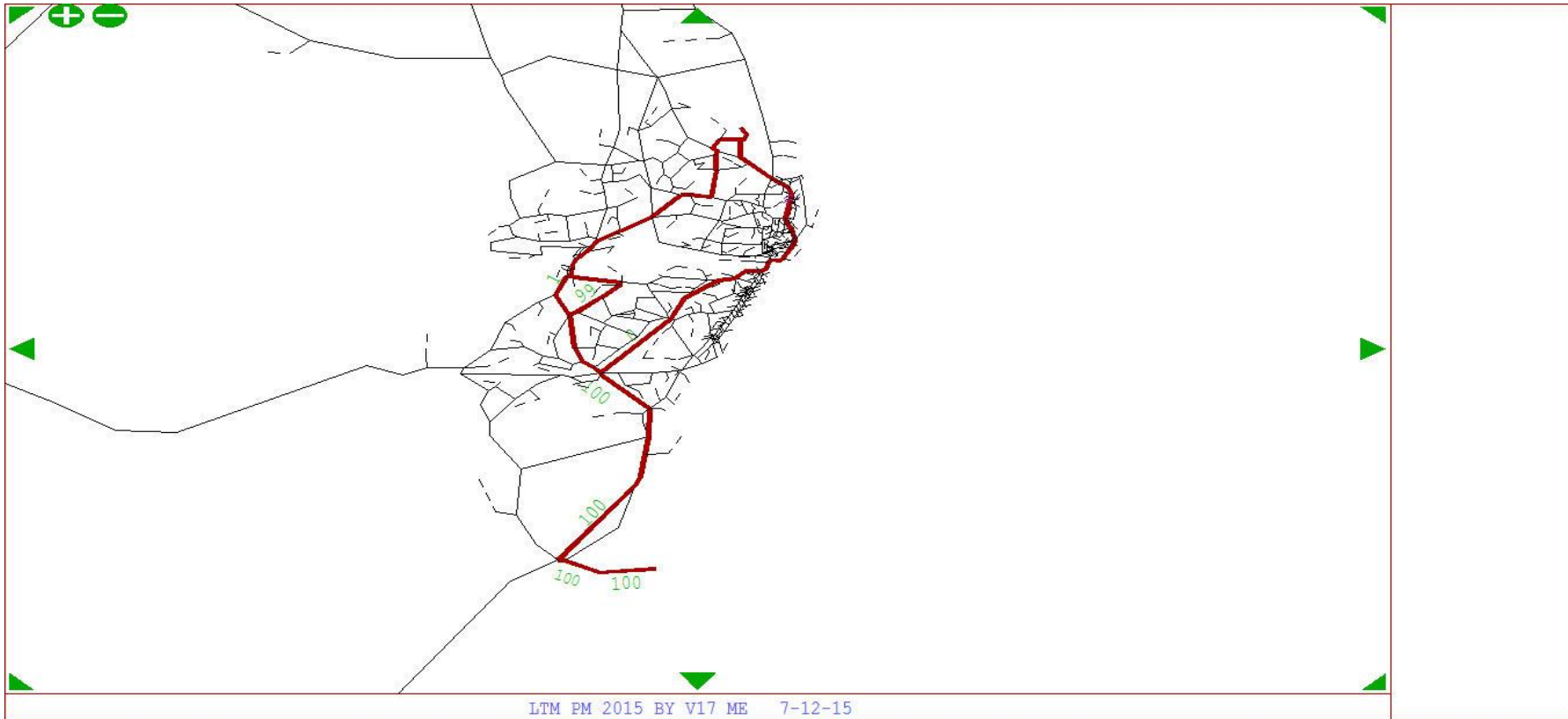


LTM PM 2015 BY V17 ME 7-12-15

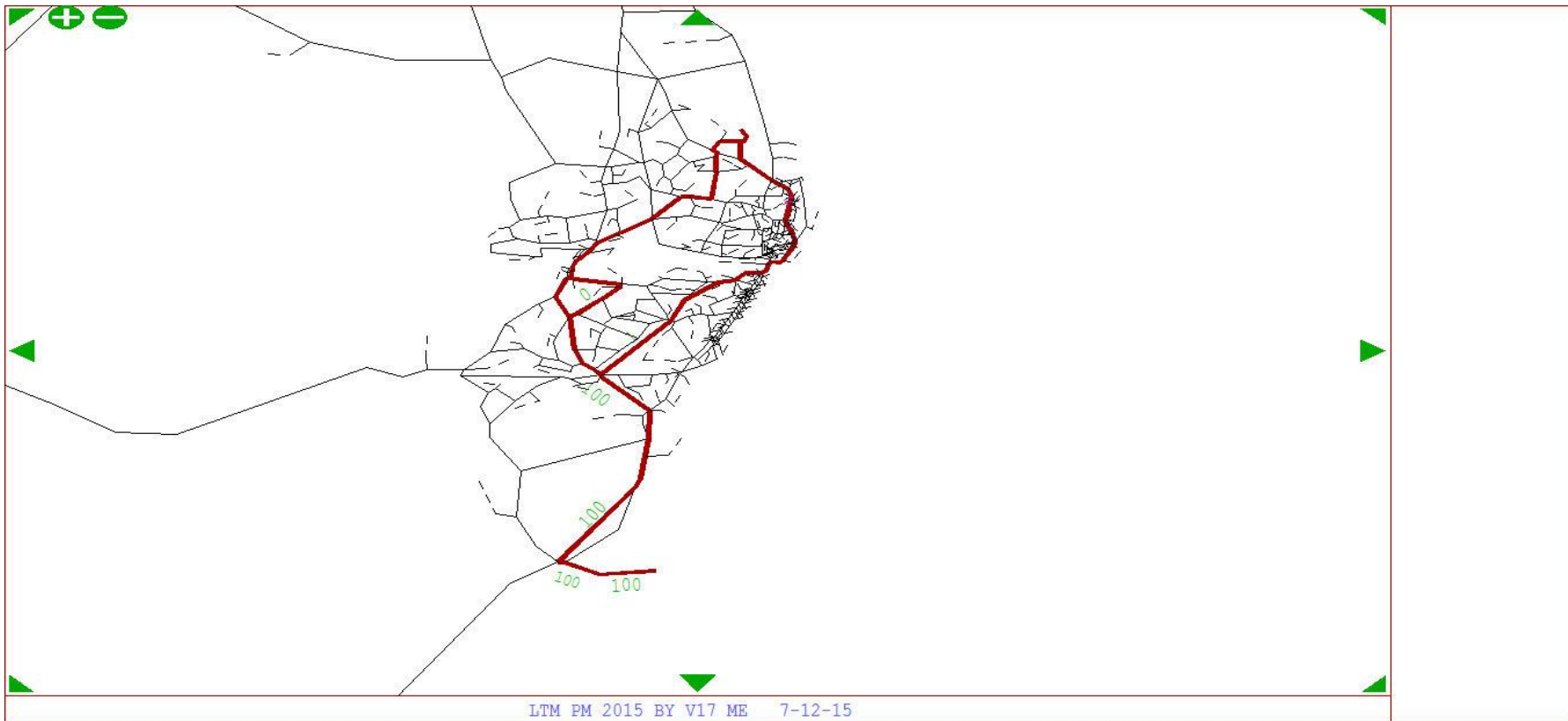
From Zone 149 To Zone 101 - User Class 2



From Zone 149 To Zone 101 - User Class 3

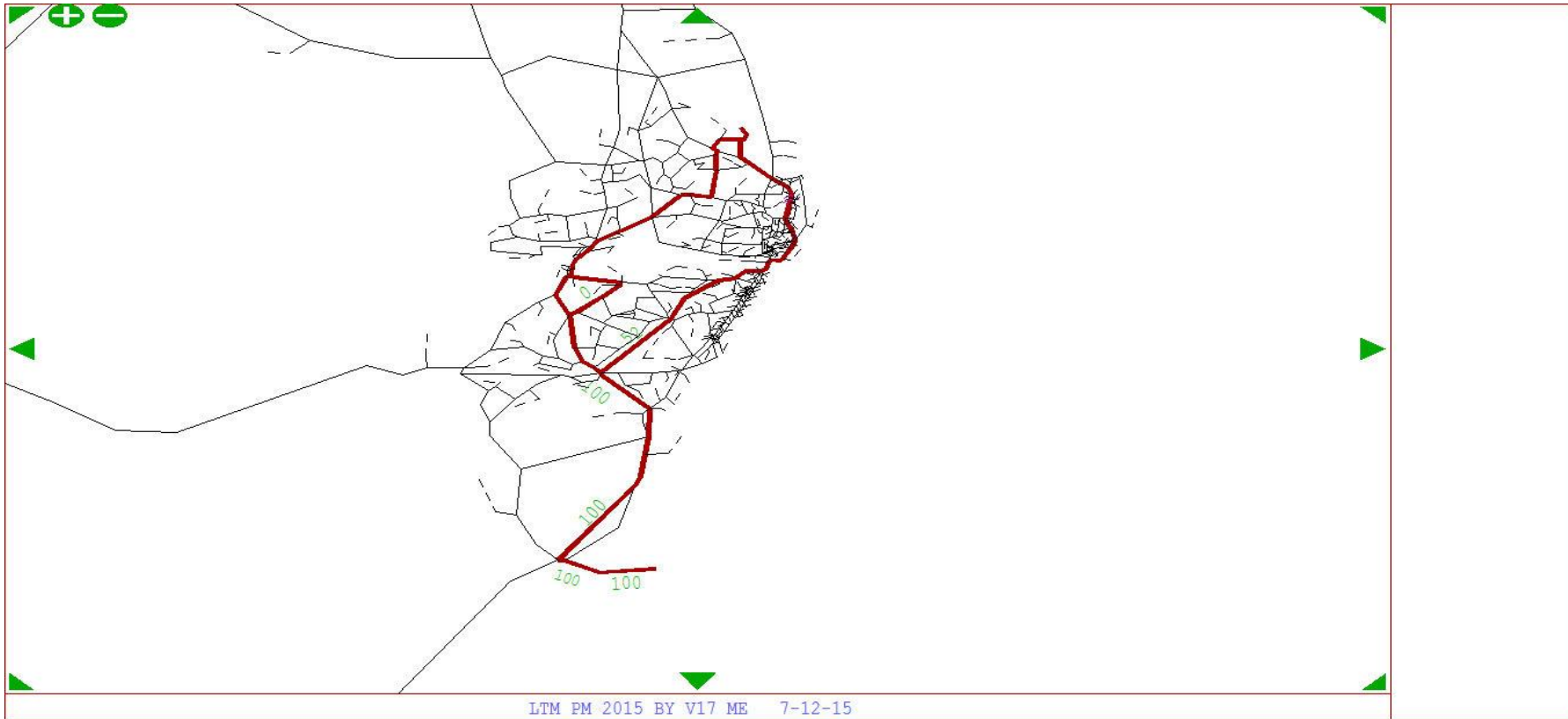


From Zone 149 To Zone 101 - User Class 4



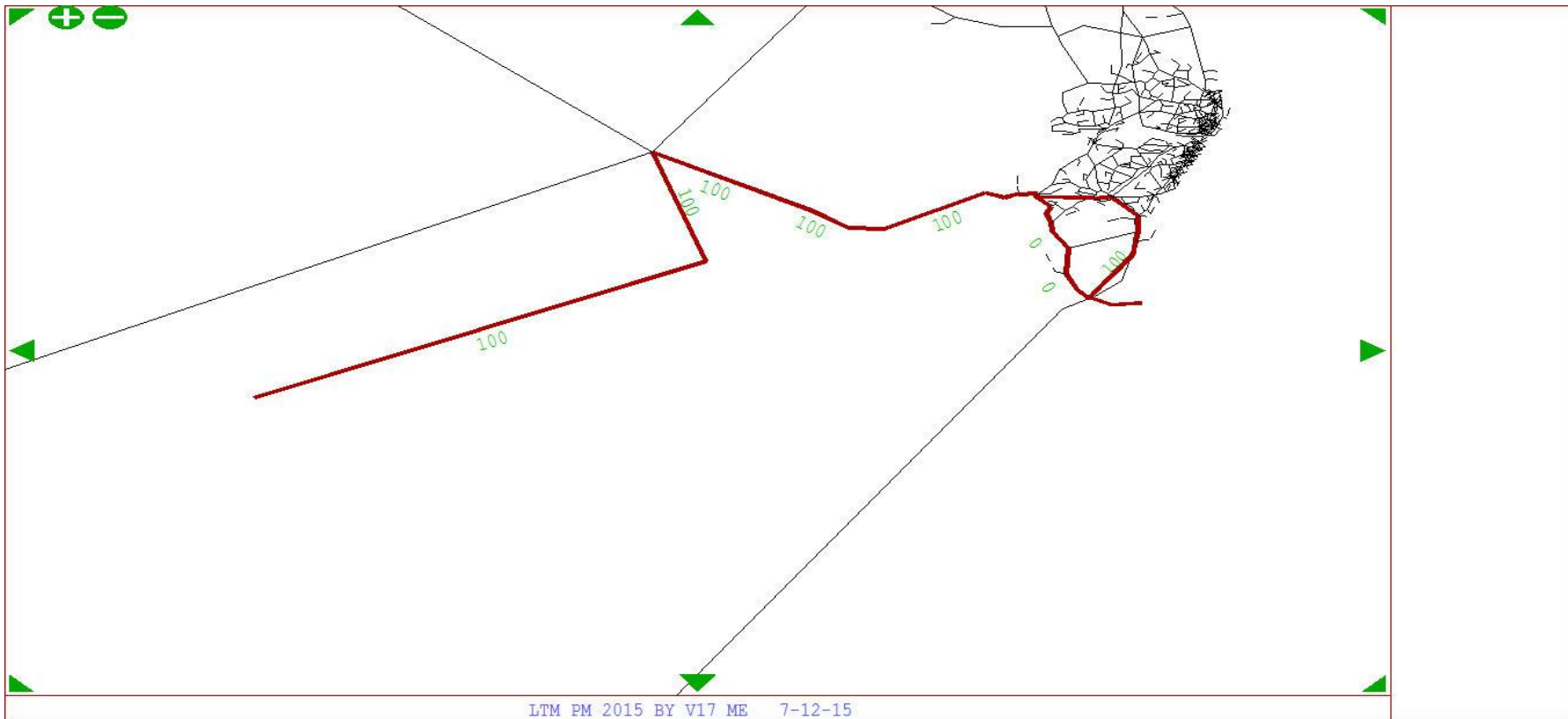
LTM PM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 101 - User Class 5

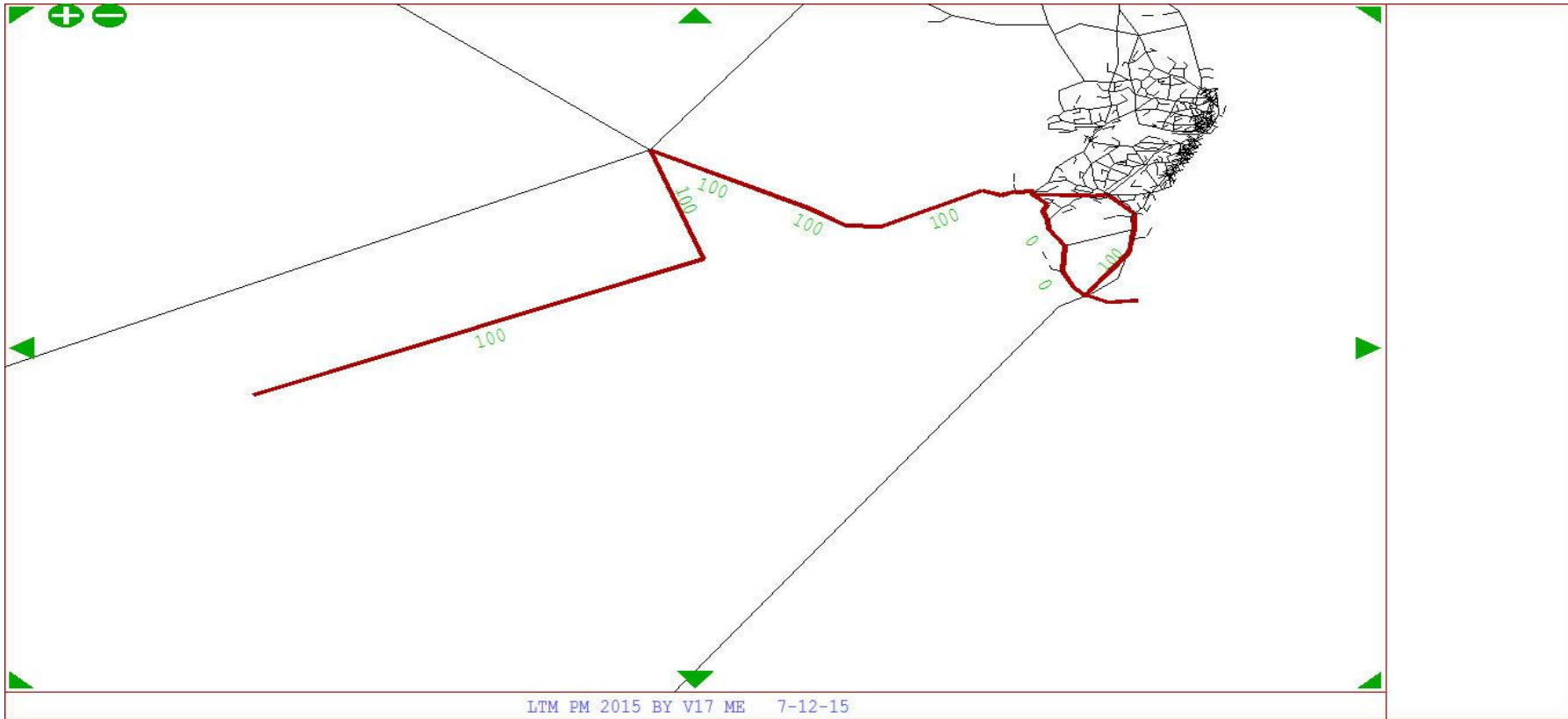


LTM PM 2015 BY V17 ME 7-12-15

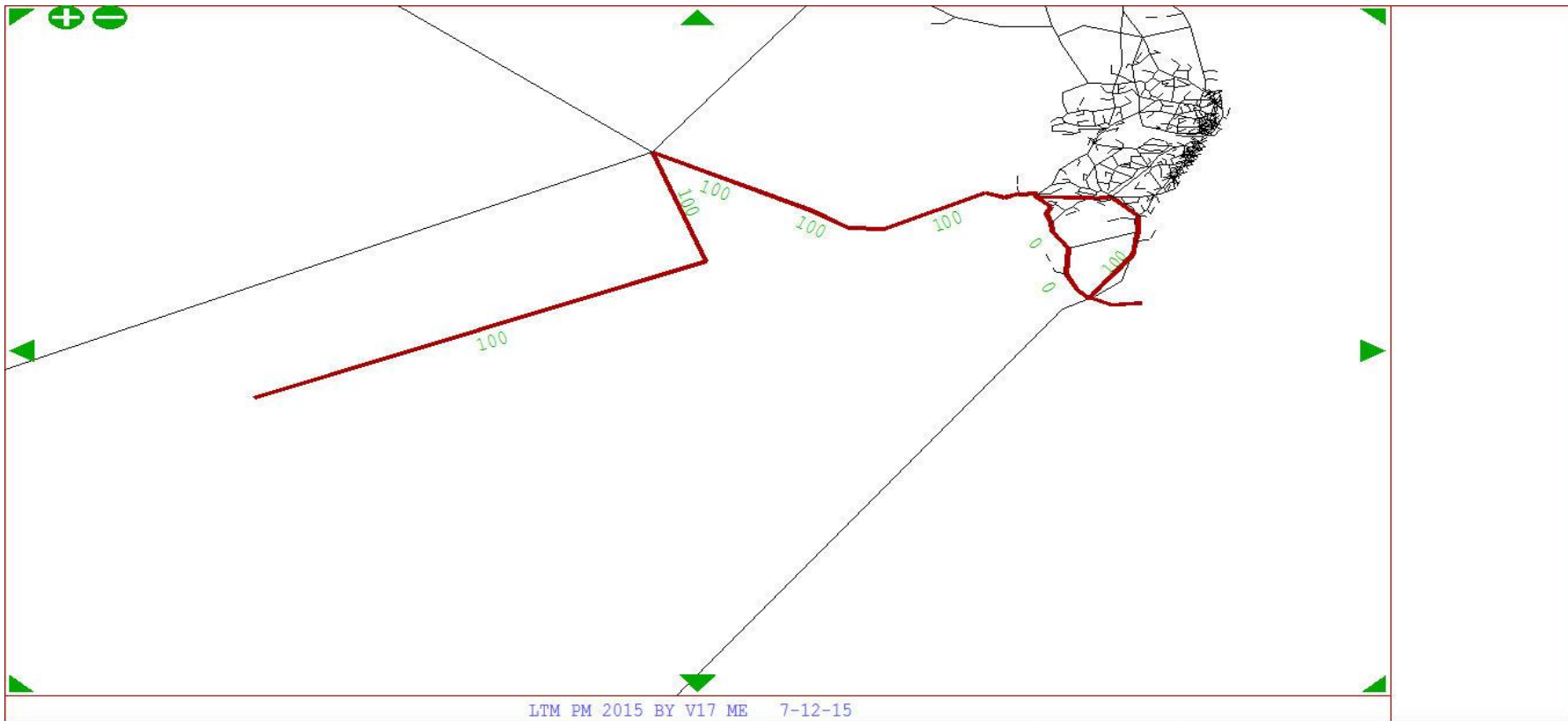
From Zone 149 To Zone 104 - User Class 1



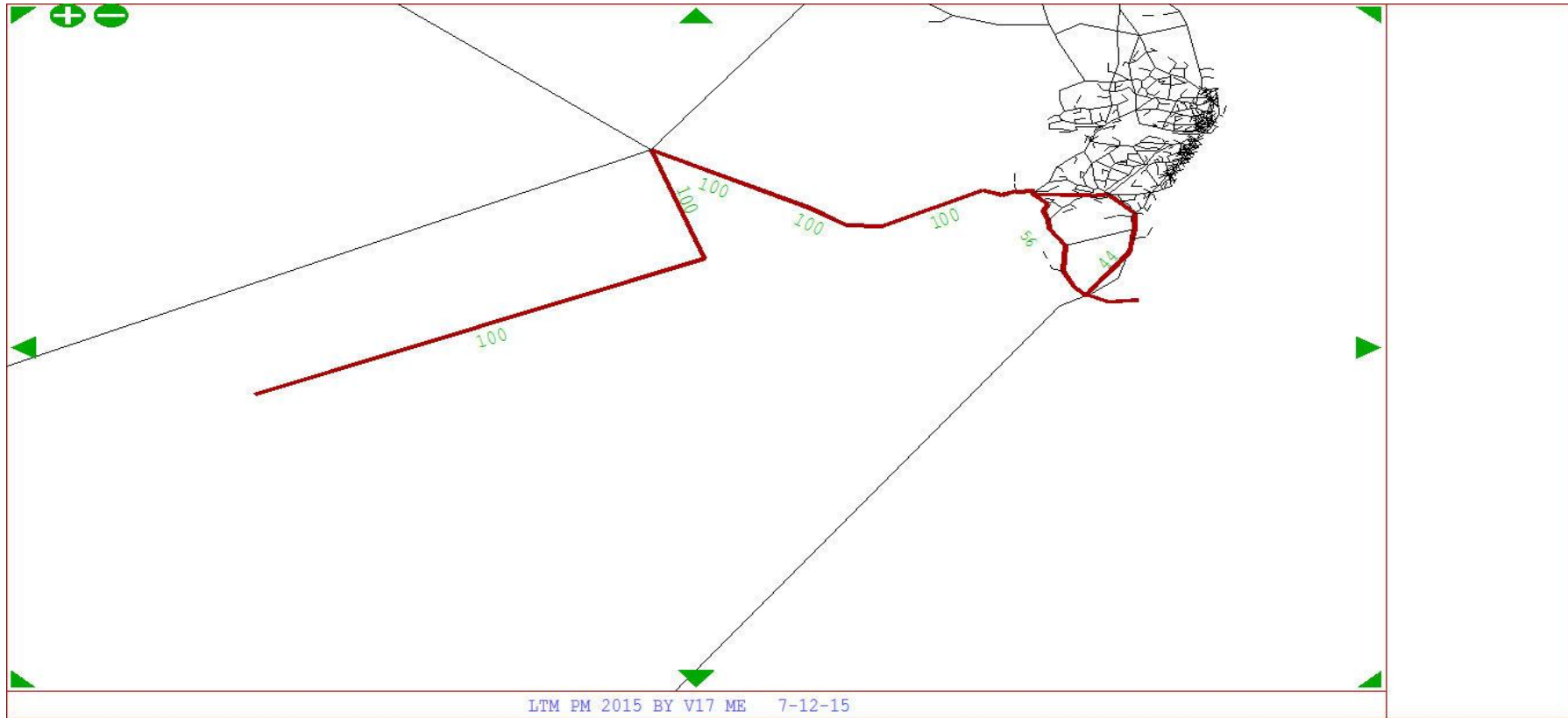
From Zone 149 To Zone 104 - User Class 2



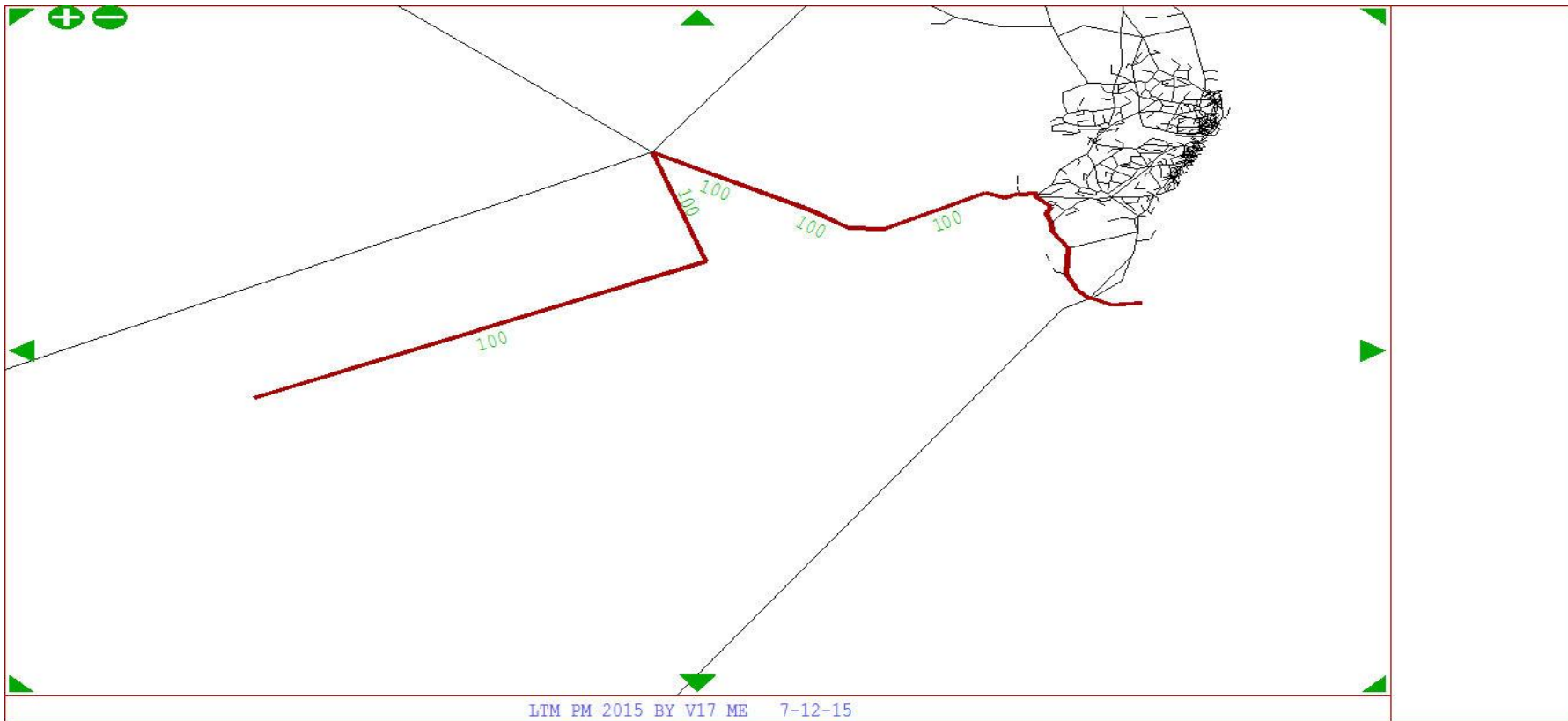
From Zone 149 To Zone 104 - User Class 3



From Zone 149 To Zone 104 - User Class 4

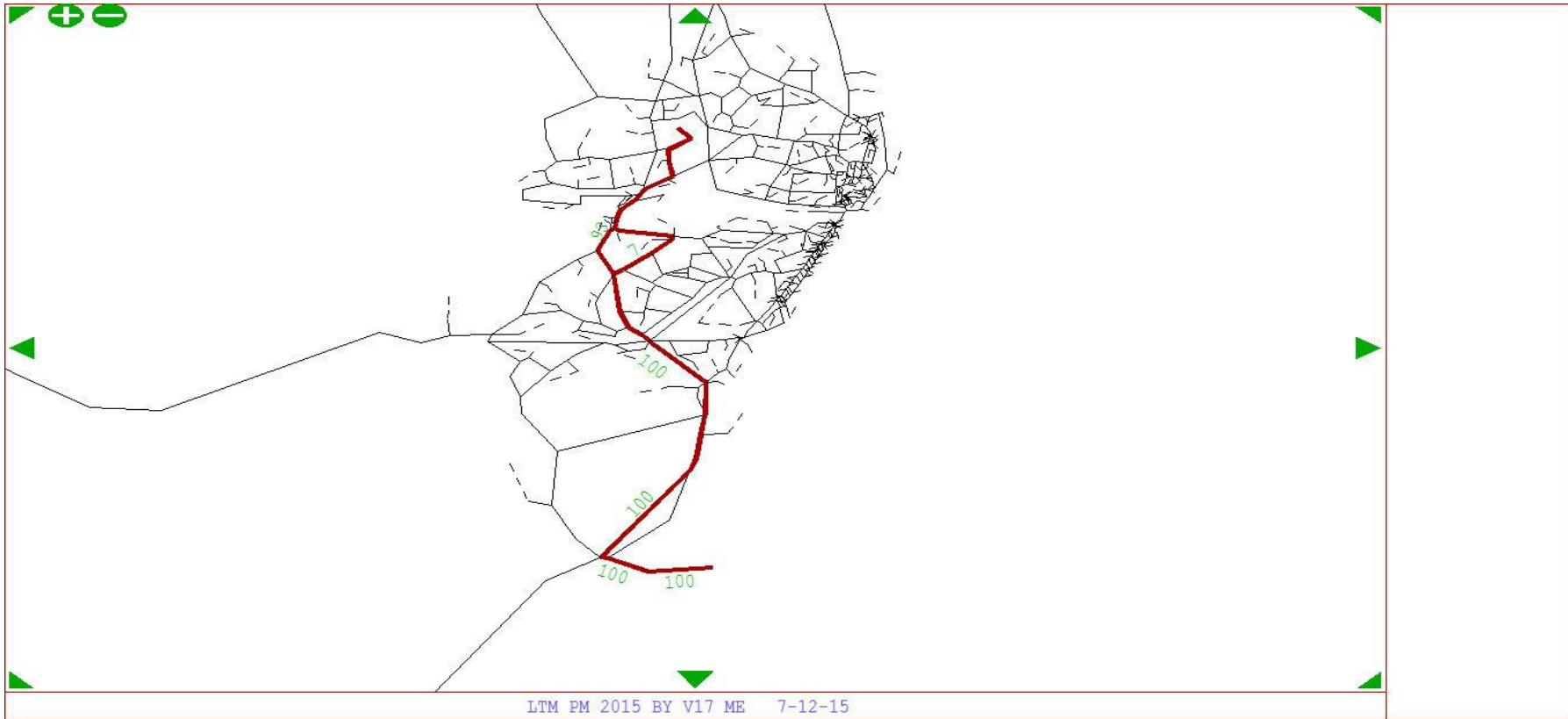


From Zone 149 To Zone 104 - User Class 5

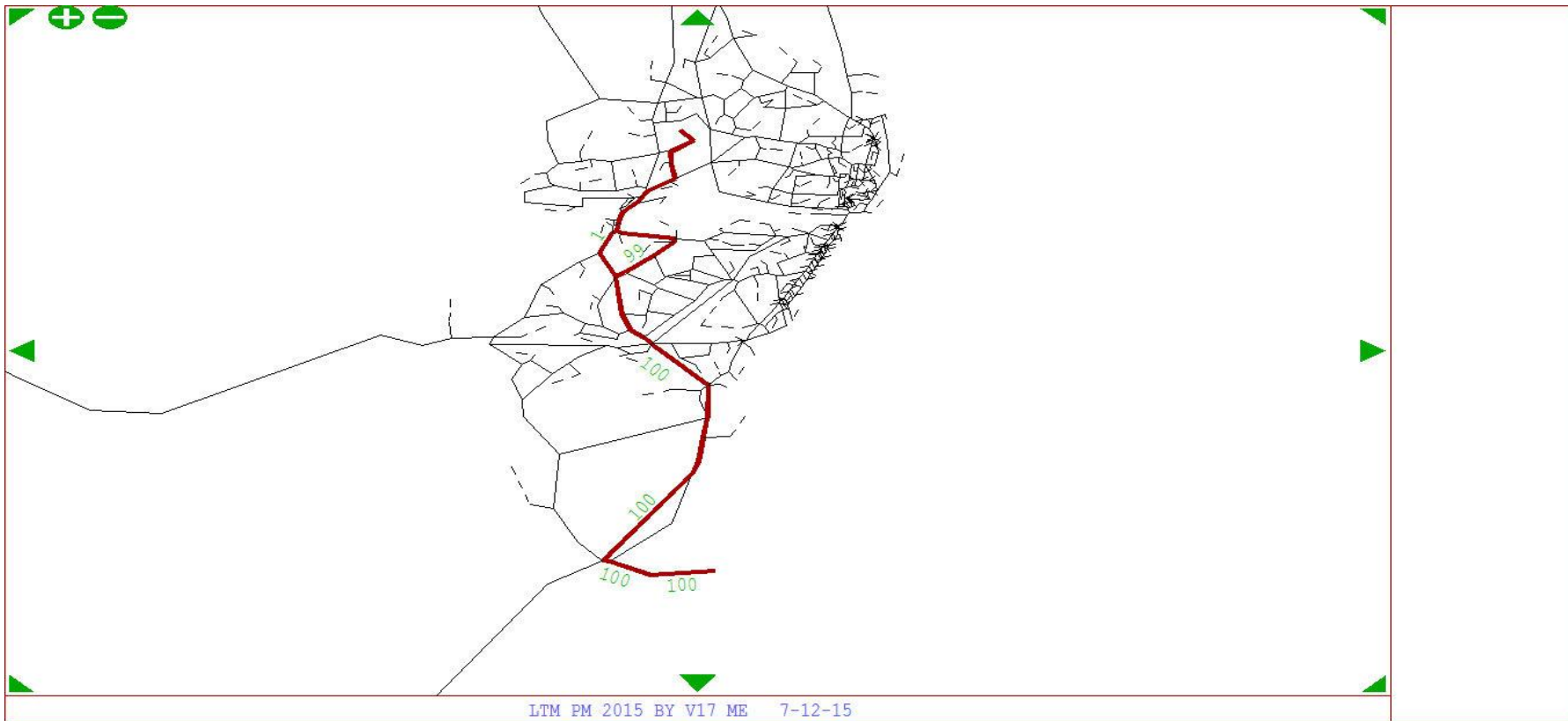


LTM PM 2015 BY V17 ME 7-12-15

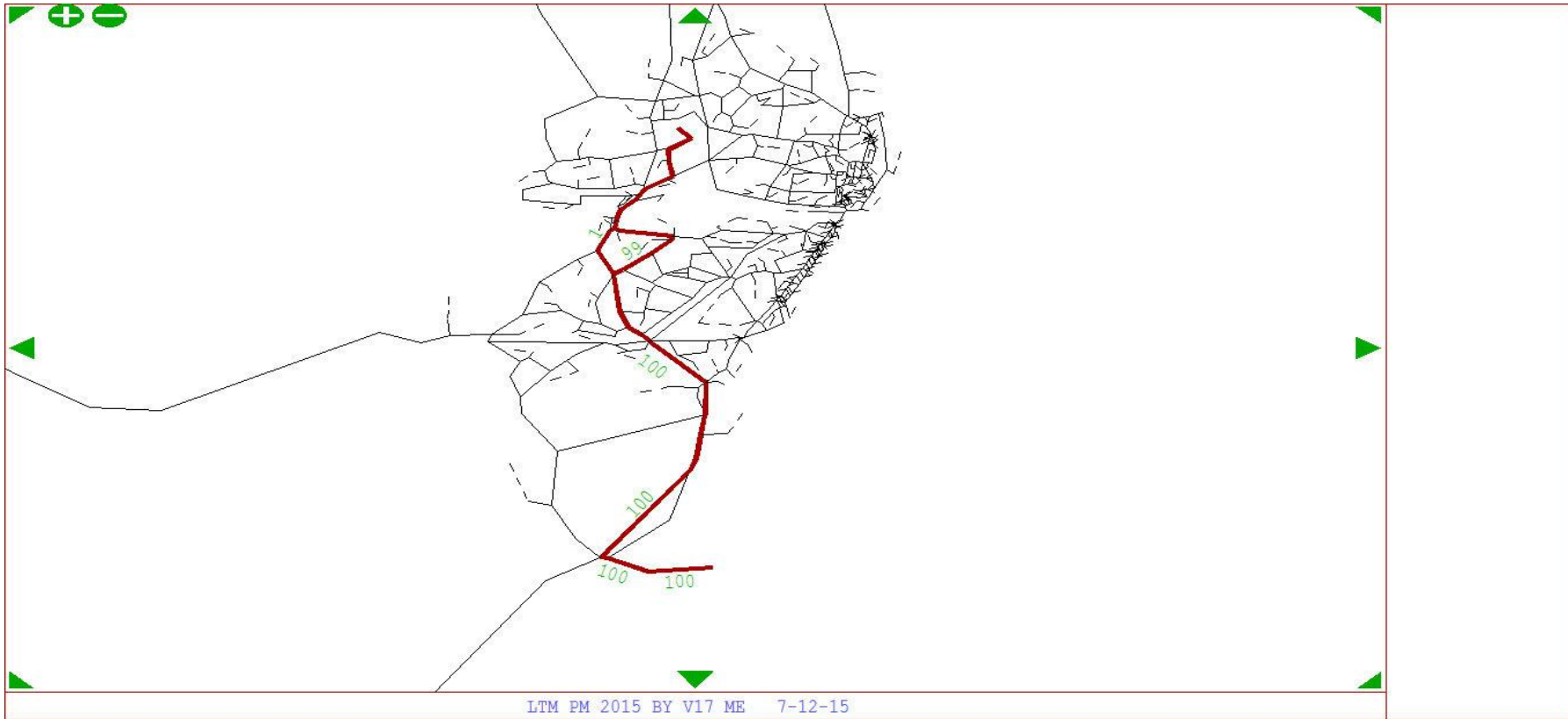
From Zone 149 To Zone 113 - User Class 1



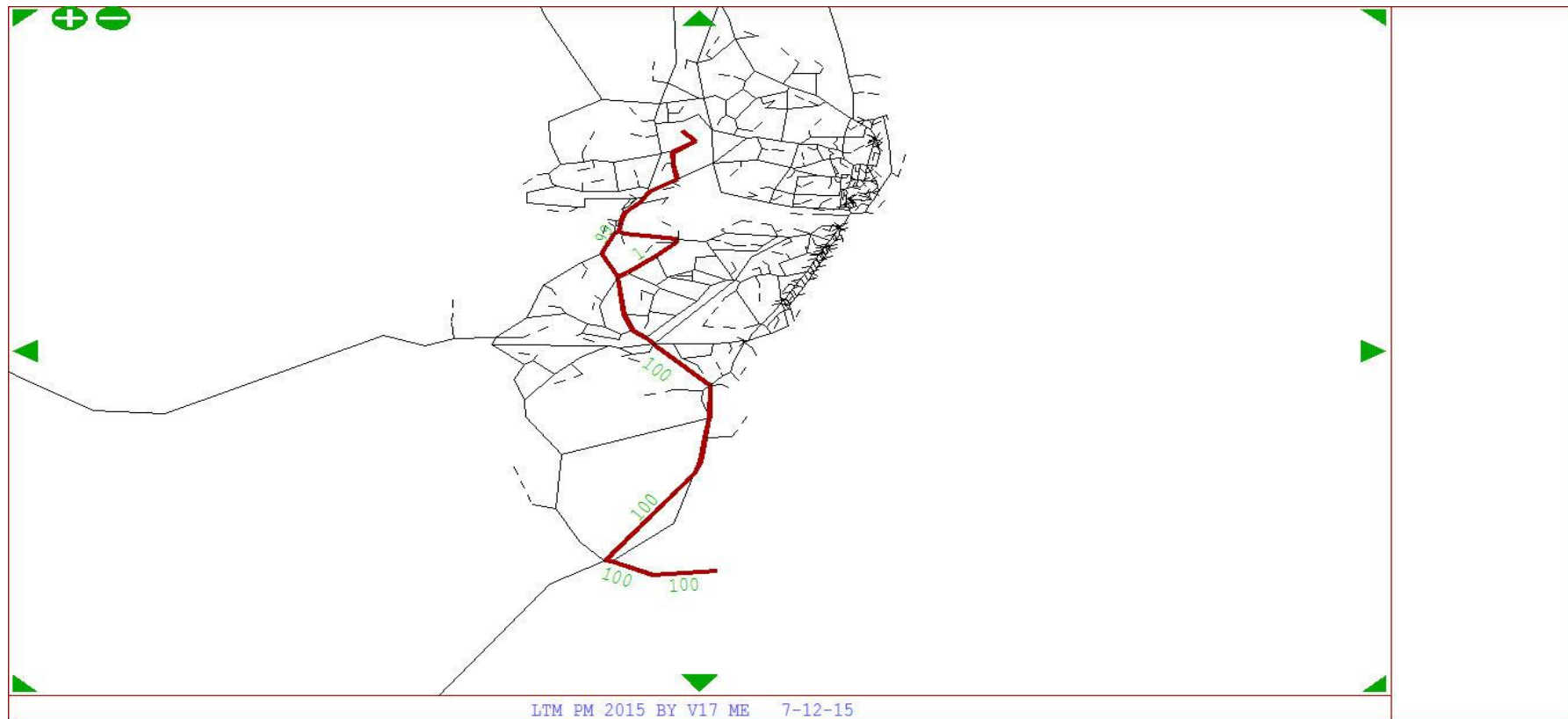
From Zone 149 To Zone 113 - User Class 2



From Zone 149 To Zone 113 - User Class 3



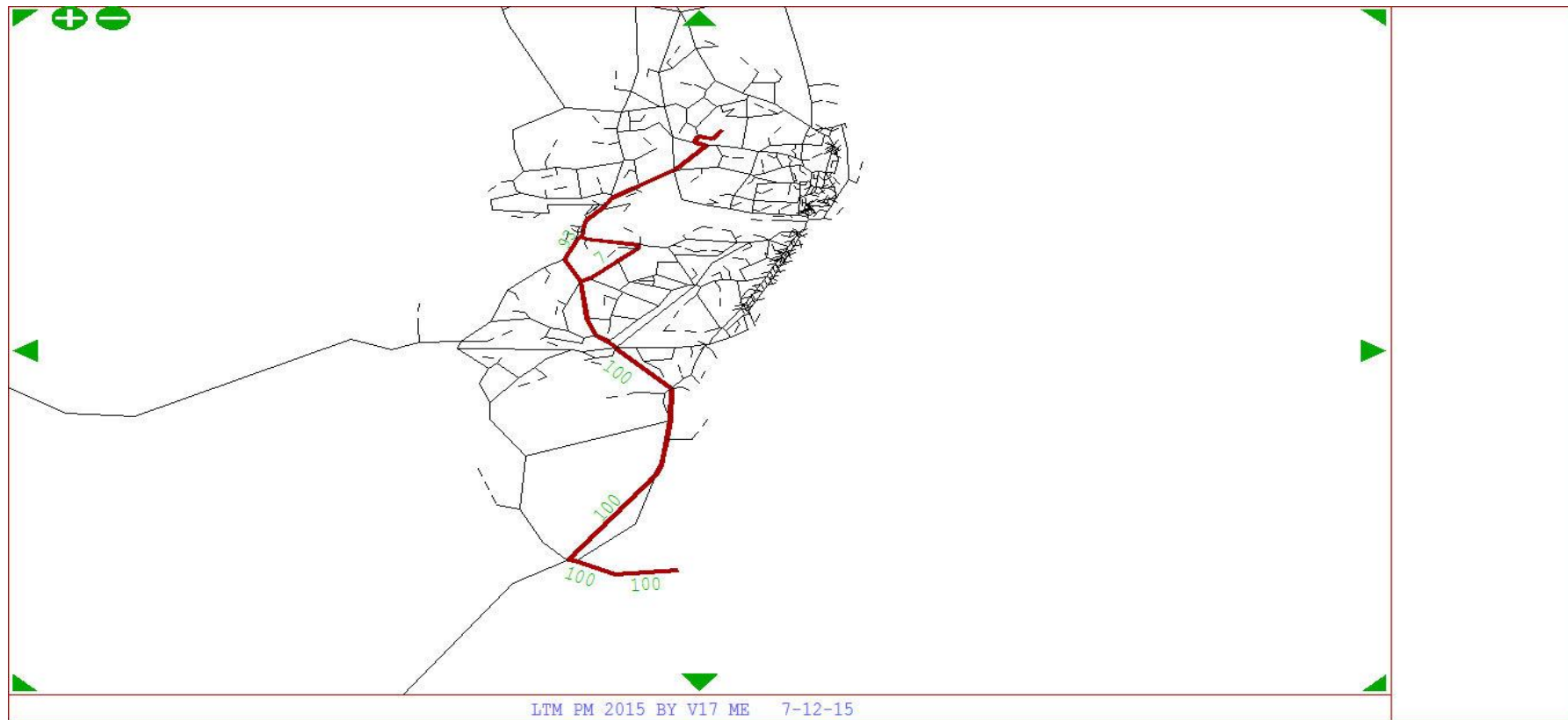
From Zone 149 To Zone 113 - User Class 4



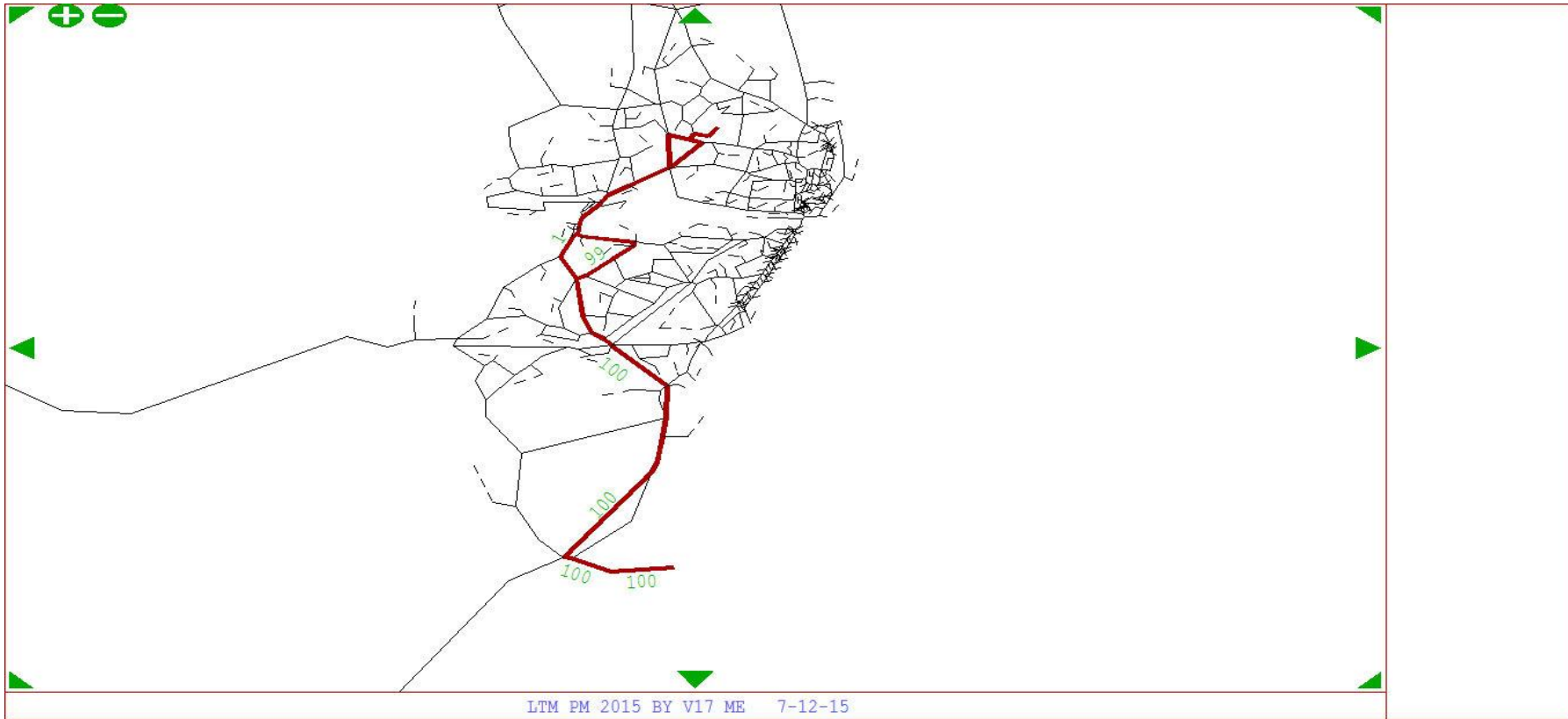
From Zone 149 To Zone 113 - User Class 5



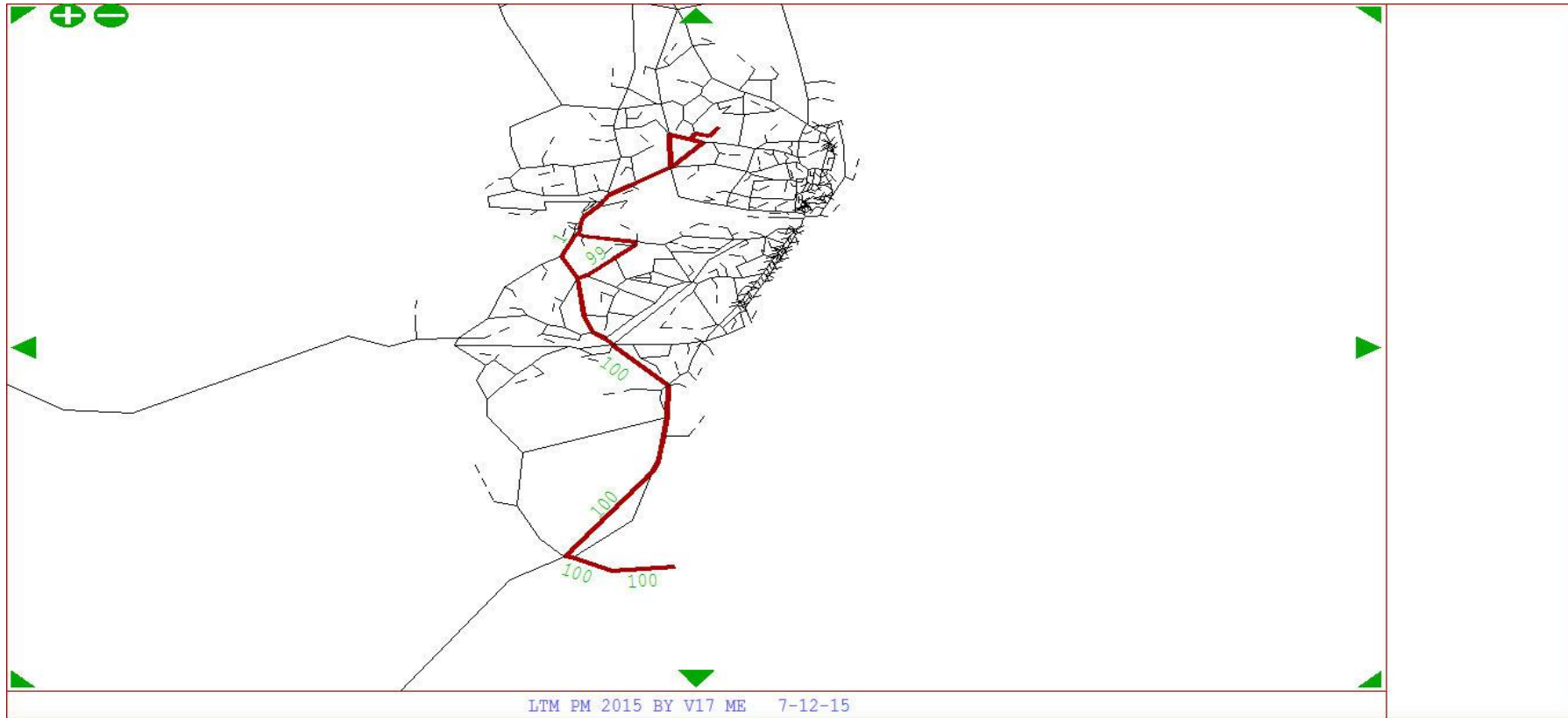
From Zone 149 To Zone 114 - User Class 1



From Zone 149 To Zone 114 - User Class 2

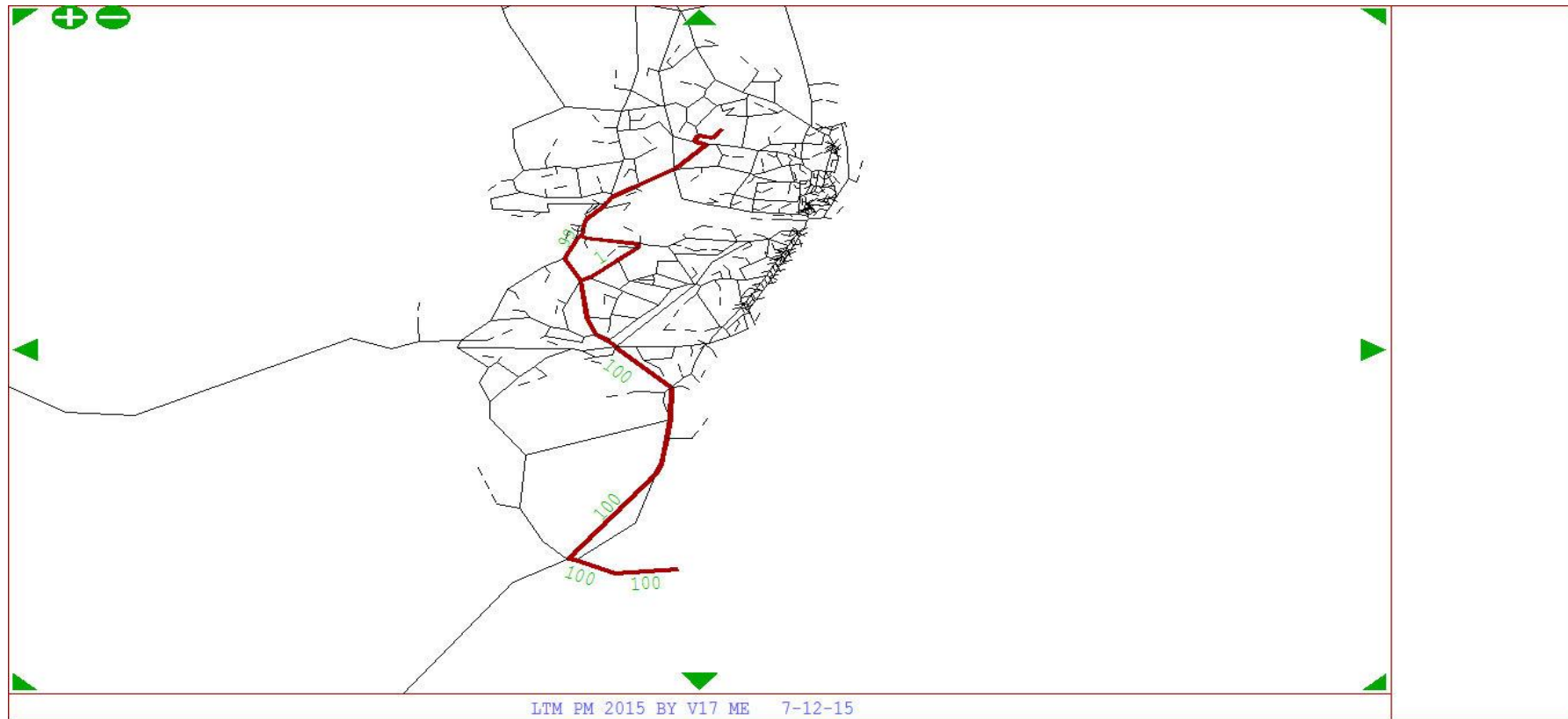


From Zone 149 To Zone 114 - User Class 3



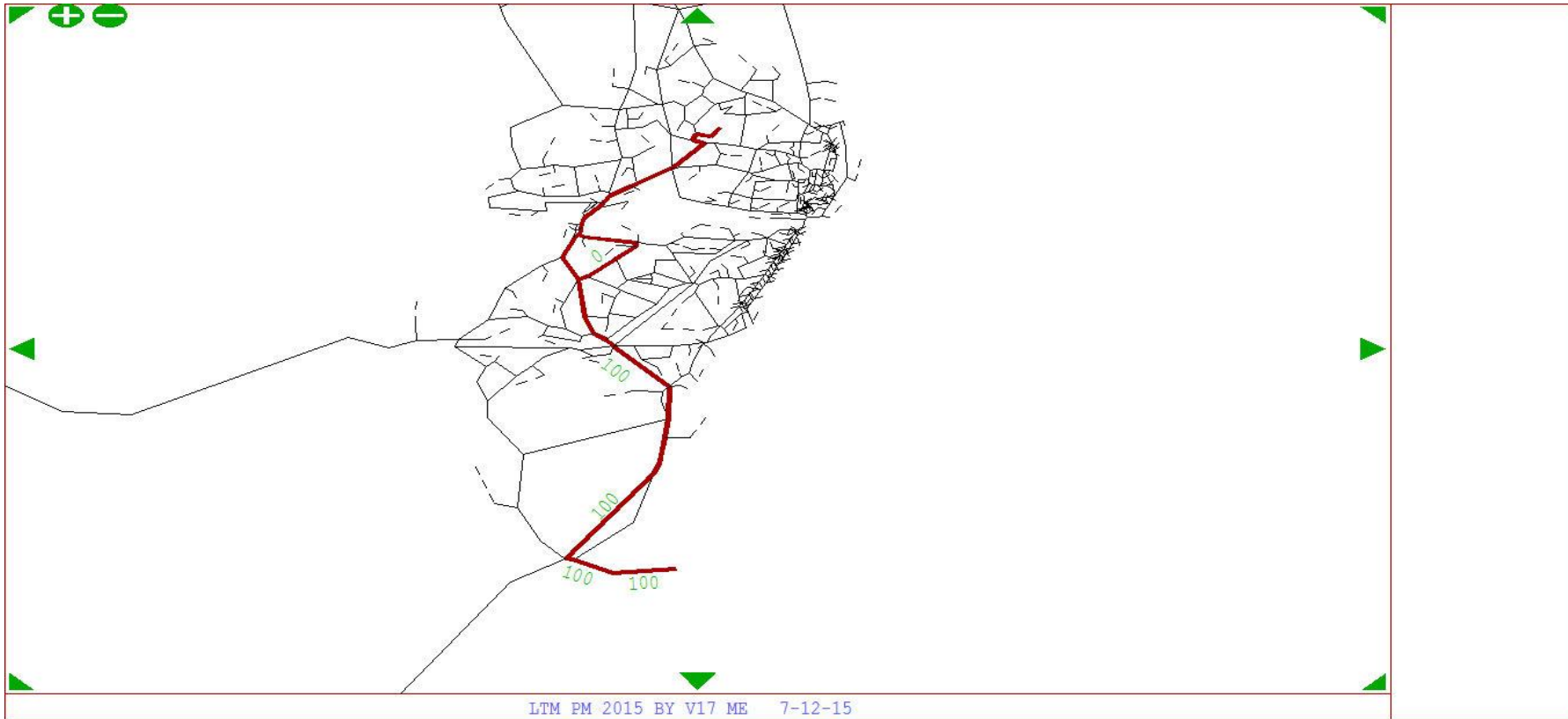
LTM PM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 114 - User Class 4



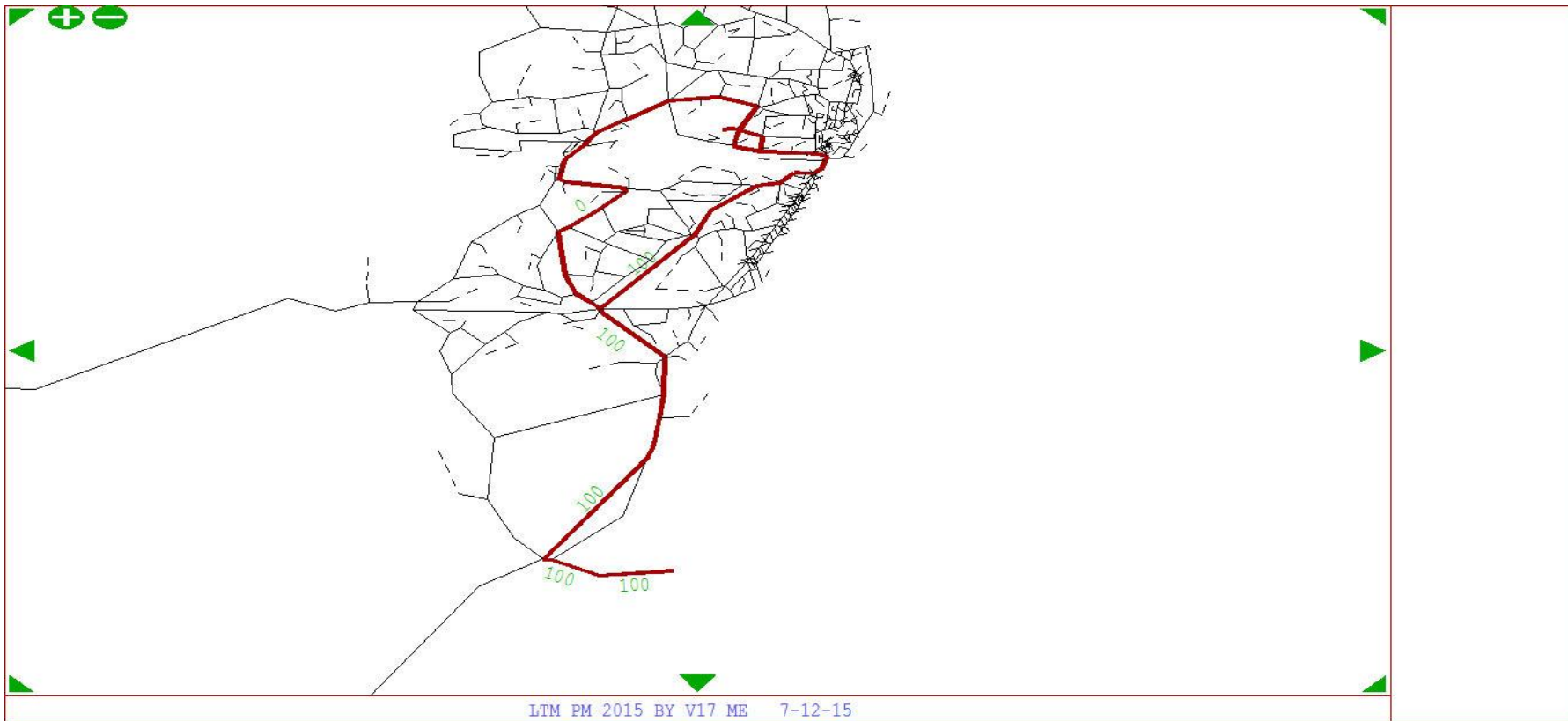
LTM PM 2015 BY V17 ME 7-12-15

From Zone 149 To Zone 114 - User Class 5

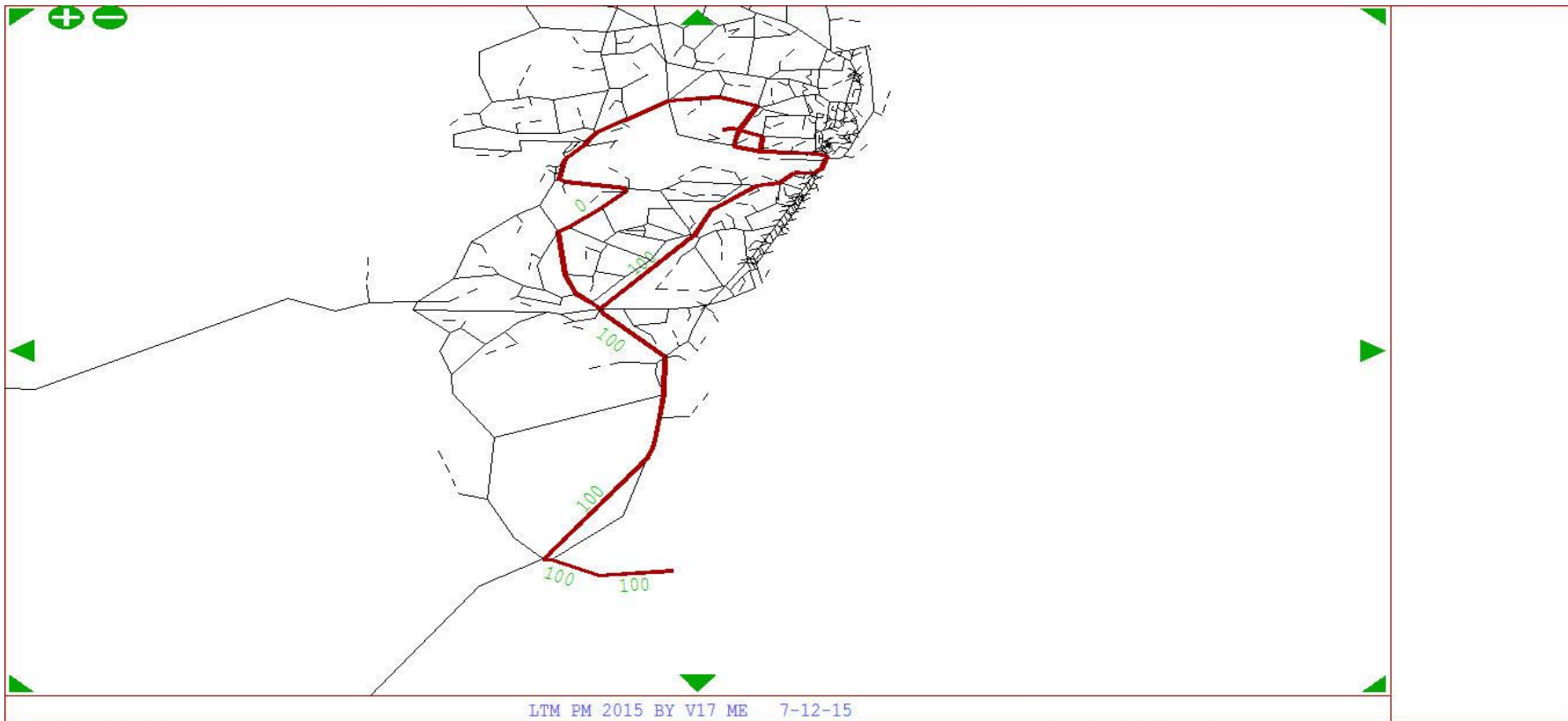


LTM PM 2015 BY V17 ME 7-12-15

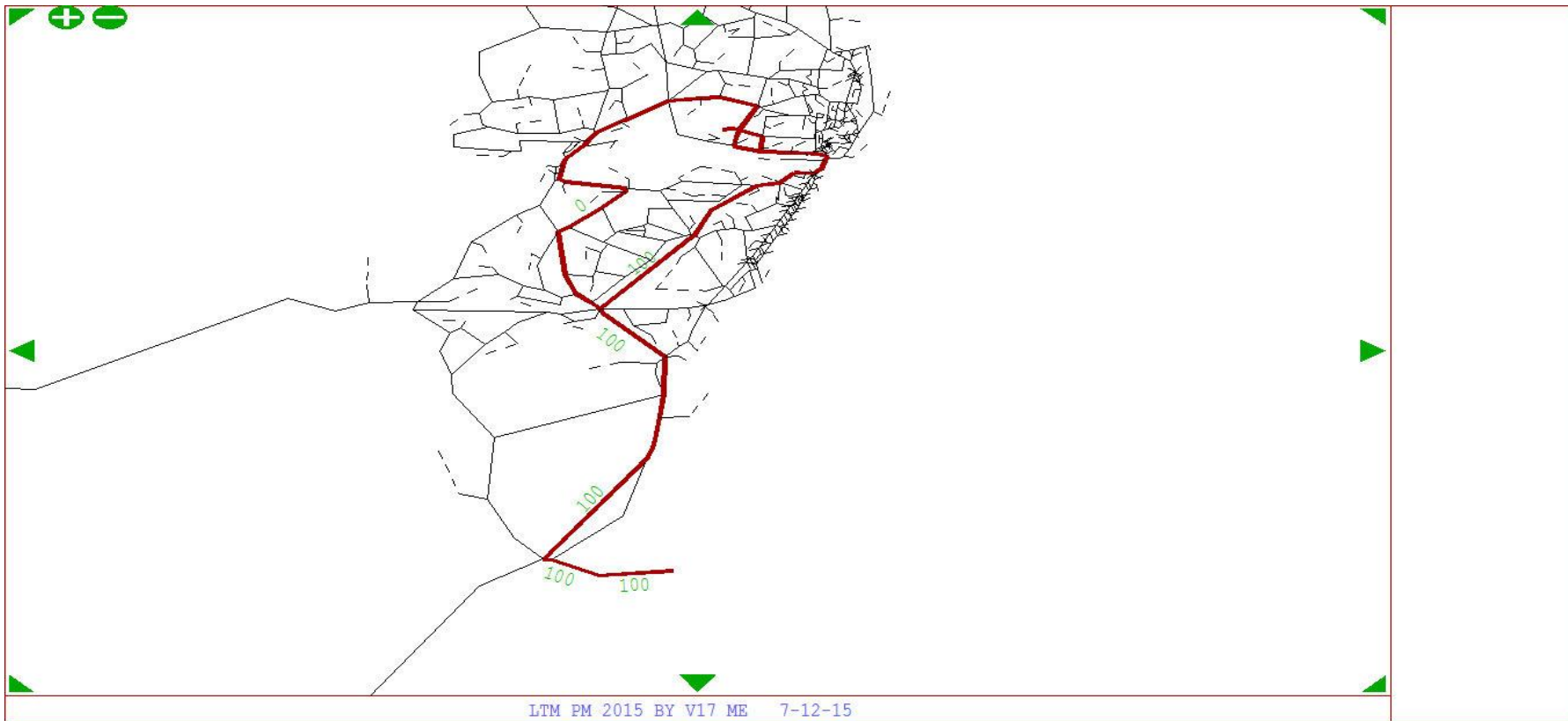
From Zone 149 To Zone 120 - User Class 1



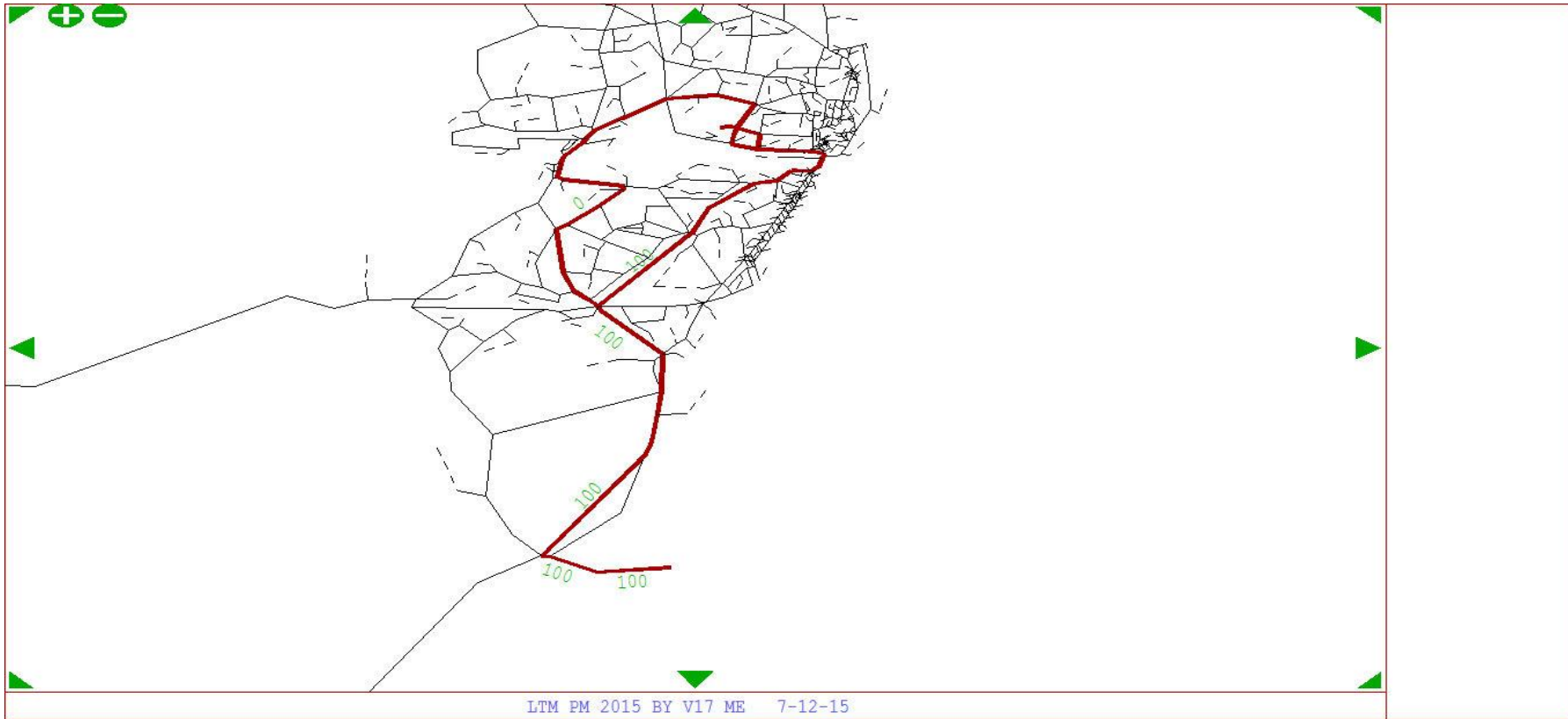
From Zone 149 To Zone 120 - User Class 2



From Zone 149 To Zone 120 - User Class 3

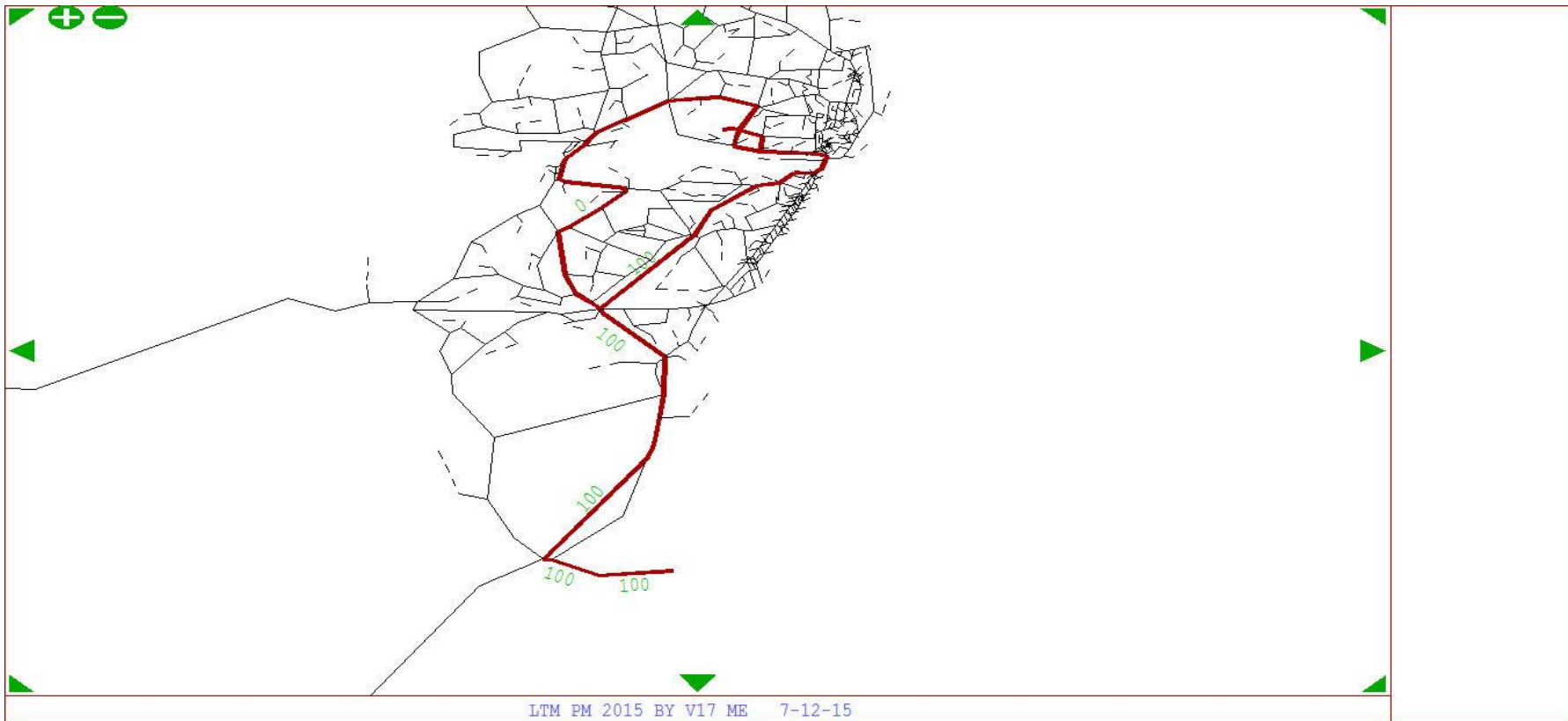


From Zone 149 To Zone 120 - User Class 4

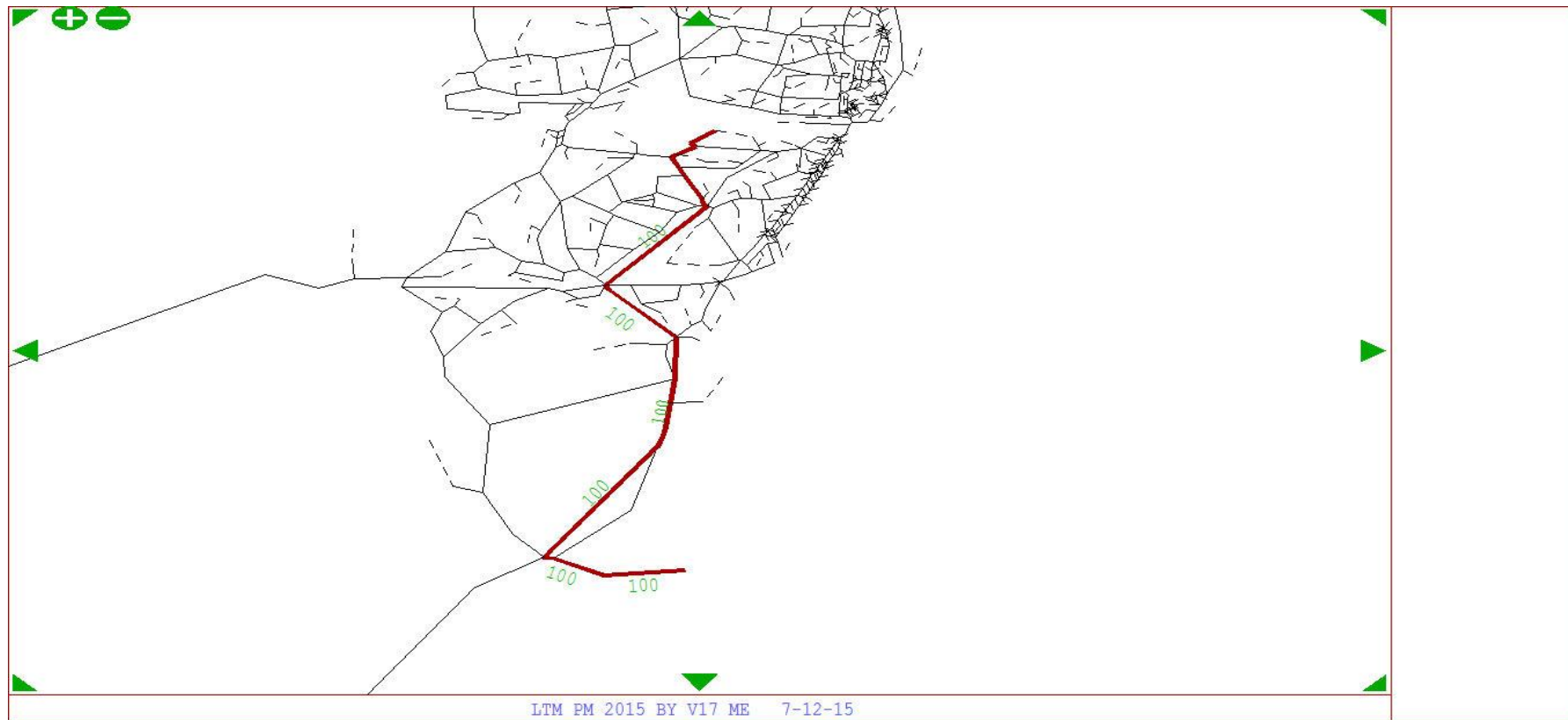


LTM PM 2015 BY V17 ME 7-12-15

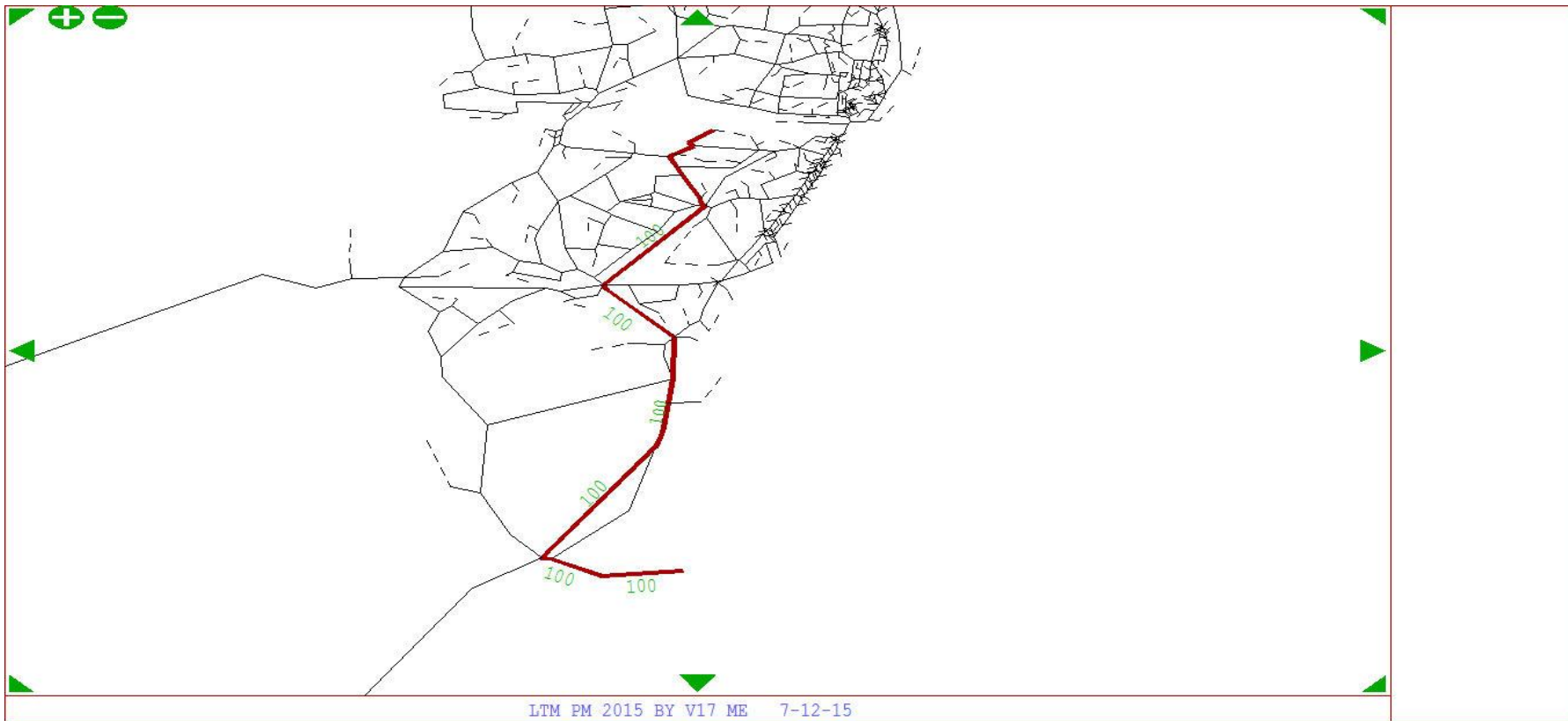
From Zone 149 To Zone 120 - User Class 5



From Zone 149 To Zone 122 - User Class 1

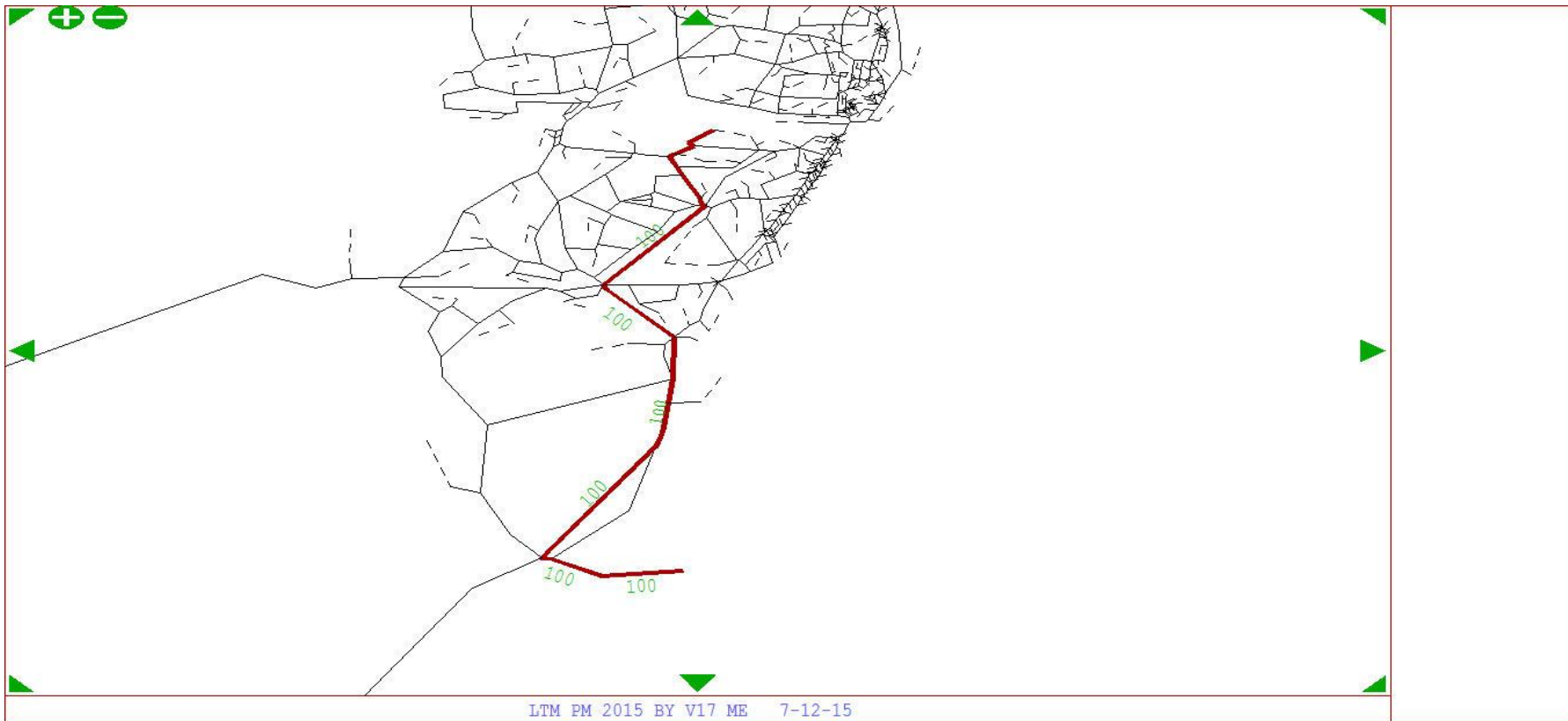


From Zone 149 To Zone 122 - User Class 2

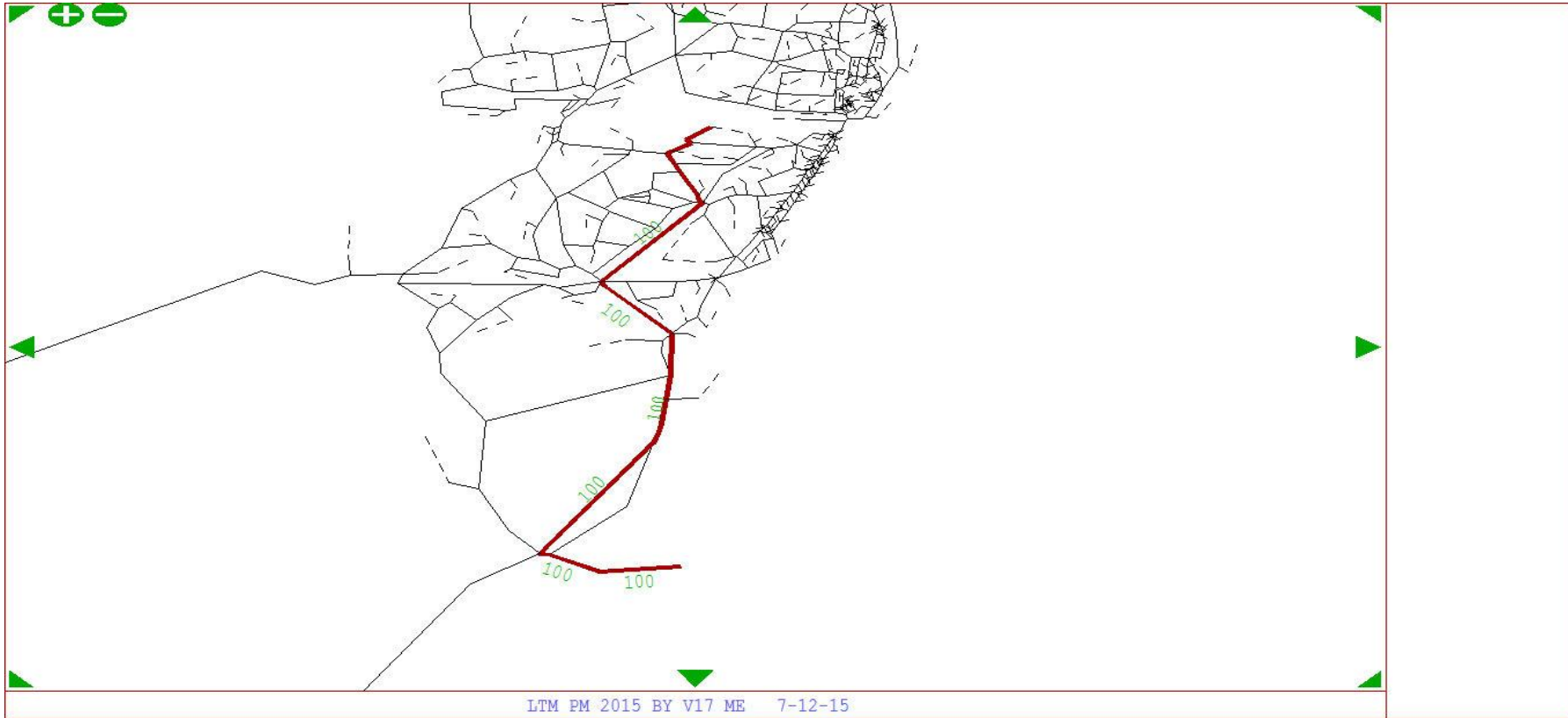


LTM PM 2015 BY V17 ME 7-12-15

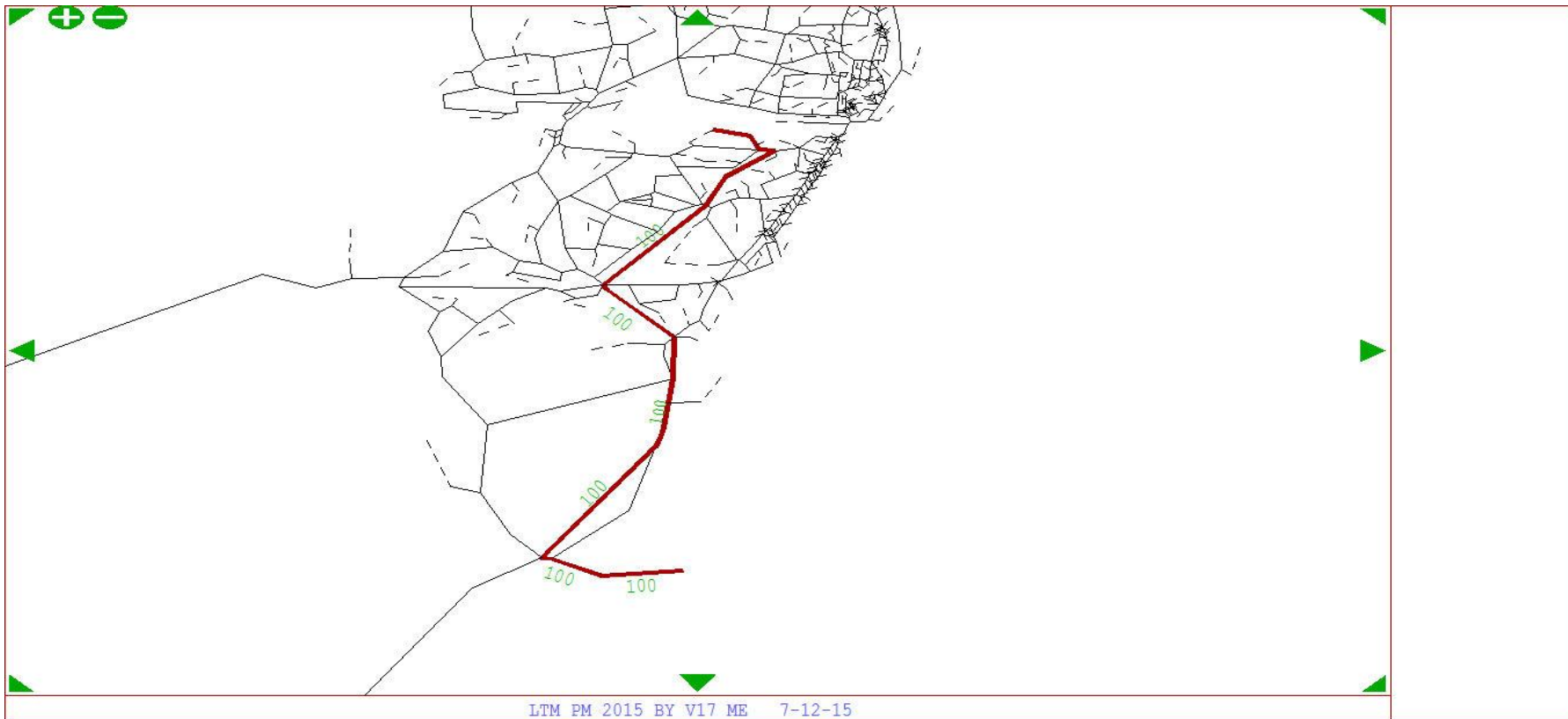
From Zone 149 To Zone 122 - User Class 3



From Zone 149 To Zone 122 - User Class 4

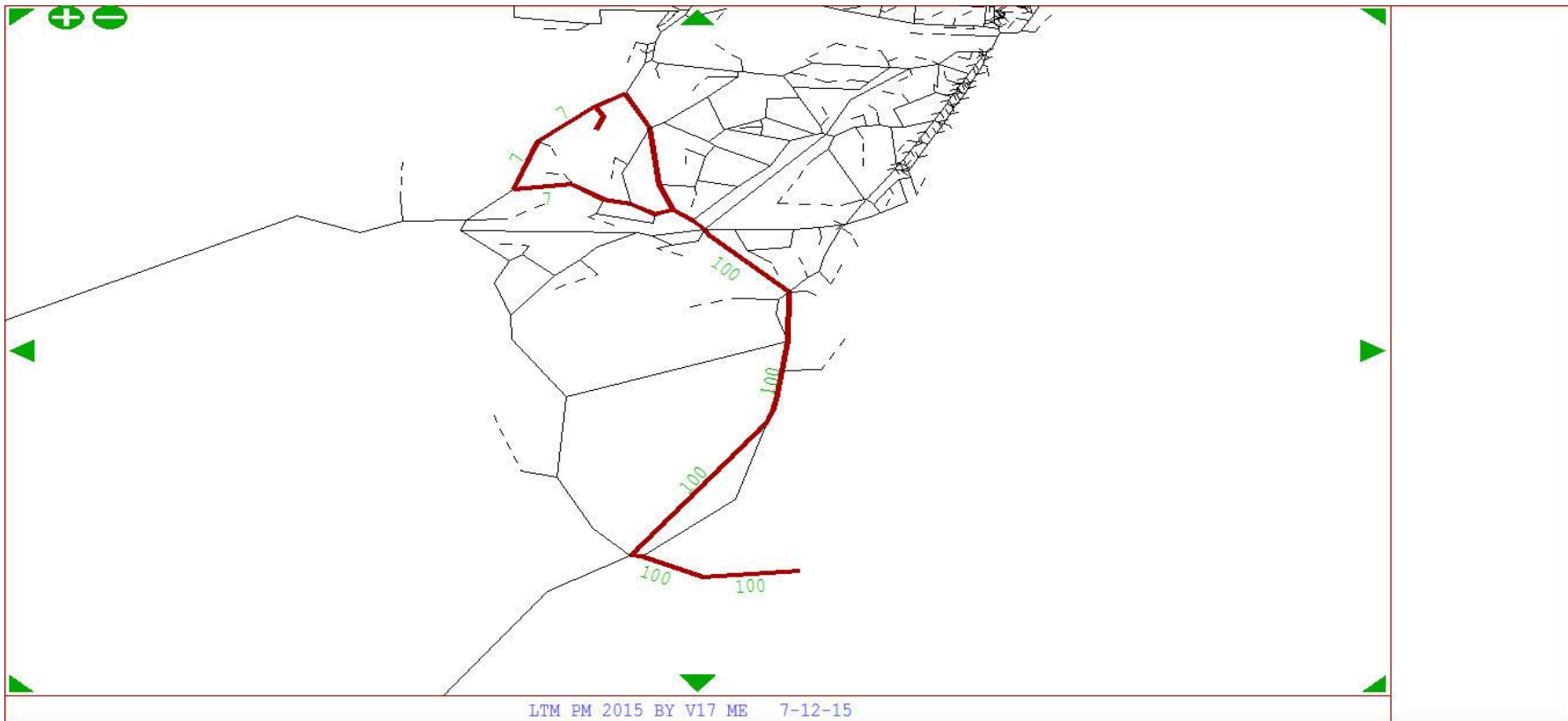


From Zone 149 To Zone 122 - User Class 5

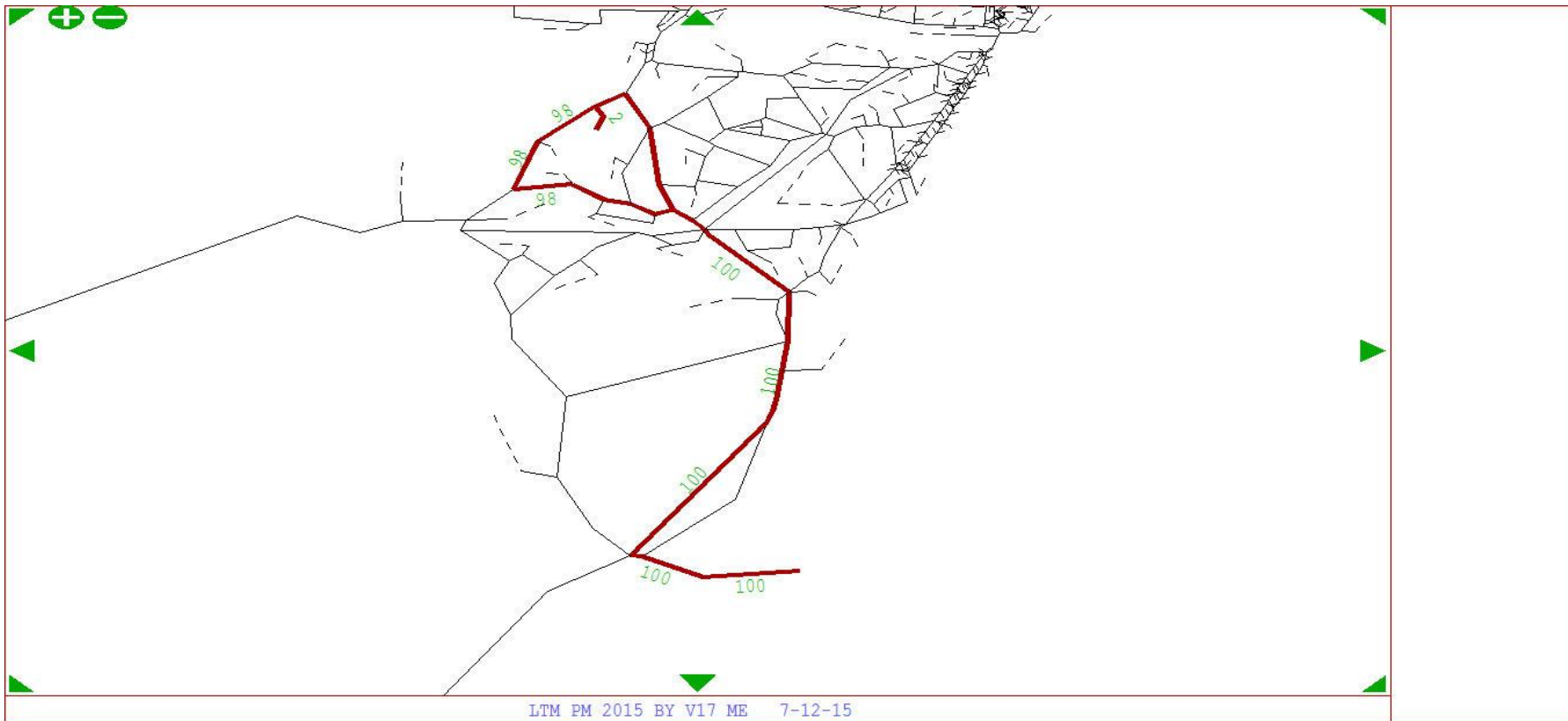


LTM PM 2015 BY V17 ME 7-12-15

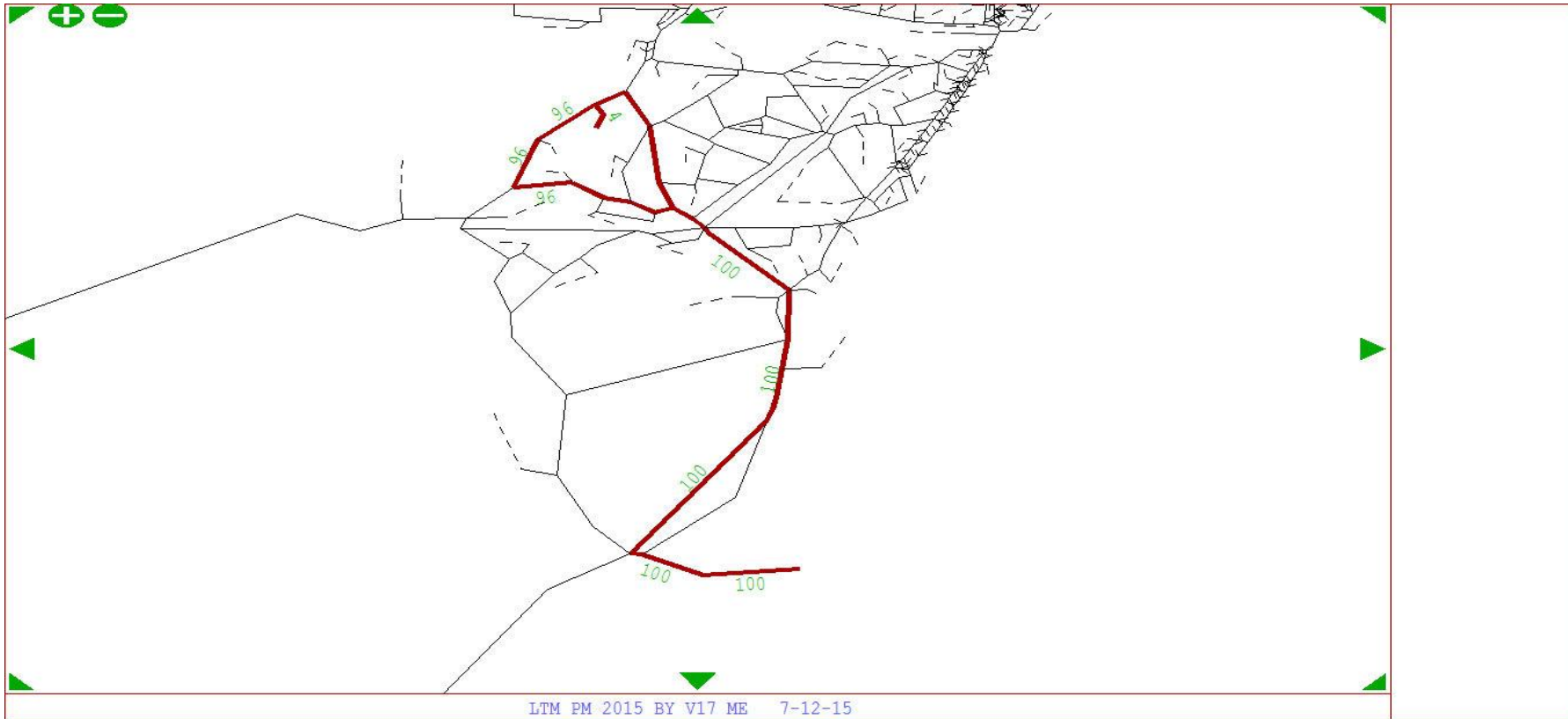
From Zone 149 To Zone 128 - User Class 1



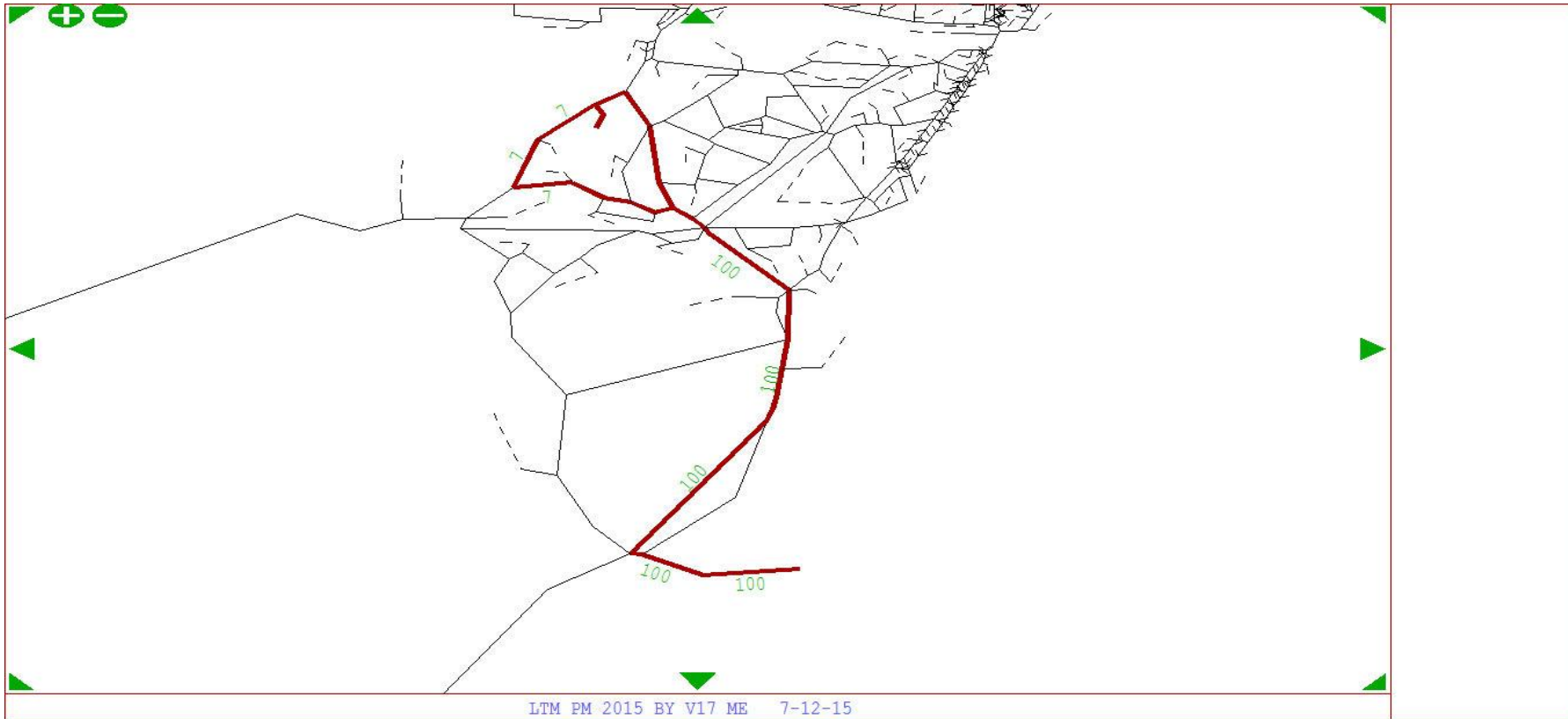
From Zone 149 To Zone 128 - User Class 2



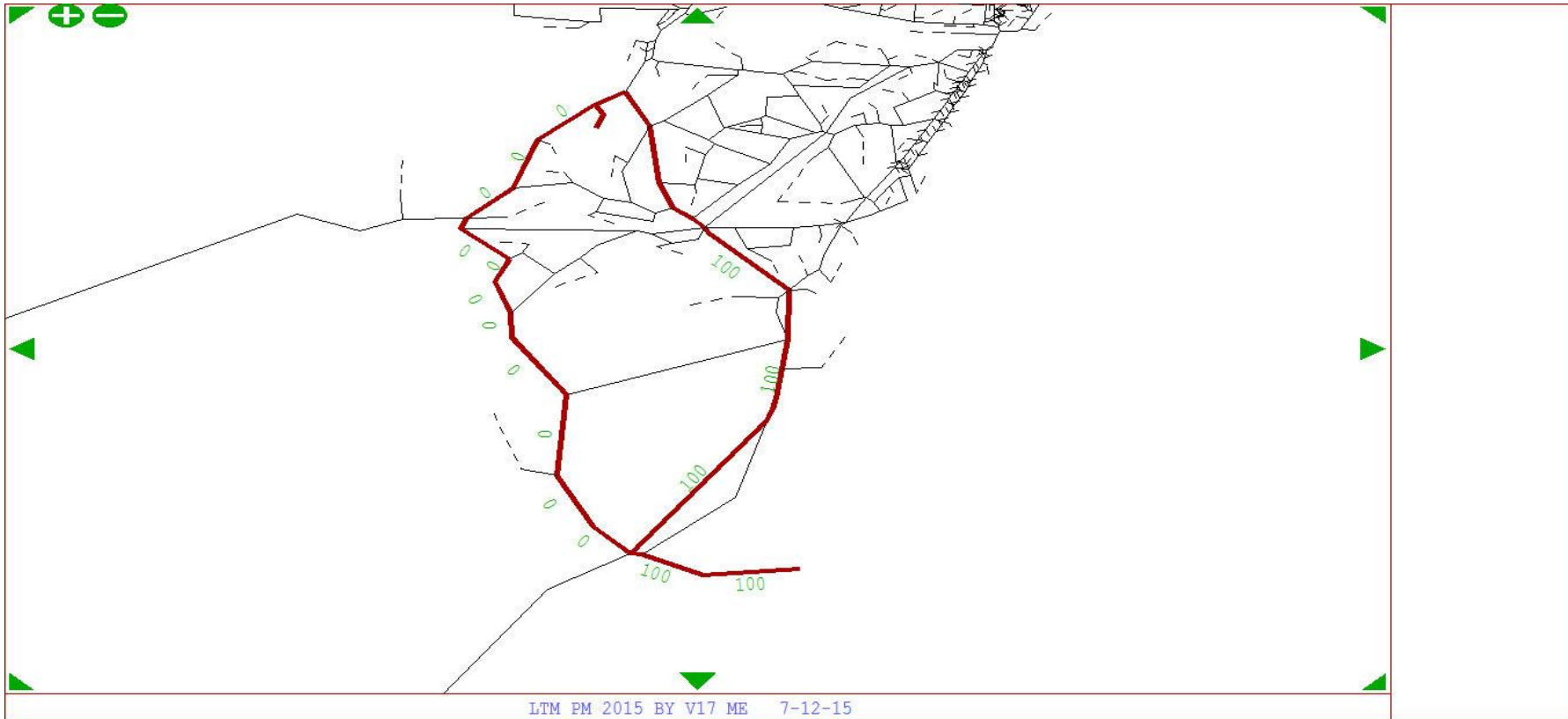
From Zone 149 To Zone 128 - User Class 3



From Zone 149 To Zone 128 - User Class 4



From Zone 149 To Zone 128 - User Class 5



Appendix D

POST ME SCREENLINE PERFORMANCE

Lowestoff Screenlines

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										Flow < 5%		Calibration		Validation		GEH < 4		Calibration		Validation									
Screenlines										All	Car	LGV	HGV	All	Car	LGV	HGV	All	Car	LGV	HGV								
Calibration										16	14	14	13	16	16	15	16	16	16	15	16	4	3	2	2	100%	100%	100%	100%
Validation										4	14	13	12	16	16	16	16	16	16	15	16	4	3	2	2	50%	50%	50%	50%

AM Peak																			
All				Car				LGV				HGV							
Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH
96	97	1%	0.089	81	82	-17%	0.111	14	14	-1%	0.033	1	1	0%	0.000	27	26	-2%	0.122
567	566	0%	0.047	456	456	-24%	0.012	85	84	-1%	0.082	13	13	-33%	2.871	8	8	-1%	0.046
461	434	-6%	1.284	384	378	-22%	0.307	64	43	-33%	2.871	13	13	-33%	2.871	8	8	-1%	0.046
179	180	0%	0.047	108	108	-66%	0.048	64	64	0%	0.031	8	8	2%	0.110	8	8	2%	0.110
1304	1277	-2%	0.746	1028	1024	0%	0.133	227	205	10%	1.472	49	48	-2%	0.132	49	48	-2%	0.132
102	101	-1%	0.062	81	80	0%	0.056	20	20	1%	0.056	1	1	-27%	0.344	1	1	-27%	0.344
567	647	14%	3.252	411	490	-19%	3.728	132	133	1%	0.120	24	24	-2%	0.076	24	24	-2%	0.076
251	269	7%	1.106	178	196	-31%	1.304	65	65	1%	0.041	8	8	-4%	0.117	8	8	-4%	0.117
113	122	8%	0.854	14	23	-665%	2.224	94	94	0%	0.000	5	5	-5%	0.110	5	5	-5%	0.110
1032	1139	10%	3.235	683	789	16%	3.906	310	312	-1%	0.111	39	38	3%	0.214	39	38	3%	0.214
584	582	0%	0.065	455	453	-29%	0.101	109	109	0%	0.000	19	20	3%	0.129	19	20	3%	0.129
376	378	1%	0.124	296	298	-26%	0.093	60	60	1%	0.052	20	20	2%	0.090	20	20	2%	0.090
959	960	0%	0.027	752	751	0%	0.020	169	169	0%	0.031	39	40	-2%	0.155	39	40	-2%	0.155
484	482	0%	0.078	377	375	-29%	0.125	91	91	0%	0.030	16	16	3%	0.108	16	16	3%	0.108
450	455	1%	0.254	375	379	-19%	0.206	59	60	2%	0.156	16	16	1%	0.050	16	16	1%	0.050
933	937	0%	0.121	752	754	0%	0.057	150	151	-1%	0.121	31	32	-2%	0.112	31	32	-2%	0.112
396	396	-2%	0.483	272	261	-49%	0.694	116	116	0%	0.000	7	9	25%	0.624	7	9	25%	0.624
687	699	2%	0.471	334	343	-103%	0.467	327	329	1%	0.110	25	27	7%	0.352	25	27	7%	0.352
1082	1085	0%	0.086	607	604	0%	0.111	443	445	0%	0.095	32	36	-11%	0.612	32	36	-11%	0.612
317	286	-10%	1.792	150	150	-111%	0.018	153	125	-18%	2.375	14	11	-21%	0.819	14	11	-21%	0.819
506	509	1%	0.115	263	265	-92%	0.148	221	221	0%	0.013	23	23	2%	0.084	23	23	2%	0.084
824	795	-3%	1.002	413	415	1%	0.107	374	346	8%	1.486	36	34	7%	0.419	36	34	7%	0.419
411	364	-11%	2.388	261	231	-69%	1.902	122	107	-12%	1.387	28	26	-8%	0.448	28	26	-8%	0.448
675	677	0%	0.077	289	290	-133%	0.042	357	357	0%	0.000	29	30	4%	0.237	29	30	4%	0.237
1086	1041	-4%	1.380	550	521	-5%	1.258	479	464	3%	0.683	57	56	2%	0.139	57	56	2%	0.139
134	123	-8%	0.970	87	87	-54%	0.054	34	34	0%	0.029	13	2	-85%	4.093	13	2	-85%	4.093
571	570	0%	0.031	337	337	-69%	0.000	205	204	0%	0.035	29	29	-1%	0.046	29	29	-1%	0.046
705	693	-2%	0.444	424	424	0%	0.024	239	238	0%	0.043	43	31	-27%	1.910	43	31	-27%	1.910
94	94	0%	0.017	67	67	-40%	0.041	23	23	-1%	0.035	4	4	0%	0.000	4	4	0%	0.000
347	344	-1%	0.138	212	211	-64%	0.038	122	120	-1%	0.142	13	13	-3%	0.122	13	13	-3%	0.122
440	438	-1%	0.114	278	278	0%	0.013	145	143	1%	0.144	17	17	3%	0.107	17	17	3%	0.107
162	159	-2%	0.263	110	110	-47%	0.032	48	45	-7%	0.464	4	4	4%	0.084	4	4	4%	0.084
151	139	-8%	0.978	68	67	-123%	0.149	72	72	0%	0.013	10	0	-100%	4.570	10	0	-100%	4.570
313	298	-5%	0.864	179	177	-1%	0.117	120	117	3%	0.301	14	4	72%	3.400	14	4	72%	3.400
215	214	0%	0.038	138	138	-55%	0.000	68	68	0%	0.027	8	8	-4%	0.117	8	8	-4%	0.117
408	413	1%	0.272	225	229	-79%	0.249	167	169	1%	0.135	15	15	0%	0.000	15	15	0%	0.000
476	377	-21%	4.789	263	263	-81%	0.007	201	103	-49%	7.919	12	11	-10%	0.359	12	11	-10%	0.359
651	595	-9%	2.248	401	401	-62%	0.017	223	167	-25%	4.040	27	27	0%	0.000	27	27	0%	0.000
1749	1599	-9%	3.668	1027	1031	0%	0.124	659	507	23%	6.313	63	61	2%	0.198	63	61	2%	0.198
304	342	13%	2.133	202	227	-38%	1.715	90	103	14%	1.300	12	12	4%	0.130	12	12	4%	0.130
190	190	0%	0.018	124	124	-53%	0.022	54	54	0%	0.034	12	12	2%	0.073	12	12	2%	0.073
246	246	0%	0.028	149	149	-65%	0.009	90	90	0%	0.023	8	7	-7%	0.206	8	7	-7%	0.206
460	460	0%	0.005	298	298	-54%	0.013	139	139	0%	0.000	23	23	1%	0.070	23	23	1%	0.070
1200	1238	3%	1.096	773	798	3%	0.893	373	386	-3%	0.654	54	54	-1%	0.064	54	54	-1%	0.064
21	5	-76%	4.360	6	3	-307%	1.308	14	1	-93%	4.663	1	1	-21%	0.250	1	1	-21%	0.250
639	639	0%	0.000	267	267	-139%	0.000	312	312	0%	0.000	60	60	0%	0.000	60	60	0%	0.000
660	644	-2%	0.611	273	270	-1%	0.166	326	313	4%	0.705	61	61	0%	0.034	61	61	0%	0.034
17	18	3%	0.127	7	6	-159%	0.467	9	9	-4%	0.110	1	3	221%	1.474	1	3	221%	1.474
670	670	0%	0.000	185	185	-262%	0.000	440	440	0%	0.000	45	45	0%	0.000	45	45	0%	0.000
687	688	0%	0.020	192	191	-1%	0.087	449	449	0%	0.016	46	48	-4%	0.302	46	48	-4%	0.302

Lowestoft Screenlines

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Screenlines	Flow < 5%		Calibration		Validation		GEH < 4		Calibration		Validation	
	All	14	88%	1	25%	All	16	100%	4	100%		
Calibration	16		Car	14	88%	2	50%	Car	16	100%	3	75%
Validation	4		LGV	13	81%	0	0%	LGV	15	94%	2	50%
			HGV	12	75%	1	25%	HGV	16	100%	4	100%

ID	Screenline	Direction	Location	Direction	AM Obs	AM Mod	AM	Interp	PM
15	Screenline 8 - NB	13	Calibration	London Road South	NEB	9606	1040		
		15	Calibration	A12 Tom Crisp Way	NEB	3000	10015		
		17	Calibration	A1117 Elm Tree Road	NB	3030	3040		
	TOTAL					Calibration		Yes	Yes
16	Screenline 8 - SB	14	Calibration	London Road South	SWB	1040	9606		
		16	Calibration	A12 Tom Crisp Way	SWB	10015	3000		
		18	Calibration	A1117 Elm Tree Road	SB	3040	3030		
	TOTAL					Calibration		Yes	Yes
19	Screenline 9 - NB	81	Validation	Saltwater Way	NB	2050	2060		
		79	Validation	A12 Pier Terrace	NB	1260	10023		
		TOTAL					Validation		Yes
20	Screenline 9 - SB	82	Validation	Saltwater Way	SB	2060	2050		
		80	Validation	A12 Pier Terrace	SB	10023	1260		
		TOTAL					Validation		Yes
21	Screenline 10 - NB	9	Validation	A1145 Castleton	EB	5110	5060		
		11	Validation	A12 London Road	NB	5390	1000		
		19	Validation	A146 Beccles Road	NEB	10111	10109		
	TOTAL					Validation		Yes	Yes
22	Screenline 10 - SB	10	Validation	A1145 Castleton	WB	5060	5110		
		12	Validation	A12 London Road	SB	1000	5390		
		20	Validation	A146 Beccles Road	SWB	10109	10111		
	TOTAL					Validation		Yes	Yes

AM Peak															
All				Car				LGV				HGV			
Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH
452	453	0%	0.035	247	247	-83%	0.008	185	188	0%	0.036	18	18	2%	0.088
823	819	0%	0.143	623	619	-33%	0.147	172	172	0%	0.000	28	28	-2%	0.084
310	309	0%	0.043	167	166	-86%	0.039	125	125	0%	0.011	18	18	-2%	0.088
1585	1581	0%	0.103	1036	1032	0%	0.133	484	485	0%	0.028	64	64	1%	0.055
356	357	0%	0.033	183	183	-95%	0.009	156	156	0%	0.040	18	18	1%	0.059
381	381	0%	0.011	247	247	-54%	0.021	111	111	0%	0.021	23	23	-1%	0.069
359	359	0%	0.013	209	209	-72%	0.000	134	134	0%	0.022	16	16	0%	0.000
1096	1097	0%	0.033	639	639	0%	0.008	400	401	0%	0.049	57	57	0%	0.011
944	973	3%	0.937	656	545	-61%	4.520	242	383	58%	7.974	46	45	-3%	0.178
1531	1395	-9%	3.556	656	839	-105%	6.688	810	500	-38%	12.103	65	56	-14%	1.173
2475	2368	-4%	2.174	1312	1384	5%	1.964	1052	883	16%	5.426	111	101	9%	1.003
904	902	0%	0.067	506	518	-76%	0.532	367	344	-6%	1.230	31	40	30%	1.539
898	768	-14%	4.504	530	473	-80%	2.555	322	256	-20%	3.869	46	39	-15%	1.077
1802	1670	-7%	3.168	1036	991	-4%	1.420	689	600	13%	3.503	77	79	-3%	0.242
441	385	-13%	2.745	260	223	-84%	2.360	155	128	-17%	2.243	26	34	29%	1.374
719	742	3%	0.856	371	471	-67%	4.874	307	247	-20%	3.829	40	24	-41%	2.895
479	391	-18%	4.225	256	162	-124%	6.510	198	204	3%	0.459	26	25	-2%	0.100
1639	1518	-7%	3.040	887	856	-3%	1.043	660	579	12%	3.239	92	83	10%	1.001
343	337	-2%	0.337	173	122	-128%	4.233	140	187	33%	3.648	29	28	-5%	0.270
595	654	10%	2.379	324	403	-59%	4.159	229	229	0%	0.028	41	22	-47%	3.450
435	302	-31%	6.928	288	94	-119%	14.024	127	190	50%	5.026	21	18	-12%	0.570
1373	1293	-6%	2.186	785	619	-21%	6.262	497	606	-22%	4.663	91	68	26%	2.618

Lowestoft Screenlines

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ID	Name	Lat In	Status	Site Location	Direction	A Mode	B Mode	AM	Interp	PM
TOTAL										
16	Screenline 8 - SB	14	Calibration	London Road South	SWB	1040	9606			
		16	Calibration	A12 Tom Crisp Way	SWB	10015	3000			
		18	Calibration	A1117 Elm Tree Road	SB	3040	3030			
TOTAL						Calibration		Yes	Yes	Yes
19	Screenline 9 - NB	81	Validation	Saltwater Way	NB	2050	2060			
		79	Validation	A12 Pier Terrace	NB	1260	10023			
		TOTAL						Validation		Yes
20	Screenline 9 - SB	82	Validation	Saltwater Way	SB	2060	2050			
		80	Validation	A12 Pier Terrace	SB	10023	1260			
		TOTAL						Validation		Yes
21	Screenline 10 - NB	9	Validation	A1145 Castleton	EB	5110	5060			
		11	Validation	A12 London Road	NB	5390	1000			
		19	Validation	A146 Beccles Road	NEB	10111	10109			
		TOTAL						Validation		Yes
22	Screenline 10 - SB	10	Validation	A1145 Castleton	WB	5060	5110			
		12	Validation	A12 London Road	SB	1000	5390			
		20	Validation	A146 Beccles Road	SWB	10109	10111			
		TOTAL						Validation		Yes

Screenlines	Flow < 5%	Calibration	Validation	GEH < 4	Calibration	Validation			
All	16	100%	1	25%	All	16	100%	3	75%
Calibration	16				Calibration	16	100%	3	75%
Validation	4				LGV	16	100%	4	100%
					HGV	16	100%	4	100%

Interpeak																
All				Car				LGV				HGV				
Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	
1257	1291	3%	0.951	772	807	5%	1.263	426	425	0%	0.057	59	59	1%	0.046	
408	411	1%	0.147	203	205	-100%	0.167	184	185	0%	0.050	21	21	0%	0.017	
492	499	2%	0.332	326	325	-51%	0.076	141	150	6%	0.706	24	24	1%	0.045	
405	395	-2%	0.481	235	235	-72%	0.028	153	143	-6%	0.792	17	17	3%	0.108	
1304	1305	0%	0.021	764	765	0%	0.021	478	478	0%	0.020	61	62	-1%	0.075	
983	957	-3%	0.845	718	526	-64%	7.686	228	391	72%	9.284	38	40	5%	0.329	
1041	955	-8%	2.722	453	570	-104%	5.177	532	346	-35%	8.883	56	39	-30%	2.459	
2024	1912	-6%	2.532	1171	1096	-6%	2.216	760	737	3%	0.835	94	79	-16%	1.601	
931	974	5%	1.410	575	577	-61%	0.079	329	357	9%	1.525	27	40	50%	2.318	
1051	1090	4%	1.182	612	687	-60%	2.932	394	352	-11%	2.178	45	51	13%	0.867	
1982	2064	4%	1.827	1187	1264	6%	2.188	723	709	2%	0.517	72	91	-27%	2.150	
306	369	20%	3.412	161	189	-73%	2.131	123	152	24%	2.514	23	28	22%	0.999	
642	615	-4%	1.096	313	355	-92%	2.322	292	237	-19%	3.355	38	23	-40%	2.775	
417	301	-28%	6.104	224	131	-127%	6.982	172	153	-11%	1.519	20	17	-16%	0.745	
1365	1285	-6%	2.209	697	675	-3%	0.856	586	542	8%	1.872	82	68	-17%	1.566	
322	297	-8%	1.430	158	102	-139%	4.941	137	168	22%	2.469	26	27	3%	0.138	
664	615	-7%	1.926	375	363	-80%	0.610	249	235	-6%	0.922	40	17	-57%	4.252	
399	316	-21%	4.391	258	116	-110%	10.393	124	179	44%	4.425	16	21	28%	1.065	
1385	1228	-11%	4.340	791	581	-27%	8.026	511	582	-14%	3.023	82	65	-21%	2.017	

Lowestoft Screenlines

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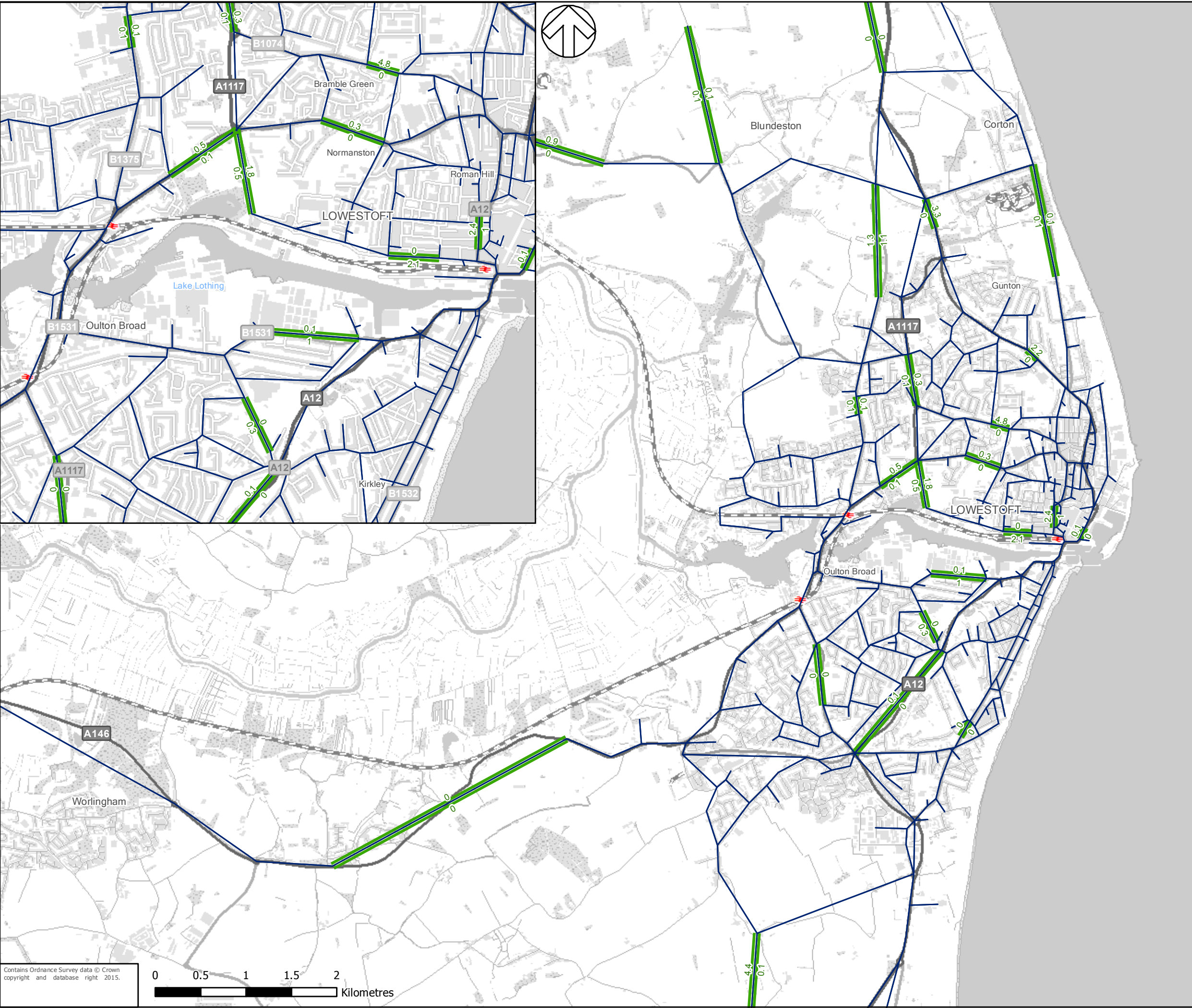
ID	Name	Lat In	Status	Site Location	Direction	A Mode	B Mode	AM	Interp	PM
TOTAL										
16	Screenline 8 - SB	14	Calibration	London Road South	SWB	1040	9606			
		16	Calibration	A12 Tom Crisp Way	SWB	10015	3000			
		18	Calibration	A1117 Elm Tree Road	SB	3040	3030			
TOTAL						Calibration		Yes	Yes	Yes
19	Screenline 9 - NB	81	Validation	Saltwater Way	NB	2050	2060			
		79	Validation	A12 Pier Terrace	NB	1260	10023			
		TOTAL						Validation		Yes
20	Screenline 9 - SB	82	Validation	Saltwater Way	SB	2060	2050			
		80	Validation	A12 Pier Terrace	SB	10023	1260			
		TOTAL						Validation		Yes
21	Screenline 10 - NB	9	Validation	A1145 Castleton	EB	5110	5060			
		11	Validation	A12 London Road	NB	5390	1000			
		19	Validation	A146 Beccles Road	NEB	10111	10109			
		TOTAL						Validation		Yes
22	Screenline 10 - SB	10	Validation	A1145 Castleton	WB	5060	5110			
		12	Validation	A12 London Road	SB	1000	5390			
		20	Validation	A146 Beccles Road	SWB	10109	10111			
		TOTAL						Validation		Yes

Screenlines		Flow < 5%		Calibration		Validation		GEH < 4		Calibration		Validation		
Calibration	16	All	14	88%	2	50%	All	16	100%	2	50%	Calibration	16	100%
Validation	4	Car	15	94%	1	25%	Car	16	100%	2	50%	LGV	15	94%
		LGV	7	44%	3	75%	LGV	15	94%	4	100%	HGV	16	100%
		HGV	7	44%	0	0%	HGV	16	100%	4	100%			

PM Peak																
All				Car				LGV				HGV				
Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed	Modelled	Difference	GEH	Observed
1443	1480	3%	0.971	923	952	3%	0.933	488	497	-2%	0.428	32	31	3%	0.171	
499	483	-3%	0.728	270	258	-8%	0.708	221	216	-2%	0.347	9	9	6%	0.169	
708	768	9%	2.227	520	540	-32%	0.891	174	211	21%	2.667	14	17	21%	0.762	
417	448	7%	1.485	256	276	-54%	1.323	152	161	6%	0.750	9	9	-1%	0.042	
1624	1699	5%	1.846	1045	1076	3%	0.940	547	588	-8%	1.732	32	35	-11%	0.585	
1114	1209	9%	2.787	852	747	-43%	3.705	228	440	93%	11.588	34	22	-35%	2.274	
1104	976	-12%	3.969	582	653	-78%	2.877	499	308	-38%	9.493	24	15	-37%	2.003	
2218	2185	-1%	0.703	1433	1400	-2%	0.885	727	748	-3%	0.779	58	37	36%	3.030	
1133	1017	-10%	3.538	749	666	-62%	3.135	370	331	-11%	2.095	13	20	50%	1.626	
1591	1583	-1%	0.201	1014	991	-5%	0.730	539	557	3%	0.772	38	35	-8%	0.493	
2724	2600	-5%	2.403	1764	1657	-6%	2.576	909	888	2%	0.705	51	55	-7%	0.502	
460	519	13%	2.690	275	279	-6%	0.240	172	229	33%	3.997	12	11	-9%	0.331	
759	617	-19%	5.403	375	351	-10%	1.282	357	256	-28%	5.784	26	10	-62%	3.771	
535	434	-19%	4.599	312	193	-110%	7.489	213	228	7%	1.044	11	13	21%	0.653	
1753	1570	-10%	4.501	962	823	-14%	4.667	742	713	4%	1.081	49	34	30%	2.311	
448	497	11%	2.278	255	196	-99%	3.921	180	283	57%	6.751	12	18	45%	1.443	
775	794	2%	0.683	460	512	-57%	2.365	294	272	-8%	1.316	21	10	-52%	2.764	
491	186	-62%	16.562	332	67	-128%	18.774	148	112	-24%	3.168	10	7	-32%	1.107	
1713	1477	-14%	5.909	1047	775	-26%	9.011	623	667	-7%	1.752	43	35	20%	1.354	

Appendix E

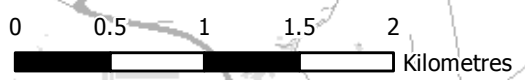
INDIVIDUAL LINK COUNT PERFORMANCE



Key

- Network
- GEH**
- 0 - 5
- 5 - 10
- > 10

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TITLE:
**LOWESTOFT
 GEH CALIBRATION
 2015 BASE YEAR
 AM PEAK**

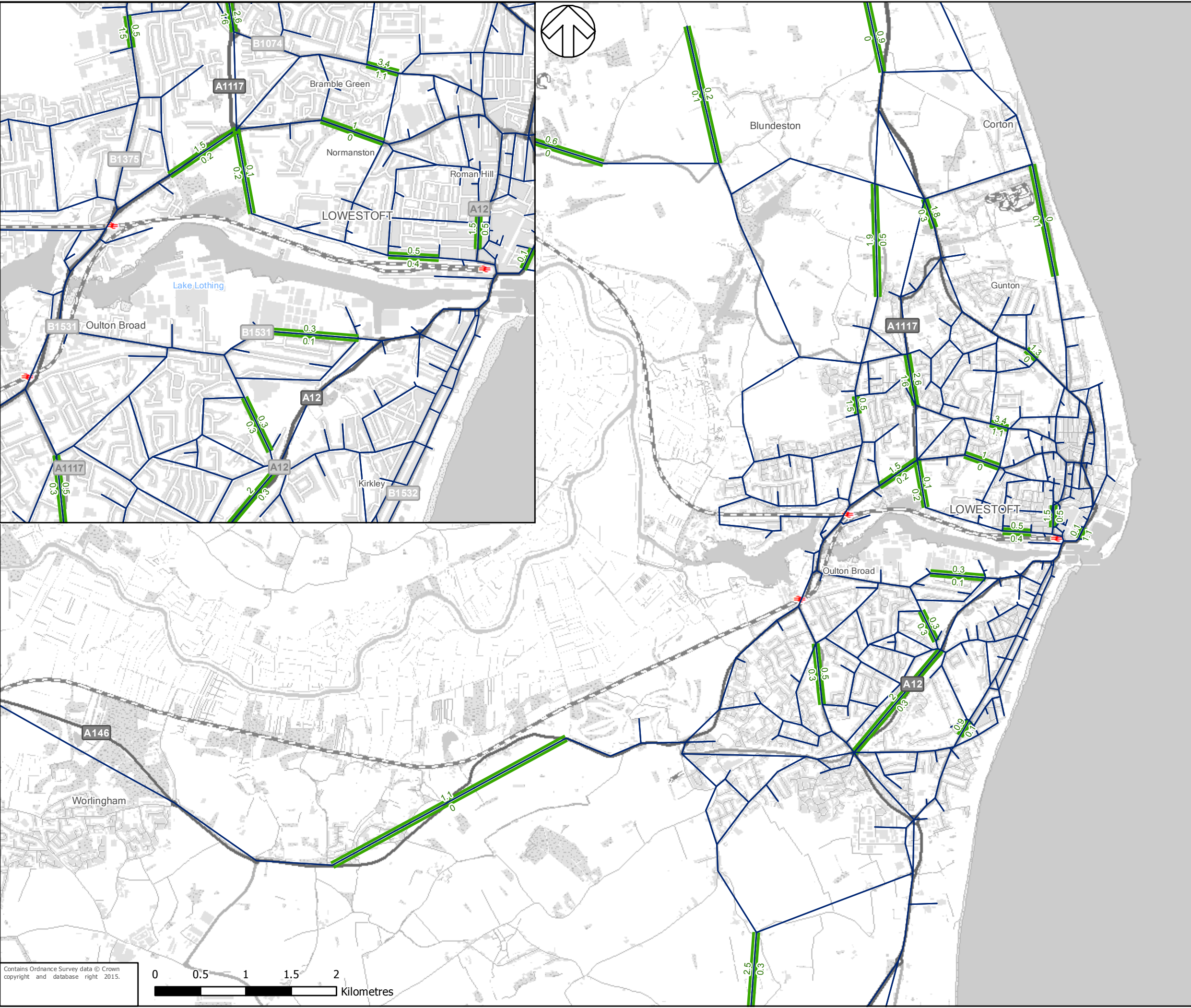
FIGURE No:

Lowestoft Link Flows

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ID	Calibration / Validation	Area	ID	Site Location	Dir	Date	Data Type	Duplicate?	Ref	A-Node	B-Node	Factor	AM Peak	Interrup	PM Peak	Check
5	Calibration		3	Gisleham Road	NB	Jul-15	ATC	No	9600-5011	9600	5010		Yes	Yes	Yes	OK
6	Calibration		3	Gisleham Road	SB	Jul-15	ATC	No	5010-9600	5010	9600		Yes	Yes	Yes	OK
7	Calibration		4	A146 Beccles Road	EB	Jul-15	ATC	No	514-451	4514	4513		Yes	Yes	Yes	OK
8	Calibration		4	A146 Beccles Road	WB	Jul-15	ATC	No	513-451	4513	4514		Yes	Yes	Yes	OK
9	Validation		5	A1145 Castleton Avenue	EB	Jul-15	ATC	No	5110-5068	5110	5068		Yes	Yes	Yes	OK
10	Validation		5	A1145 Castleton Avenue	WB	Jul-15	ATC	No	5068-5110	5068	5110		Yes	Yes	Yes	OK
11	Validation		6	A12 London Road	NB	Jul-15	ATC	No	5390-1003	5390	1000		Yes	Yes	Yes	OK
12	Validation		6	A12 London Road	SB	Jul-15	ATC	No	1000-5390	1000	5390		Yes	Yes	Yes	OK
13	Calibration		7	London Road South	NEB	Jul-15	ATC	No	9606-1044	9606	1040		Yes	Yes	Yes	OK
14	Calibration		7	London Road South	SWB	Jul-15	ATC	No	1040-9606	1040	9606		Yes	Yes	Yes	OK
15	Calibration		8	A12 Tom Crisp Way	NEB	Jul-15	ATC	No	000-1001	3000	10015		Yes	Yes	Yes	OK
16	Calibration		8	A12 Tom Crisp Way	SWB	Jul-15	ATC	No	10015-3000	10015	3000		Yes	Yes	Yes	OK
17	Calibration		9	A1117 Elm Tree Road	NB	Jul-15	ATC	No	3030-3040	3030	3040		Yes	Yes	Yes	OK
18	Calibration		9	A1117 Elm Tree Road	SB	Jul-15	ATC	No	3040-3030	3040	3030		Yes	Yes	Yes	OK
19	Validation		10	A146 Beccles Road	NEB	Jul-15	ATC	No	1111-1011	1011	10109		Yes	Yes	Yes	OK
20	Validation		10	A146 Beccles Road	SWB	Jul-15	ATC	No	10109-1111	10109	1011		Yes	Yes	Yes	OK
21	Calibration		11	Kirkley Run	NWB	Jul-15	ATC	No	10103-5271	10103	5270		Yes	Yes	Yes	OK
22	Calibration		11	Kirkley Run	SEB	Jul-15	ATC	No	5270-10103	5270	10103		Yes	Yes	Yes	OK
23	Calibration		12	A146 Waveney Drive	EB	Jul-15	ATC	No	0088-401	10088	4010		Yes	Yes	Yes	OK
24	Calibration		12	A146 Waveney Drive	WB	Jul-15	ATC	No	010-1008	4010	10088		Yes	Yes	Yes	OK
29	Calibration		15	Katwijk Way	NB	Jul-15	ATC	No	040-1013	6040	10136		Yes	Yes	Yes	OK
30	Calibration		15	Katwijk Way	SB	Jul-15	ATC	No	10136-6040	10136	6040		Yes	Yes	Yes	OK
31	Calibration		16	A12 Battery Green Road	NB	Jul-15	ATC	No	1160-6150	6160	6150		Yes	Yes	Yes	OK
32	Calibration		16	A12 Battery Green Road	SB	Jul-15	ATC	No	6150-1160	6150	6160		Yes	Yes	Yes	OK
33	Validation		17	A12 Old Nelson Street	NB	Jul-15	ATC	No	1140-6130	6140	6130		Yes	Yes	Yes	OK
34	Validation		17	A12 Old Nelson Street	SB	Jul-15	ATC	No	6130-1140	6130	6140		Yes	Yes	Yes	OK
35	Validation		18	St Peter's Street	EB	Jul-15	ATC	No	6075-6070	6070	6075		Yes	Yes	Yes	OK
36	Validation		18	St Peter's Street	WB	Jul-15	ATC	No	6075-6070	6070	6075		Yes	Yes	Yes	OK
37	Calibration		19	Denmark Road	EB	Jul-15	ATC	No	200-1013	7200	10139		Yes	Yes	Yes	OK
38	Calibration		19	Denmark Road	WB	Jul-15	ATC	No	0139-720	10139	7200		Yes	Yes	Yes	OK
39	Validation		20	Rotterdam Road	NEB	Jul-15	ATC	No	7210-9130	7210	9130		Yes	Yes	Yes	OK
40	Validation		20	Rotterdam Road	SWB	Jul-15	ATC	No	9130-7210	9130	7210		Yes	Yes	Yes	OK
41	Calibration		21	Peto Way	NB	Jul-15	ATC	No	0190-7060	10190	7060		Yes	Yes	Yes	OK
42	Calibration		21	Peto Way	SB	Jul-15	ATC	No	7060-0190	7060	10190		Yes	Yes	Yes	OK
43	Calibration		22	A1117 Normanston Drive	NEB	Jul-15	ATC	No	7050-7060	7050	7060		Yes	Yes	Yes	OK
44	Calibration		22	A1117 Normanston Drive	SWB	Jul-15	ATC	No	7060-7050	7060	7050		Yes	Yes	Yes	OK
45	Calibration		23	A1144 Normanston Drive	EB	Jul-15	ATC	No	9240-9130	9240	9130		Yes	Yes	Yes	OK
46	Calibration		23	A1144 Normanston Drive	WB	Jul-15	ATC	No	9130-9240	9130	9240		Yes	Yes	Yes	OK
47	Calibration		24	Oulton Road	EB	Jul-15	ATC	No	270-1001	9270	10010		Yes	Yes	Yes	OK
48	Calibration		24	Oulton Road	WB	Jul-15	ATC	No	10010-270	9270	10010		Yes	Yes	Yes	OK
49	Calibration		25	B1375 Gorleston Road	NB	Jul-15	ATC	No	8030-8040	8030	8040		Yes	Yes	Yes	OK
50	Calibration		25	B1375 Gorleston Road	SB	Jul-15	ATC	No	8040-8030	8040	8030		Yes	Yes	Yes	OK
51	Calibration		26	A1117 Millennium Way	NB	Jul-15	ATC	No	7070-7080	7070	7080		Yes	Yes	Yes	OK
52	Calibration		26	A1117 Millennium Way	SB	Jul-15	ATC	No	7080-7070	7080	7070		Yes	Yes	Yes	OK
53	Calibration		27	A12 Yarmouth Road	NWB	Jul-15	ATC	No	10242-10248	10242	10248		Yes	Yes	Yes	OK
54	Calibration		27	A12 Yarmouth Road	SEB	Jul-15	ATC	No	10248-10242	10248	10242		Yes	Yes	Yes	OK
55	Calibration		28	B1385 Corton Road	NB	Jul-15	ATC	No	9460-9460	9460	9460		Yes	Yes	Yes	OK
56	Calibration		28	B1385 Corton Road	SB	Jul-15	ATC	No	9460-9460	9460	9460		Yes	Yes	Yes	OK
57	Calibration		29	A12 Yarmouth Road	NB	Jul-15	ATC	No	10257-6250	10257	6250		Yes	Yes	Yes	OK
58	Calibration		29	A12 Yarmouth Road	SB	Jul-15	ATC	No	6250-10257	6250	10257		Yes	Yes	Yes	OK
59	Calibration		30	B1375 Parkhill	NB	Jul-15	ATC	No	8070-1001	8070	1001		Yes	Yes	Yes	OK
60	Calibration		30	B1375 Parkhill	SB	Jul-15	ATC	No	1001-8070	1001	8070		Yes	Yes	Yes	OK
61	Calibration		31	B1074 Bludleston Road	NB	Jul-15	ATC	No	10025-2002	10025	20026		Yes	Yes	Yes	OK
62	Calibration		31	B1074 Bludleston Road	SB	Jul-15	ATC	No	20026-10025	10025	20026		Yes	Yes	Yes	OK
63	Calibration		32	Fixton Road	NB	Jul-15	ATC	No	9440-10024	9440	10024		Yes	Yes	Yes	OK
64	Calibration		32	Fixton Road	SB	Jul-15	ATC	No	10024-9440	10024	9440		Yes	Yes	Yes	OK
65	Validation		33	Coast Road	NB	Jul-15	ATC	No	510-1002	9510	10027		Yes	Yes	Yes	OK
66	Validation		33	Coast Road	SB	Jul-15	ATC	No	10027-9510	9510	10027		Yes	Yes	Yes	OK
67	Validation	06/605		Katwijk Way	NB	Jul-13	TRADS	No	6280-6060	7280	6060		Yes	Yes	Yes	OK
75	Calibration		3	A12 Yarmouth Rd	NB	Jul-15	ANPR	No	5280-4520	6280	4520		Yes	Yes	Yes	OK
76	Calibration		3	A12 Yarmouth Rd	SB	Jul-15	ANPR	No	4520-5280	4520	6280		Yes	Yes	Yes	OK
79	Validation		5	A12 Pier Terrace	NB	Jul-15	ANPR	No	2060-1002	1260	10023		Yes	Yes	Yes	OK
80	Validation		5	A12 Pier Terrace	SB	Jul-15	ANPR	No	10023-2060	10023	1260		Yes	Yes	Yes	OK
81	Validation		6	Saltwater Way	NB	Jul-15	ANPR	No	2050-2060	2050	2060		Yes	Yes	Yes	OK
82	Validation		6	Saltwater Way	SB	Jul-15	ANPR	No	2060-2050	2060	2050		Yes	Yes	Yes	OK
99	Validation		15	North Quay Retail Park	Entry	Jul-15	ANPR	No	9220-9230	9220	9230		Yes	Yes	Yes	OK
100	Validation		15	North Quay Retail Park	Exit	Jul-15	ANPR	No	9230-9220	9230	9220		Yes	Yes	Yes	OK
101	Validation		16	Links Road Car Park	EB	Jul-15	ANPR	No	9602-9601	9602	9603		Yes	Yes	Yes	OK
102	Validation		16	Links Road Car Park	WB	Jul-15	ANPR	No	9603-9602	9603	9602		Yes	Yes	Yes	OK
103	Validation		17	Swimming Pool Road Car Park	EB	Jul-15	ANPR	No	9090-1025	9090	10251		Yes	Yes	Yes	OK
104	Validation		17	Swimming Pool Road Car Park	WB	Jul-15	ANPR	No	10251-9090	10251	9090		Yes	Yes	Yes	OK
106	Validation		18	Battery Green Road Car Park	Exit	Jul-15	ANPR	No	10127-1011	10127	10126		Yes	Yes	Yes	OK
107	Validation		19	Gordon Road Car Park Entry	Entry	Jul-15	ANPR	No	9050-6140	9050	6140		Yes	Yes	Yes	OK
109	Validation		20	Surrey Street Car Park Entry	Entry	Jul-15	ANPR	No	10130-1011	10130	10131		Yes	Yes	Yes	OK
112	Validation		21	Clapham Road Car Park Exit	Exit	Jul-15	ANPR	No	10131-1013	10131	10133		Yes	Yes	Yes	OK
113	Validation		22	Clapham Road South	Entry	Jul-15	ANPR	No	10133-1013	10133	10138		Yes	Yes	Yes	OK
114	Validation		22	Clapham Road South	Exit	Jul-15	ANPR	No	10138-1013	10138	10137		Yes	Yes	Yes	OK
115	Validation		23	St Johns Road Car Park	Entry	Jul-15	ANPR	No	10083-10083	10083	10084		Yes	Yes	Yes	OK
116	Validation		23	St Johns Road Car Park	Exit	Jul-15	ANPR	No	10084-10083	10084	10083		Yes	Yes	Yes	OK
117	Validation		24a	Kirkley Rise Car Park	Entry	Jul-15	ANPR	No	1000-1025	4000	10256		Yes	Yes	Yes	OK
118	Validation		24a	Kirkley Rise Car Park	Exit	Jul-15	ANPR	No	10256-1000	10256	4000		Yes	Yes	Yes	OK
119	Validation		24b	Kirkley Rise Car Park	Entry	Jul-15	ANPR	No	5300-1007	5300	10075		Yes	Yes	Yes	OK
120	Validation		24b	Kirkley Rise Car Park	Exit	Jul-15	ANPR	No	10075-5300	10075	5300		Yes	Yes	Yes	OK
121	Validation		25	Kirkley Cliff Road Car Park	Entry	Jul-15	ANPR	No	1120-1007	1120	10075		Yes	Yes	Yes	OK
122	Validation		25	Kirkley Cliff Road Car Park	Exit	Jul-15	ANPR	No	10075-1120	10075	1120		Yes	Yes	Yes	OK
125	Validation		27	Marine Parade	Entry	Jul-15	ANPR	No	10081-1008	10081	10082		Yes	Yes	Yes	OK
126	Validation		27	Marine Parade	Exit	Jul-15	ANPR	No	10082-10081	10082	10081		Yes	Yes	Yes	OK
127	Validation		28	Asda Car Park	Entry	Jul-15	ANPR	No	1000-1008	4000	10085		Yes	Yes	Yes	OK
128	Validation		28	Asda Car Park	Exit	Jul-15	ANPR	No	10085-4000	10085	4000		Yes	Yes	Yes	OK

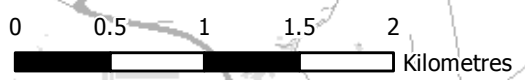
			Interpeak															
			ALL VEHICLES				CAR				LGV				HGV			
Observed	Modelled	GEH	GEH Pass?	Flow Pass?	Observed	Modelled	GEH	GEH Pass?	Flow Pass?	Observed	Modelled	GEH	Observed	Modelled	GEH			
20	10	2.521	Yes	Yes	7	6	0.536	Yes	Yes	11	3	3.118	1	1	0.021			
22	24	0.318	Yes	Yes	10	10	0.063	Yes	Yes	12	12	0.109	1	2	0.777			
589	617	1.147	Yes	Yes	227	254	1.752	Yes	Yes	316	316	0.028	47	47	0.073			
588	588	0.014	Yes	Yes	161	162	0.066	Yes	Yes	378	378	0.009	48					



Key

- Network
- GEH**
- 0 - 5
- 5 - 10
- > 10

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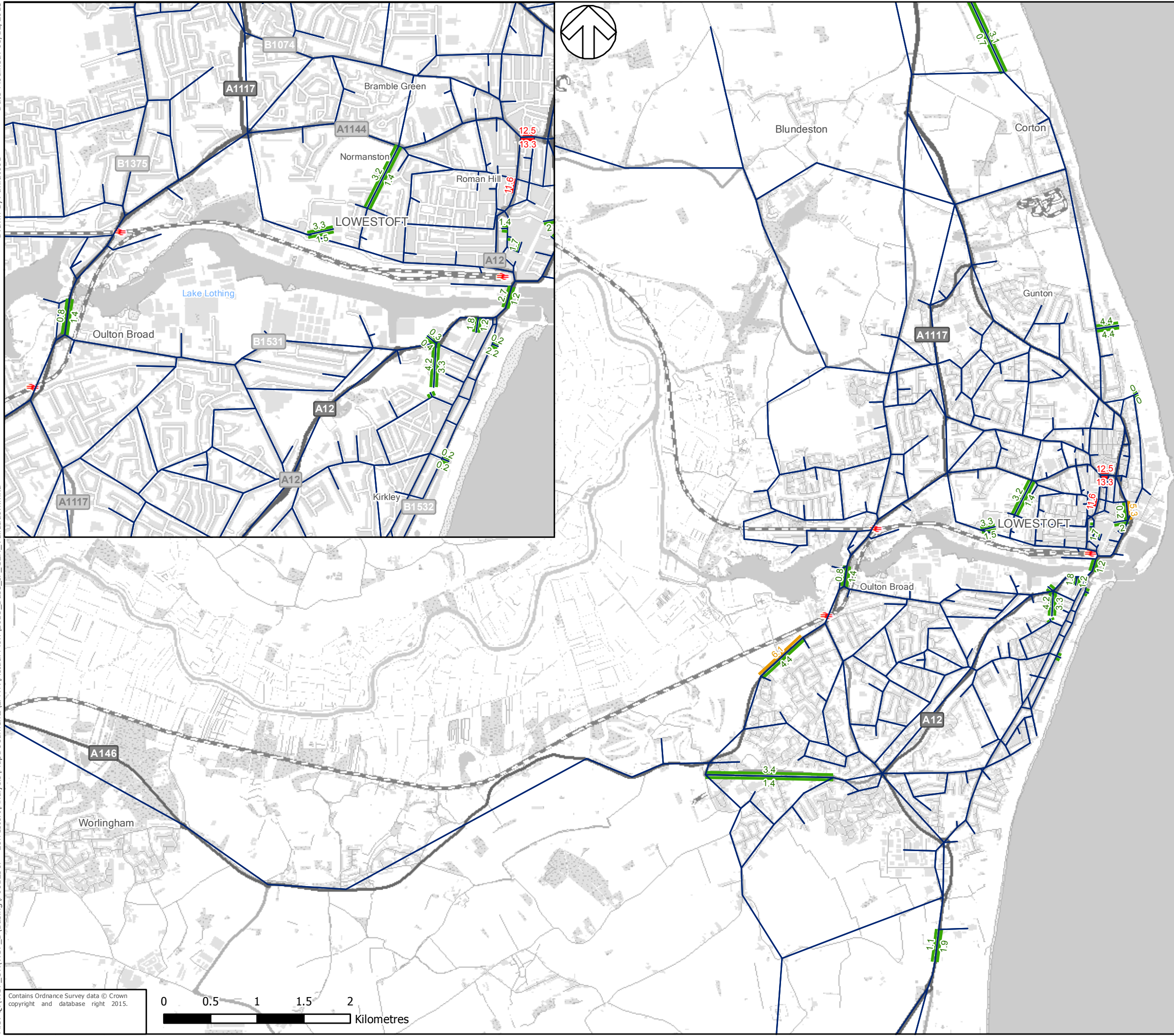
TITLE:
**LOWESTOFT
 GEH VALIDATION
 2015 BASE YEAR
 INTERPEAK**

FIGURE No:

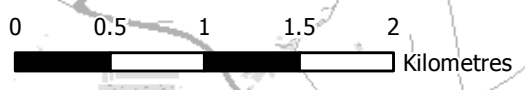


Key

- Network
- GEH
 - 0 - 5
 - 5 - 10
 - > 10



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TITLE:
**LOWESTOFT
GEH VALIDATION
2015 BASE YEAR
INTERPEAK**

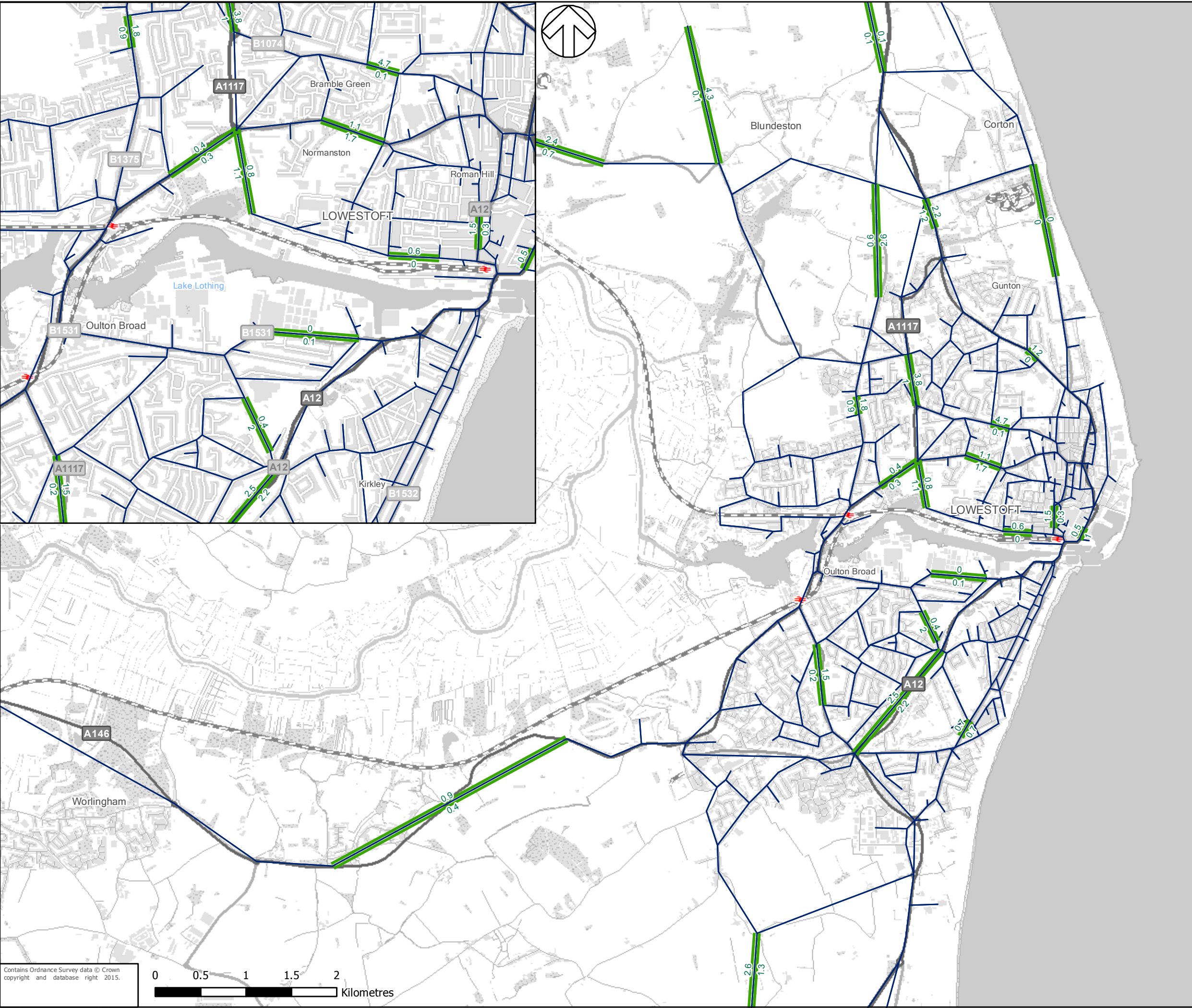
FIGURE No:

Lowestoft Link Flows

[Index](#)

ID	Calibration / Validation	Area	ID	Site Location	Dir	Date	Data Type	Duplicate?	Ref	A-Node	B-Node	Factor	AM Peak	Interp Peak	PM Peak	Check
5	Calibration		3	Gisleham Road	NB	Jul-15	ATC	No	9600-5011	9600	5010		Yes	Yes	Yes	OK
6	Calibration		3	Gisleham Road	SB	Jul-15	ATC	No	5010-9600	5010	9600		Yes	Yes	Yes	OK
7	Calibration		4	A146 Beccles Road	EB	Jul-15	ATC	No	5114-4511	4514	4513		Yes	Yes	Yes	OK
8	Calibration		4	A146 Beccles Road	WB	Jul-15	ATC	No	4513-4511	4513	4514		Yes	Yes	Yes	OK
9	Validation		5	A1145 Castleton Avenue	EB	Jul-15	ATC	No	5110-5066	5110	5066		Yes	Yes	Yes	OK
10	Validation		5	A1145 Castleton Avenue	WB	Jul-15	ATC	No	5066-5110	5066	5110		Yes	Yes	Yes	OK
11	Validation		6	A12 London Road	NB	Jul-15	ATC	No	5390-1000	5390	1000		Yes	Yes	Yes	OK
12	Validation		6	A12 London Road	SB	Jul-15	ATC	No	1000-5390	1000	5390		Yes	Yes	Yes	OK
13	Calibration		7	London Road South	NEB	Jul-15	ATC	No	9606-1040	9606	1040		Yes	Yes	Yes	OK
14	Calibration		7	London Road South	SWB	Jul-15	ATC	No	1040-9606	1040	9606		Yes	Yes	Yes	OK
15	Calibration		8	A12 Tom Crisp Way	NEB	Jul-15	ATC	No	900-1001	3000	10015		Yes	Yes	Yes	OK
16	Calibration		8	A12 Tom Crisp Way	SWB	Jul-15	ATC	No	10015-3000	10015	3000		Yes	Yes	Yes	OK
17	Calibration		9	A117 Elm Tree Road	NB	Jul-15	ATC	No	8030-3040	3030	3040		Yes	Yes	Yes	OK
18	Calibration		9	A117 Elm Tree Road	SB	Jul-15	ATC	No	8040-3030	3040	3030		Yes	Yes	Yes	OK
19	Validation		10	A146 Beccles Road	NEB	Jul-15	ATC	No	1011-1011	10110	10109		Yes	Yes	Yes	OK
20	Validation		10	A146 Beccles Road	SWB	Jul-15	ATC	No	10109-1011	10109	10111		Yes	Yes	Yes	OK
21	Calibration		11	Kirkley Run	NWB	Jul-15	ATC	No	0103-527	10103	5270		Yes	Yes	Yes	OK
22	Calibration		11	Kirkley Run	SEB	Jul-15	ATC	No	270-1010	5270	10103		Yes	Yes	Yes	OK
23	Calibration		12	A146 Waveney Drive	EB	Jul-15	ATC	No	0088-401	10088	4010		Yes	Yes	Yes	OK
24	Calibration		12	A146 Waveney Drive	WB	Jul-15	ATC	No	010-1008	4010	10088		Yes	Yes	Yes	OK
29	Calibration		15	Katwijk Way	NB	Jul-15	ATC	No	040-1013	6040	10136		Yes	Yes	Yes	OK
30	Calibration		15	Katwijk Way	SB	Jul-15	ATC	No	0136-6040	10136	6040		Yes	Yes	Yes	OK
31	Calibration		16	A12 Battery Green Road	NB	Jul-15	ATC	No	160-6150	6160	6150		Yes	Yes	Yes	OK
32	Calibration		16	A12 Battery Green Road	SB	Jul-15	ATC	No	160-6150	6150	6160		Yes	Yes	Yes	OK
33	Validation		17	A12 Old Nelson Street	NB	Jul-15	ATC	No	1140-6130	6140	6130		Yes	Yes	Yes	OK
34	Validation		17	A12 Old Nelson Street	SB	Jul-15	ATC	No	1130-6140	6130	6140		Yes	Yes	Yes	OK
35	Validation		18	St Peter's Street	EB	Jul-15	ATC	No	6070-6070	6070	6075		Yes	Yes	Yes	OK
36	Validation		18	St Peter's Street	WB	Jul-15	ATC	No	6075-6070	6070	6075		Yes	Yes	Yes	OK
37	Calibration		19	Denmark Road	EB	Jul-15	ATC	No	200-1013	7200	10139		Yes	Yes	Yes	OK
38	Calibration		19	Denmark Road	WB	Jul-15	ATC	No	0139-720	10139	7200		Yes	Yes	Yes	OK
39	Validation		20	Rotterdam Road	NEB	Jul-15	ATC	No	210-9130	9130	9130		Yes	Yes	Yes	OK
40	Validation		20	Rotterdam Road	SWB	Jul-15	ATC	No	1130-9130	9130	9130		Yes	Yes	Yes	OK
41	Calibration		21	Peto Way	NB	Jul-15	ATC	No	0190-7060	7060	7060		Yes	Yes	Yes	OK
42	Calibration		21	Peto Way	SB	Jul-15	ATC	No	060-1019	7060	10190		Yes	Yes	Yes	OK
43	Calibration		22	A1117 Normanston Drive	NEB	Jul-15	ATC	No	7050-7060	7050	7060		Yes	Yes	Yes	OK
44	Calibration		22	A1117 Normanston Drive	SWB	Jul-15	ATC	No	7060-7050	7060	7050		Yes	Yes	Yes	OK
45	Calibration		23	A1144 Normanston Drive	EB	Jul-15	ATC	No	9240-9130	9240	9130		Yes	Yes	Yes	OK
46	Calibration		23	A1144 Normanston Drive	WB	Jul-15	ATC	No	1130-9240	9130	9240		Yes	Yes	Yes	OK
47	Calibration		24	Oulton Road	EB	Jul-15	ATC	No	270-1001	9270	10010		Yes	Yes	Yes	OK
48	Calibration		24	Oulton Road	WB	Jul-15	ATC	No	0010-927	10010	9270		Yes	Yes	Yes	OK
49	Calibration		25	B1375 Gorleston Road	NB	Jul-15	ATC	No	8030-8040	8030	8040		Yes	Yes	Yes	OK
50	Calibration		25	B1375 Gorleston Road	SB	Jul-15	ATC	No	8040-8030	8040	8030		Yes	Yes	Yes	OK
51	Calibration		26	A1117 Millennium Way	NB	Jul-15	ATC	No	070-7080	7070	7080		Yes	Yes	Yes	OK
52	Calibration		26	A1117 Millennium Way	SB	Jul-15	ATC	No	080-7070	7080	7070		Yes	Yes	Yes	OK
53	Calibration		27	A12 Yarmouth Road	NWB	Jul-15	ATC	No	1024-1024	10242	10248		Yes	Yes	Yes	OK
54	Calibration		27	A12 Yarmouth Road	SEB	Jul-15	ATC	No	1024-1024	10248	10242		Yes	Yes	Yes	OK
55	Calibration		28	B1385 Corton Road	NB	Jul-15	ATC	No	9480-9480	9480	9480		Yes	Yes	Yes	OK
56	Calibration		28	B1385 Corton Road	SB	Jul-15	ATC	No	9480-9480	9480	9480		Yes	Yes	Yes	OK
57	Calibration		28	A12 Yarmouth Road	NB	Jul-15	ATC	No	1025-6250	6250	6250		Yes	Yes	Yes	OK
58	Calibration		29	A12 Yarmouth Road	SB	Jul-15	ATC	No	6250-1025	6250	10257		Yes	Yes	Yes	OK
59	Calibration		30	B1375 Parkhill	NB	Jul-15	ATC	No	0001-8070	10001	8070		Yes	Yes	Yes	OK
60	Calibration		30	B1375 Parkhill	SB	Jul-15	ATC	No	070-1000	8070	10001		Yes	Yes	Yes	OK
61	Calibration		31	B1074 Bludleston Road	NB	Jul-15	ATC	No	10025-2000	20025	20026		Yes	Yes	Yes	OK
62	Calibration		31	B1074 Bludleston Road	SB	Jul-15	ATC	No	10026-1000	20026	10025		Yes	Yes	Yes	OK
63	Calibration		32	Fixton Road	NB	Jul-15	ATC	No	440-1002	9440	10024		Yes	Yes	Yes	OK
64	Calibration		32	Fixton Road	SB	Jul-15	ATC	No	0024-944	10024	9440		Yes	Yes	Yes	OK
65	Validation		33	Coast Road	NB	Jul-15	ATC	No	510-1002	9510	10027		Yes	Yes	Yes	OK
66	Validation		33	Coast Road	SB	Jul-15	ATC	No	10027-951	10027	9510		Yes	Yes	Yes	OK
67	Validation	06/06/15	33	Katwijk Way	NB	Jul-13	TRADS	No	7280-6060	7280	6060		Yes	Yes	Yes	OK
75	Calibration		3	A12 Yarmouth Rd	NB	Jul-15	ANPR	No	5280-4520	6280	4520		Yes	Yes	Yes	OK
76	Calibration		3	A12 Yarmouth Rd	SB	Jul-15	ANPR	No	4520-6280	4520	6280		Yes	Yes	Yes	OK
79	Validation		5	A12 Pier Terrace	NB	Jul-15	ANPR	No	260-1002	1260	10023		Yes	Yes	Yes	OK
80	Validation		5	A12 Pier Terrace	SB	Jul-15	ANPR	No	0023-1260	10023	1260		Yes	Yes	Yes	OK
81	Validation		6	Saltwater Way	NB	Jul-15	ANPR	No	2050-2060	2050	2060		Yes	Yes	Yes	OK
82	Validation		6	Saltwater Way	SB	Jul-15	ANPR	No	2060-2050	2060	2050		Yes	Yes	Yes	OK
99	Validation		15	North Quay Retail Park	Entry	Jul-15	ANPR	No	1220-9230	9230	9230		Yes	Yes	Yes	OK
100	Validation		15	North Quay Retail Park	Exit	Jul-15	ANPR	No	9230-1220	9230	9220		Yes	Yes	Yes	OK
101	Validation		16	Links Road Car Park	EB	Jul-15	ANPR	No	9602-9600	9602	9603		Yes	Yes	Yes	OK
102	Validation		16	Links Road Car Park	WB	Jul-15	ANPR	No	9603-9600	9603	9602		Yes	Yes	Yes	OK
103	Validation		17	Swimming Pool Road Car Park	EB	Jul-15	ANPR	No	090-1025	9090	10251		Yes	Yes	Yes	OK
104	Validation		17	Swimming Pool Road Car Park	WB	Jul-15	ANPR	No	10251-090	10251	9090		Yes	Yes	Yes	OK
106	Validation		18	Battery Green Road Car Park	Exit	Jul-15	ANPR	No	1127-1011	10127	10126		Yes	Yes	Yes	OK
107	Validation		19	Gordon Road Car Park Entry	Entry	Jul-15	ANPR	No	1140-9050	6140	9050		Yes	Yes	Yes	OK
109	Validation		20	Surrey Street Car Park Entry	Entry	Jul-15	ANPR	No	1130-1011	10130	10131		Yes	Yes	Yes	OK
112	Validation		21	Clapham Road Car Park Exit	Exit	Jul-15	ANPR	No	1132-1011	10132	10133		Yes	Yes	Yes	OK
113	Validation		22	Clapham Road South	Entry	Jul-15	ANPR	No	1137-1011	10137	10138		Yes	Yes	Yes	OK
114	Validation		22	Clapham Road South	Exit	Jul-15	ANPR	No	1138-1011	10138	10137		Yes	Yes	Yes	OK
115	Validation		23	St Johns Road Car Park	Entry	Jul-15	ANPR	No	1083-1000	10083	10084		Yes	Yes	Yes	OK
116	Validation		23	St Johns Road Car Park	Exit	Jul-15	ANPR	No	1084-1000	10084	10083		Yes	Yes	Yes	OK
117	Validation		24a	Kirkley Rise Car Park	Entry	Jul-15	ANPR	No	000-1025	4000	10256		Yes	Yes	Yes	OK
118	Validation		24a	Kirkley Rise Car Park	Exit	Jul-15	ANPR	No	0256-4000	10256	4000		Yes	Yes	Yes	OK
119	Validation		24b	Kirkley Rise Car Park	Entry	Jul-15	ANPR	No	300-1007	5300	10078		Yes	Yes	Yes	OK
120	Validation		24b	Kirkley Rise Car Park	Exit	Jul-15	ANPR	No	10078-300	10078	5300		Yes	Yes	Yes	OK
121	Validation		25	Kirkley Cliff Road Car Park	Entry	Jul-15	ANPR	No	1120-1007	1120	10075		Yes	Yes	Yes	OK
122	Validation		25	Kirkley Cliff Road Car Park	Exit	Jul-15	ANPR	No	0075-1120	10075	1120		Yes	Yes	Yes	OK
125	Validation		27	Marine Parade	Entry	Jul-15	ANPR	No	1081-1000	10081	10082		Yes	Yes	Yes	OK
126	Validation		27	Marine Parade	Exit	Jul-15	ANPR	No	1082-1000	10082	10081		Yes	Yes	Yes	OK
127	Validation		28	Asda Car Park	Entry	Jul-15	ANPR	No	000-1008	4000	10085		Yes	Yes	Yes	OK
128	Validation		28	Asda Car Park	Exit	Jul-15	ANPR	No	10085-4000	10085	4000		Yes	Yes	Yes	OK

ALL VEHICLES						PM Peak						LGV		HGV			
Observed	Modelled	GEH	GEH Pass?	Flow Pass?	Flow Pass?	Observed	Modelled	GEH	GEH Pass?	Flow Pass?	Flow Pass?	Observed	Modelled	GEH	Observed	Modelled	GEH
42	27	2.594	Yes	Yes	Yes	16	12	1.019	Yes	Yes	Yes	26	15	2.417	1	0	1.033
26	20	1.342	Yes	Yes	Yes	12	12	1.015	Yes	Yes	Yes	13	7	1.933	1	1	0.968
896	924	0.928	Yes	Yes	Yes	383	405	1.134	Yes	Yes	Yes	489	494	0.248	25	25	0.000
664	653	0.429	Yes	Yes	Yes	193	182	0.803	Yes	Yes	Yes	448	448	0.024	24	23	0.

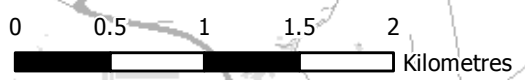


Key

GEH

- 0 - 5
- 5 - 10
- > 10
- Network

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TITLE:

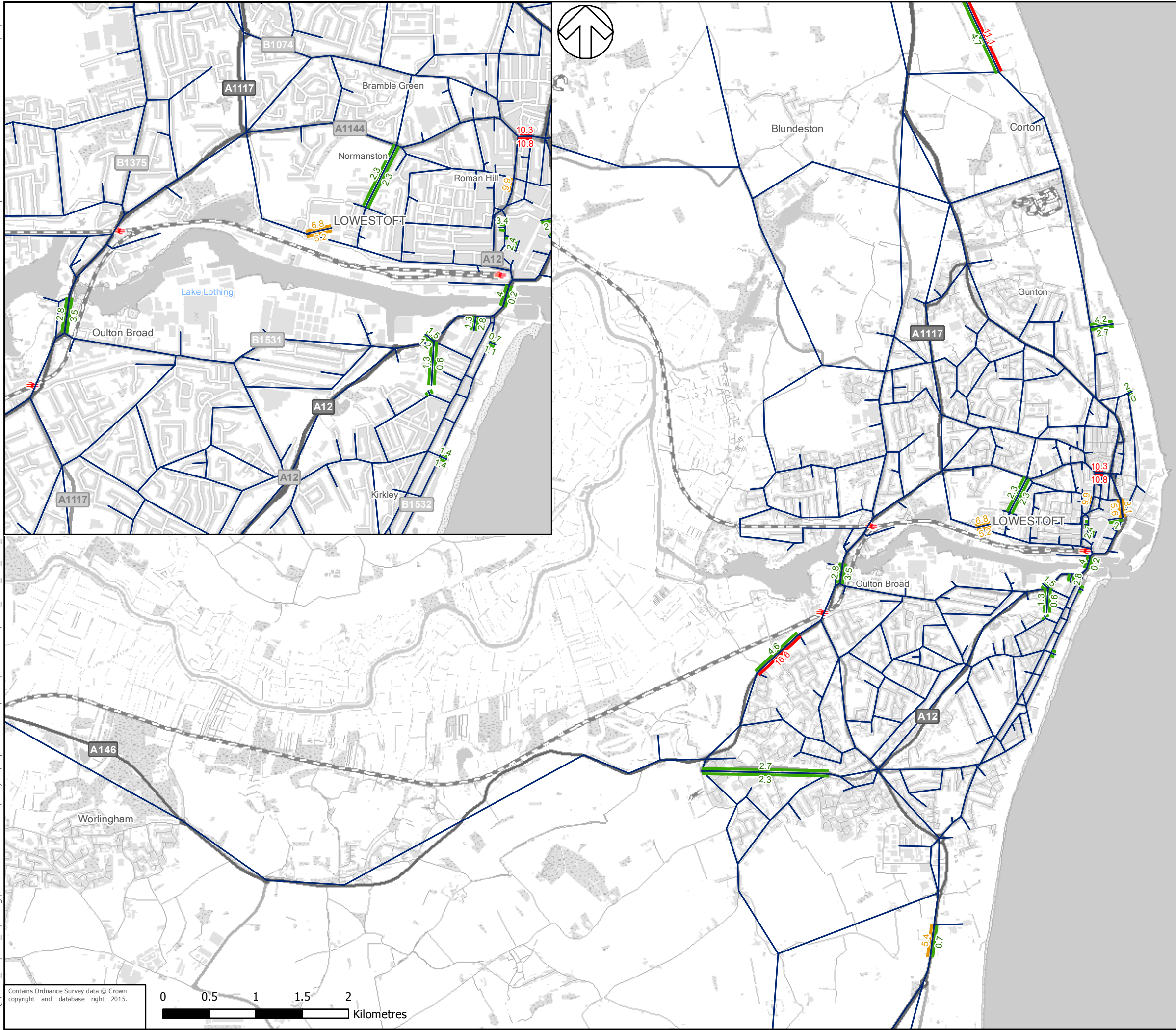
**LOWESTOFT
GEH CALIBRATION
2015 BASE YEAR
PM PEAK HOUR**

FIGURE No:

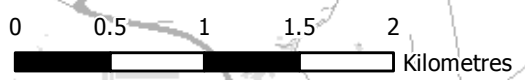


Key

- Network
- GEH
 - 0 - 5
 - 5 - 10
 - > 10



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TITLE:
**LOWESTOFT
 GEH VALIDATION
 2015 BASE YEAR
 PM PEAK**

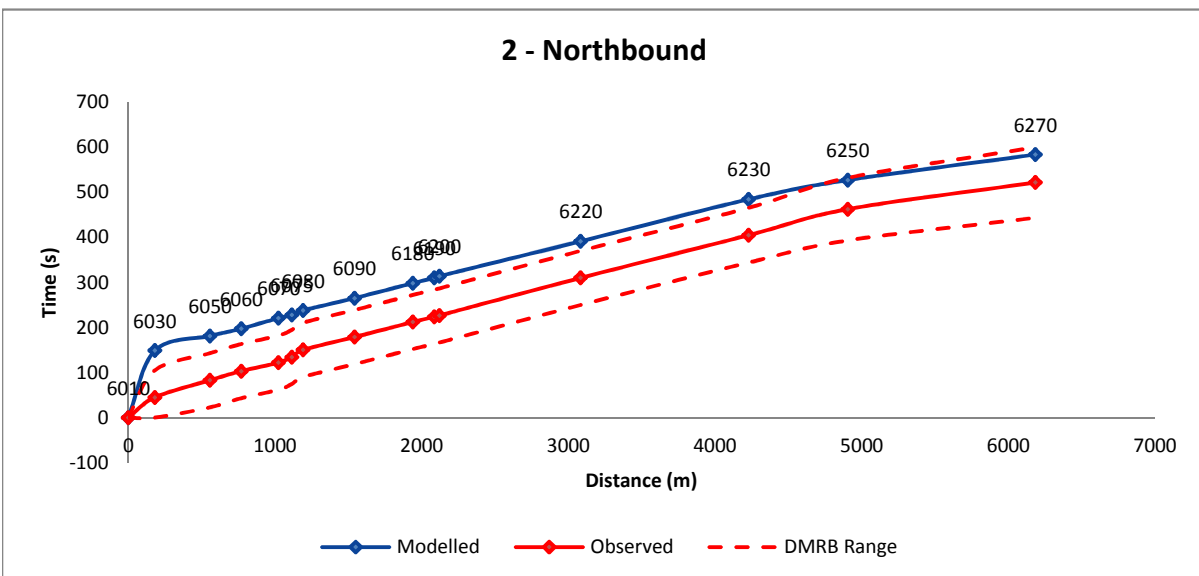
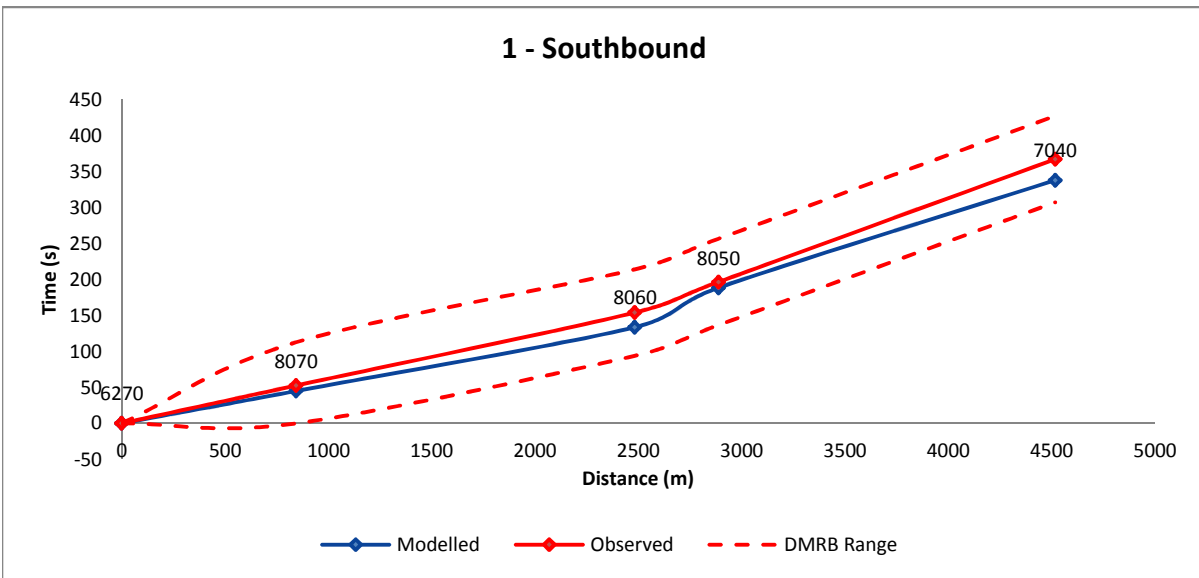
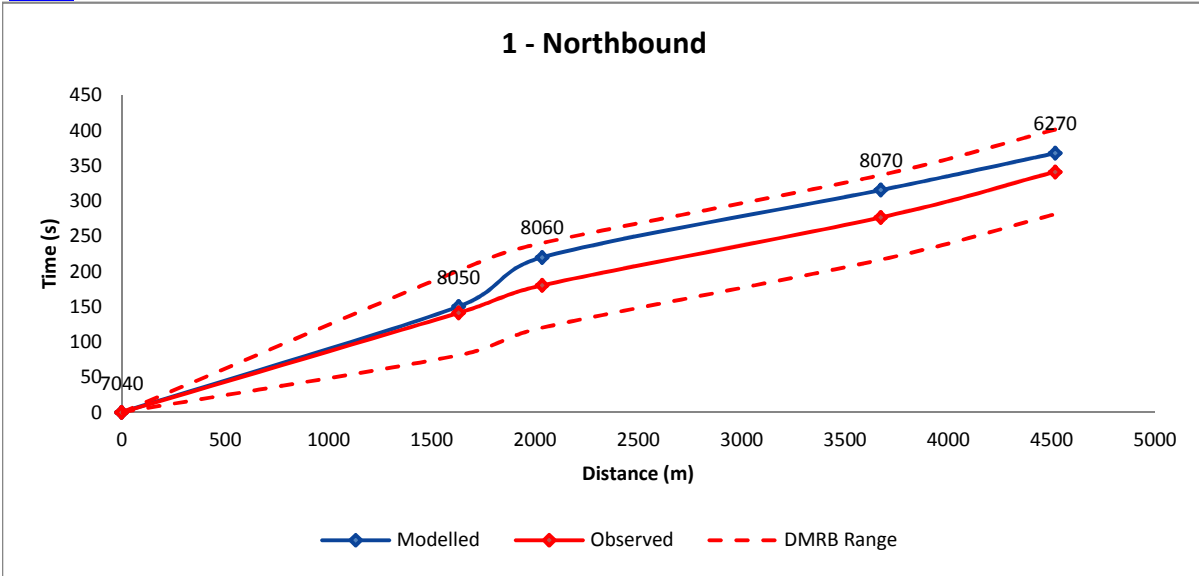
FIGURE No:

Appendix F

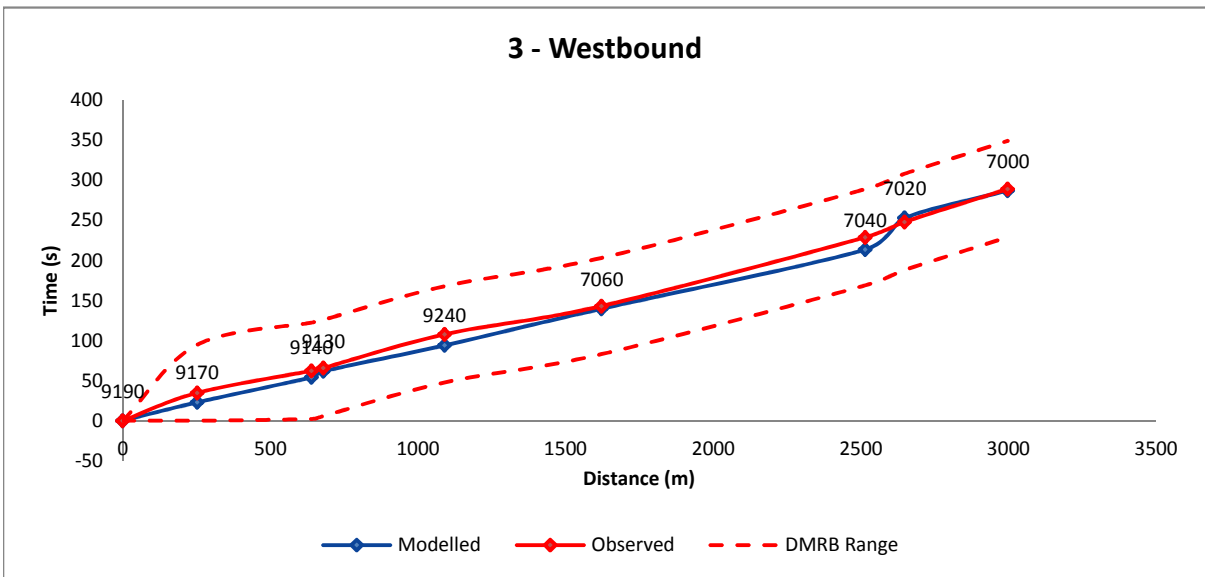
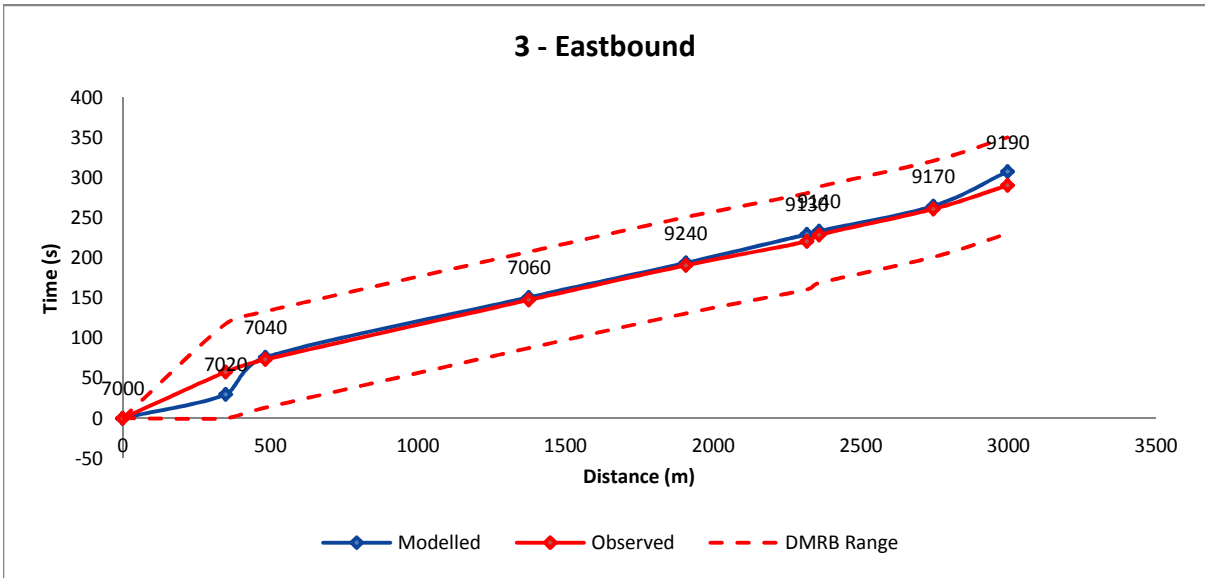
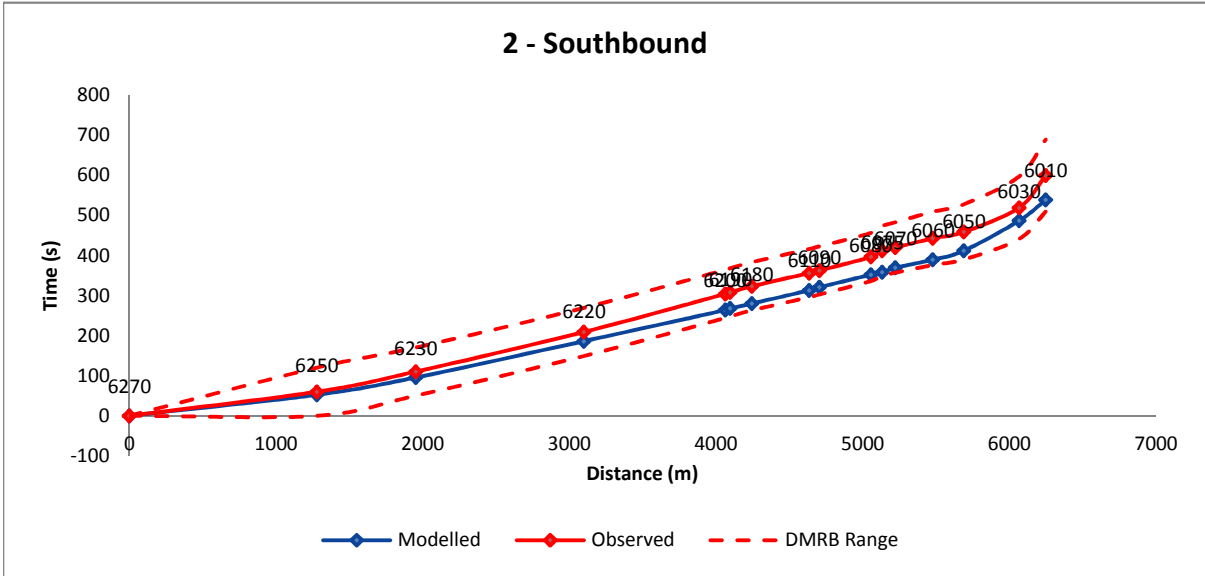
JOURNEY TIME GRAPHS

Lowestoft Journey Time Graphs - AM Peak

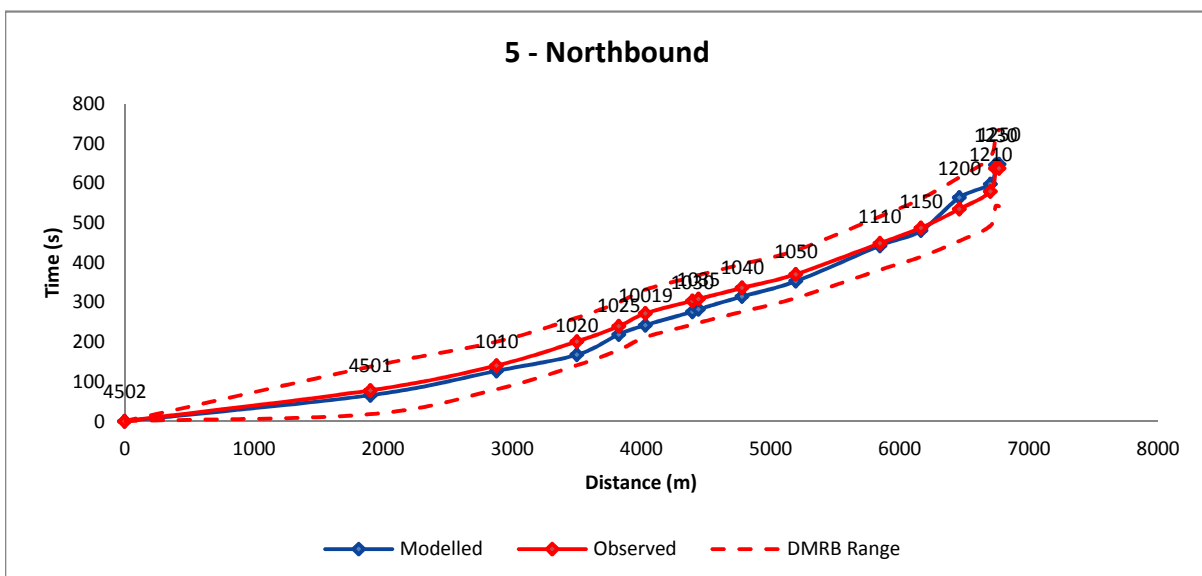
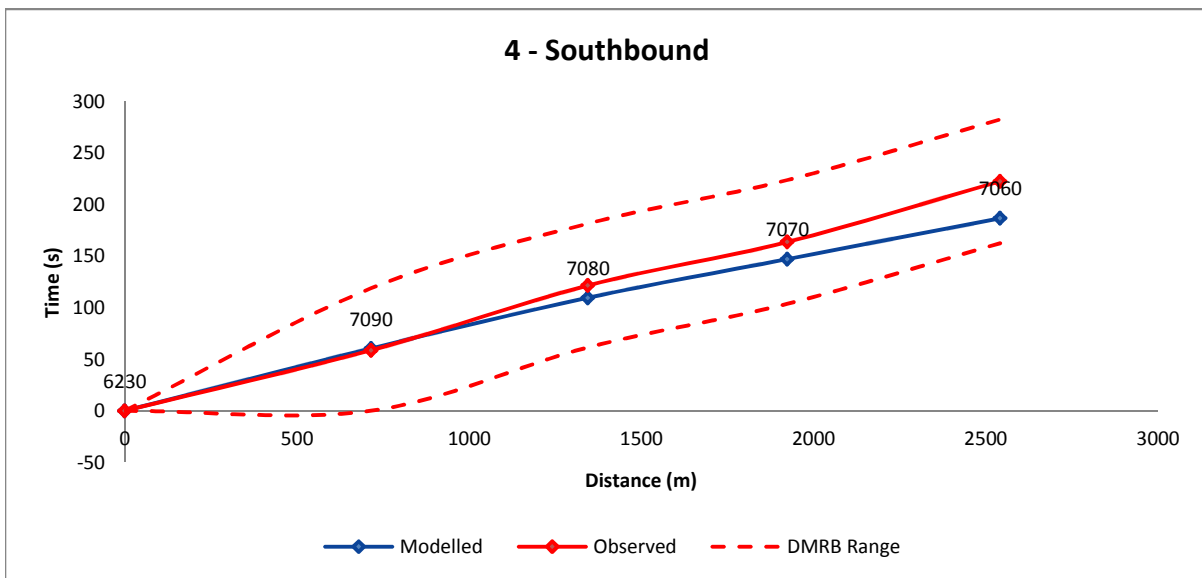
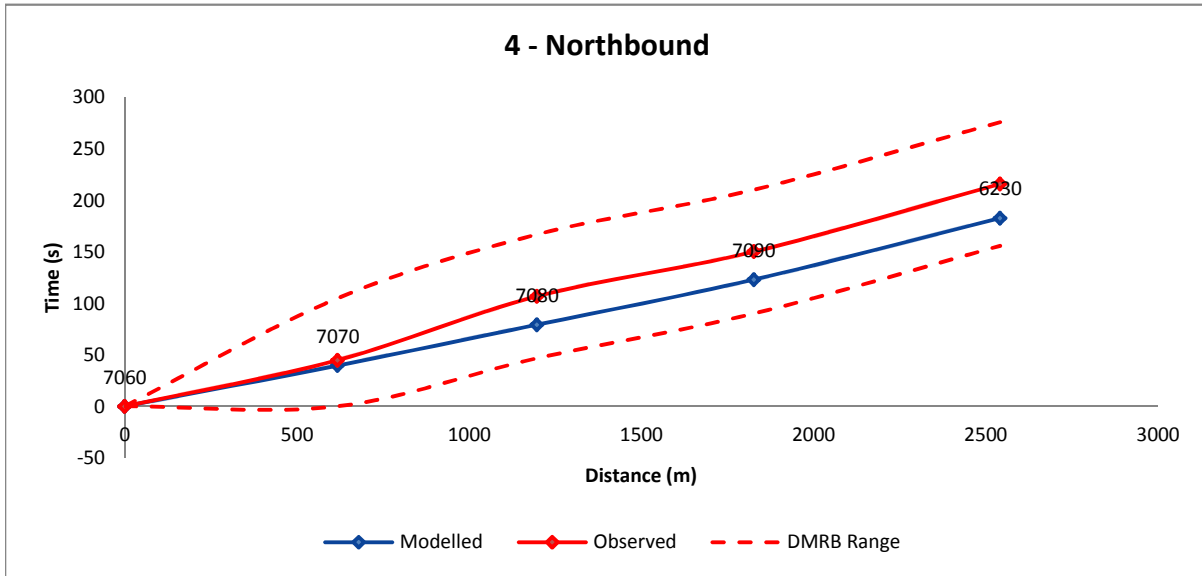
[Index](#)



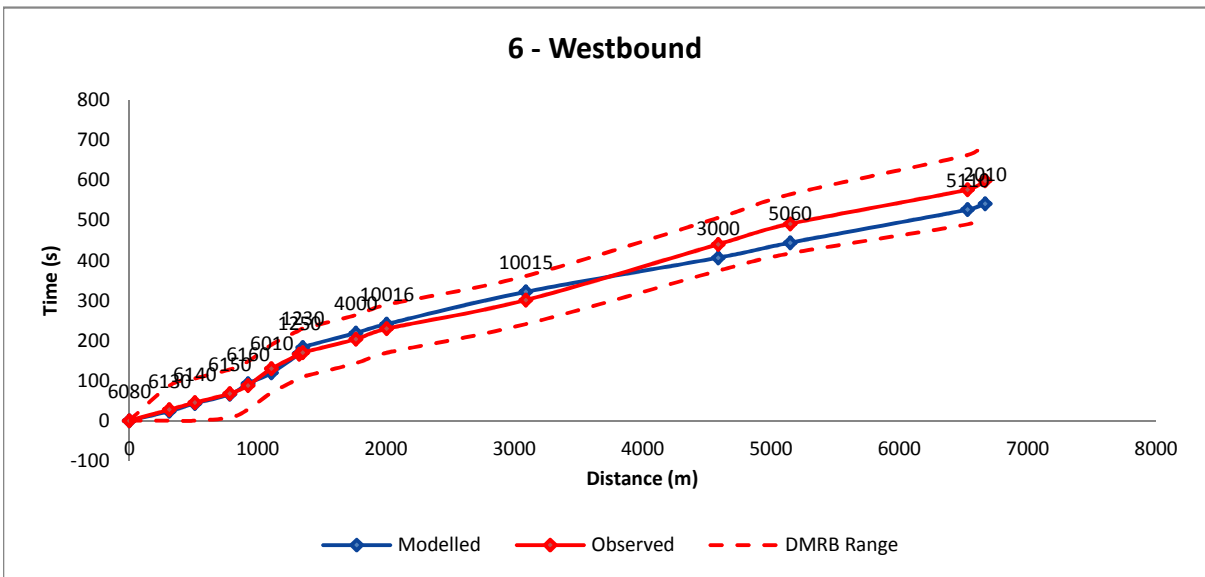
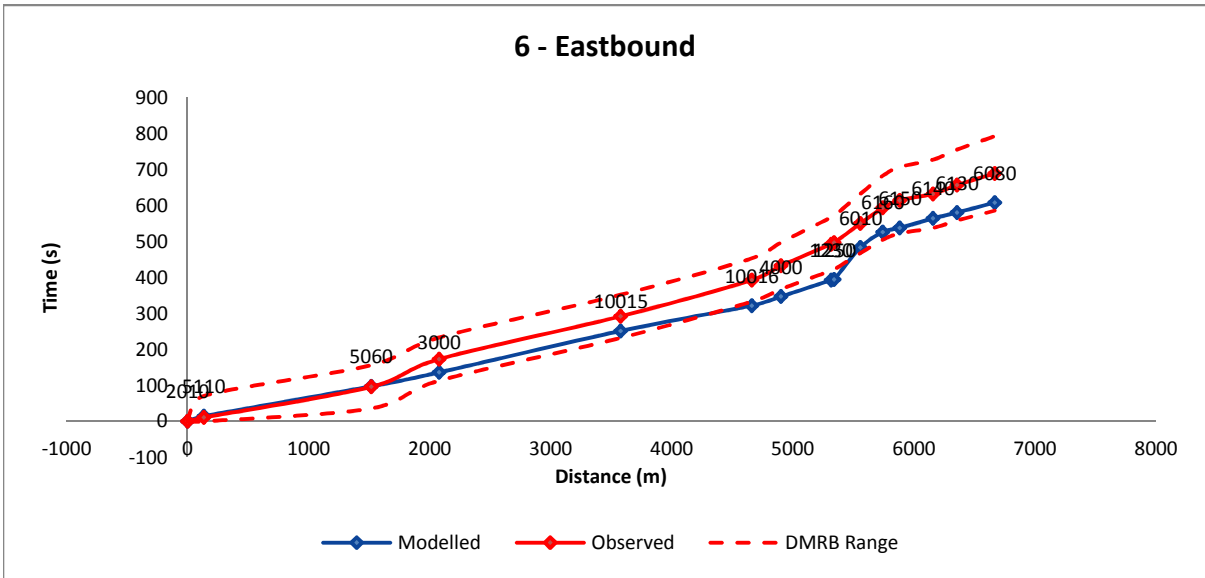
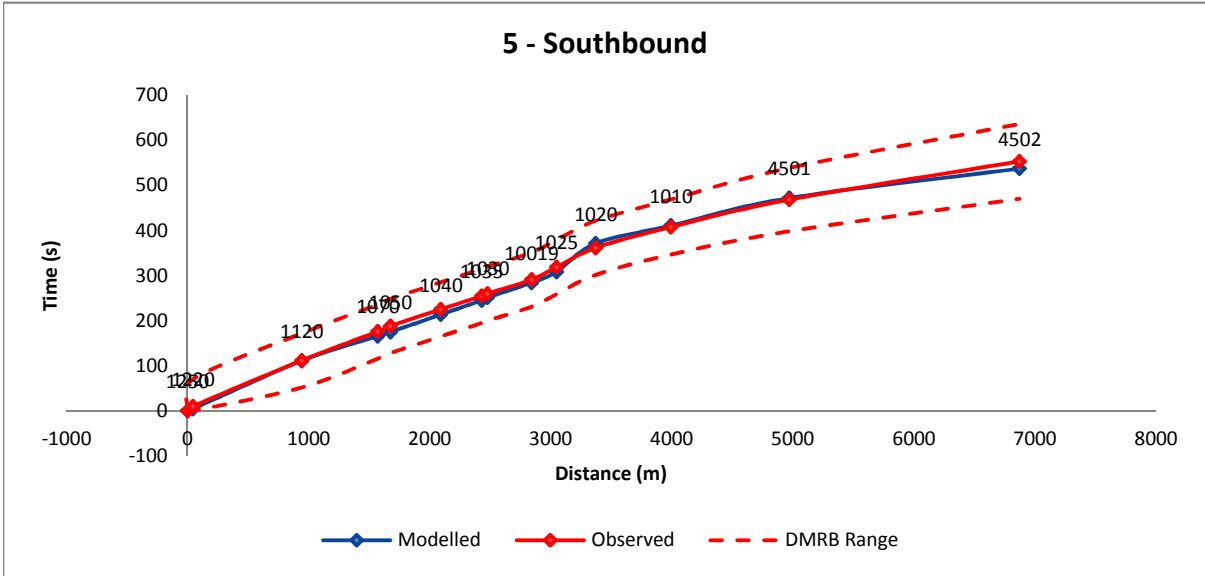
Lowestoft Journey Time Graphs - AM Peak



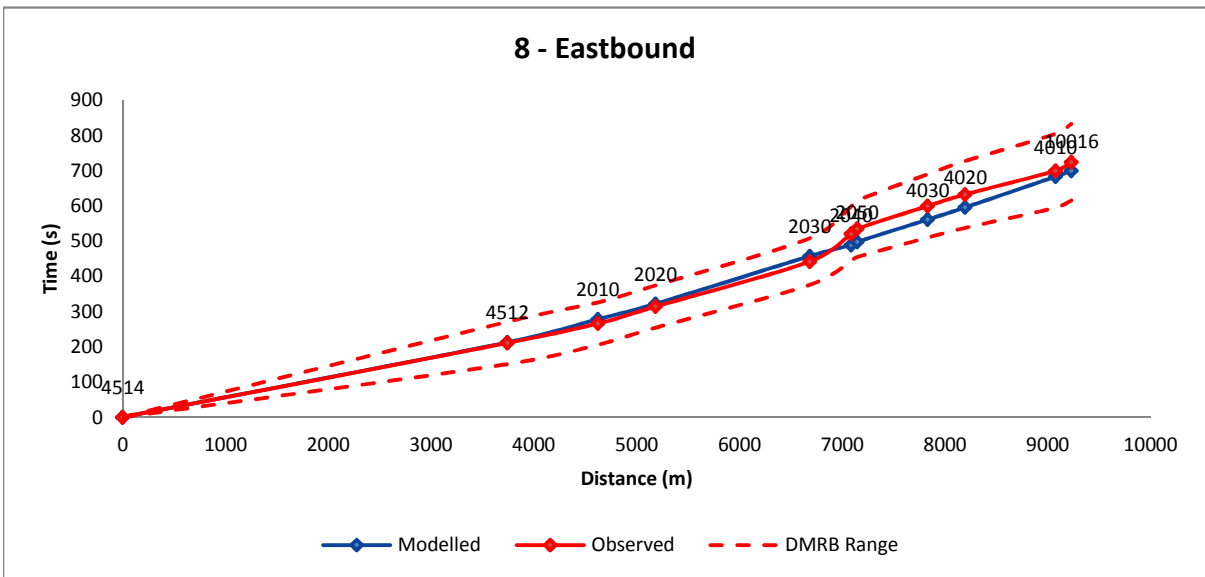
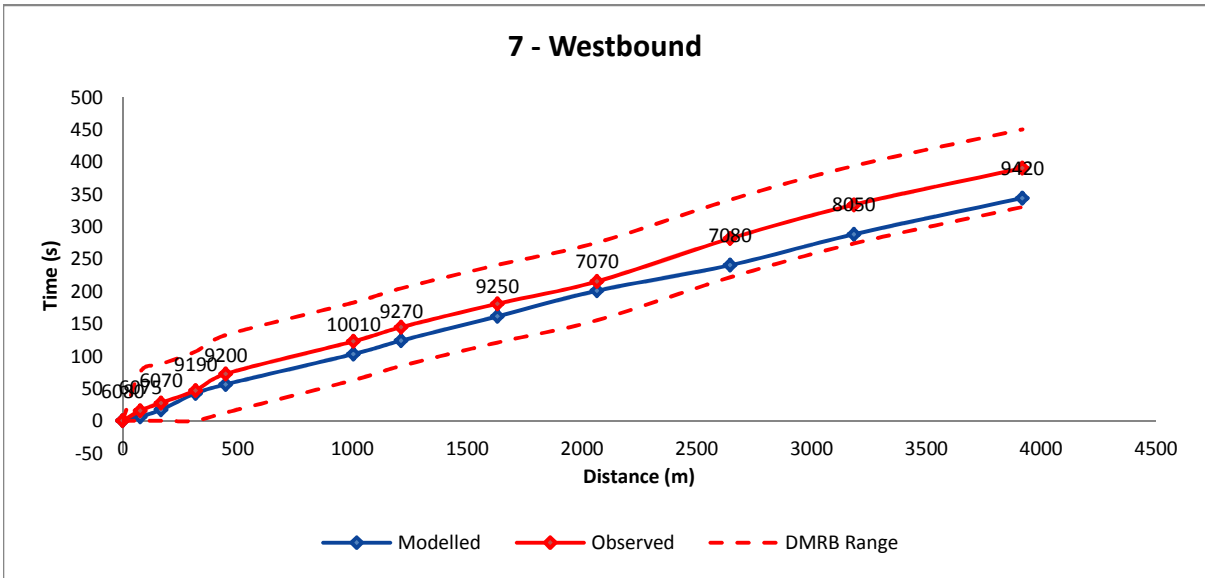
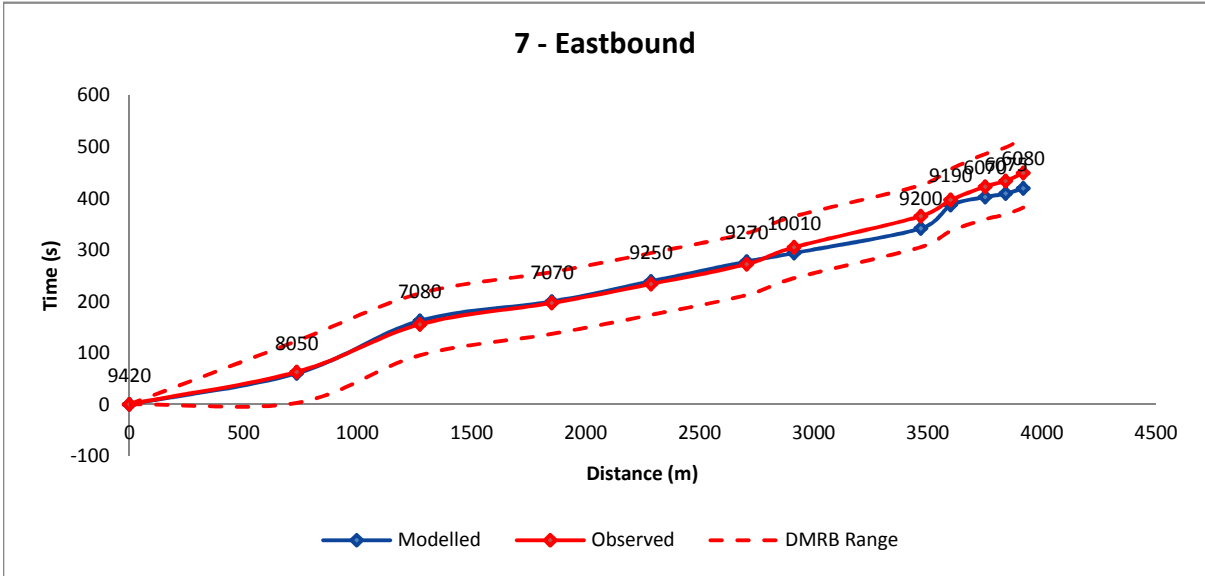
Lowestoft Journey Time Graphs - AM Peak



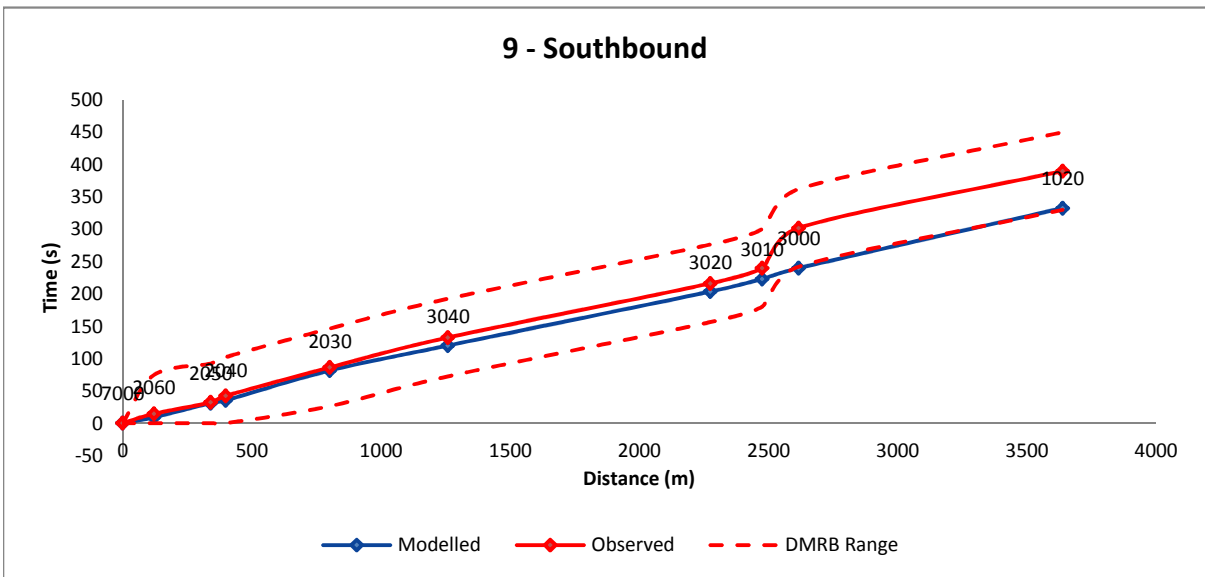
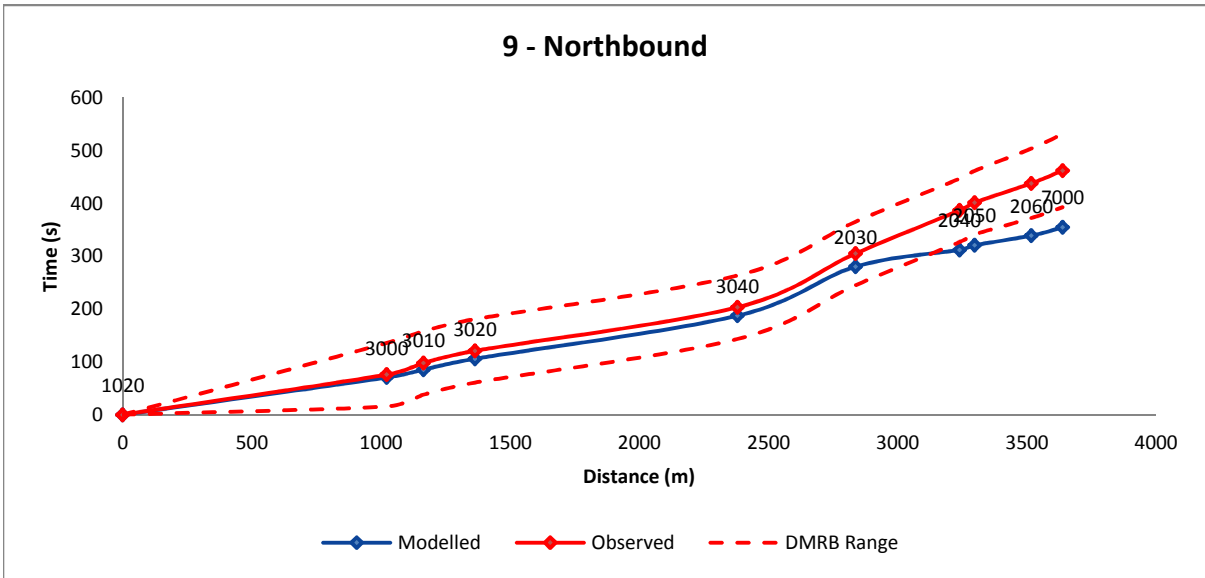
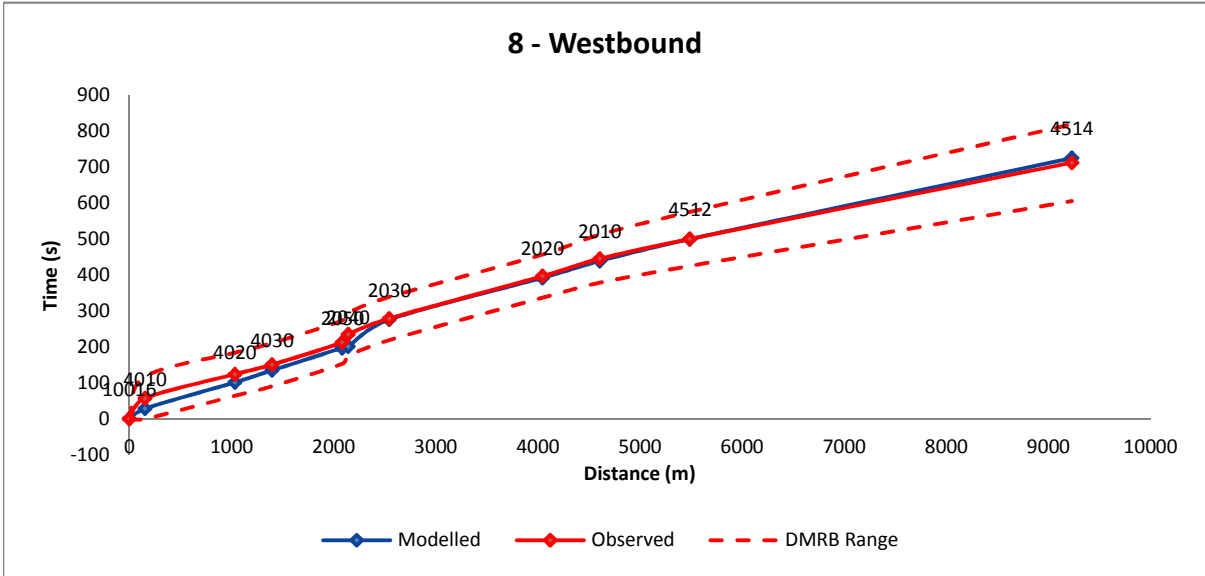
Lowestoft Journey Time Graphs - AM Peak



Lowestoft Journey Time Graphs - AM Peak

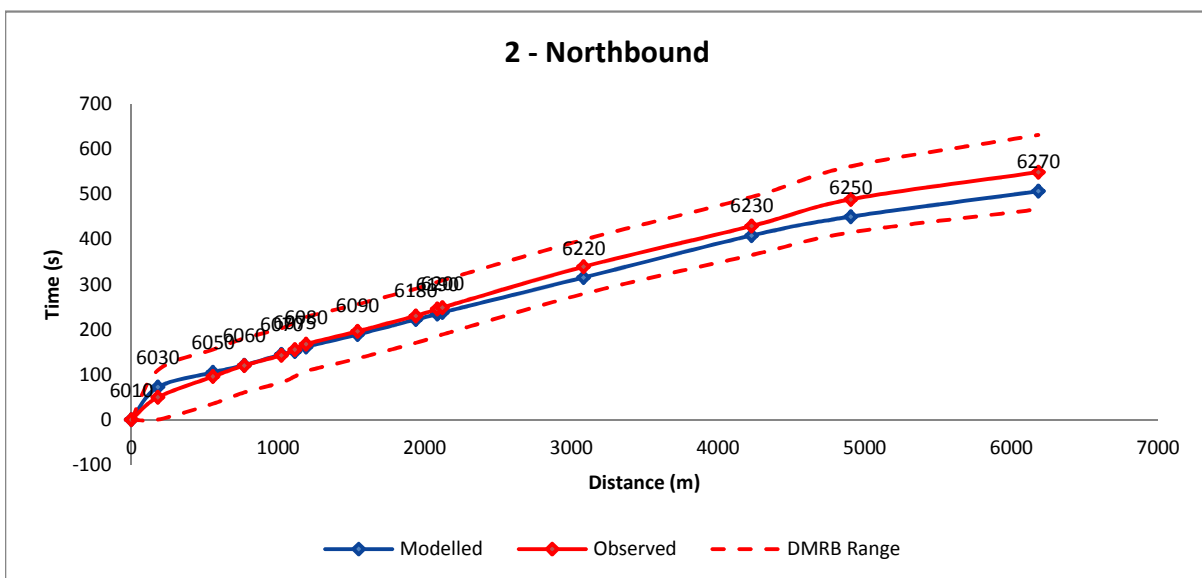
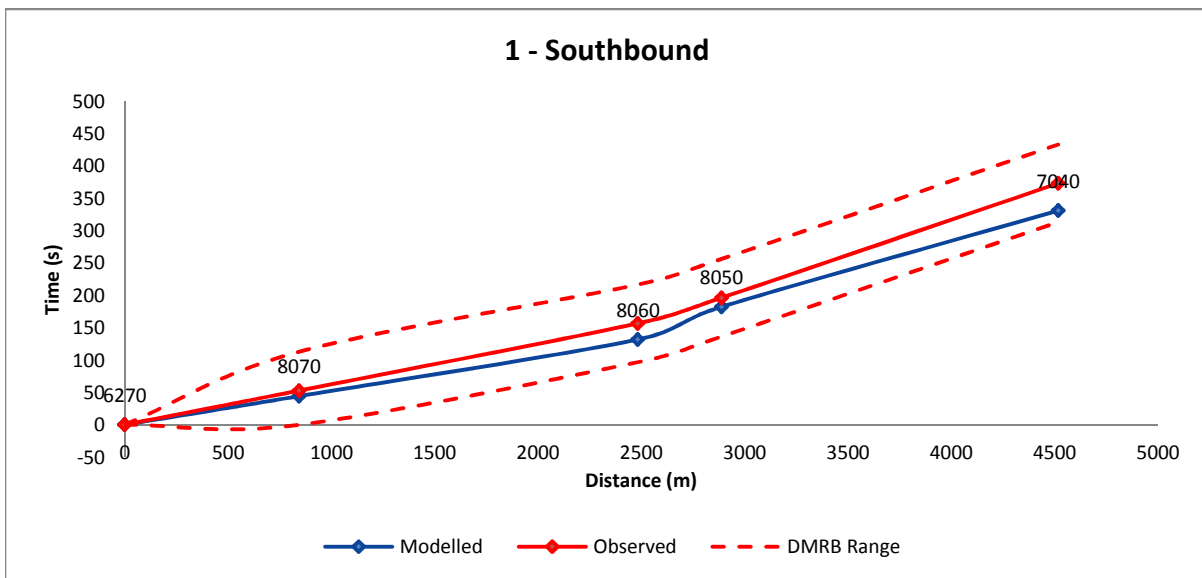
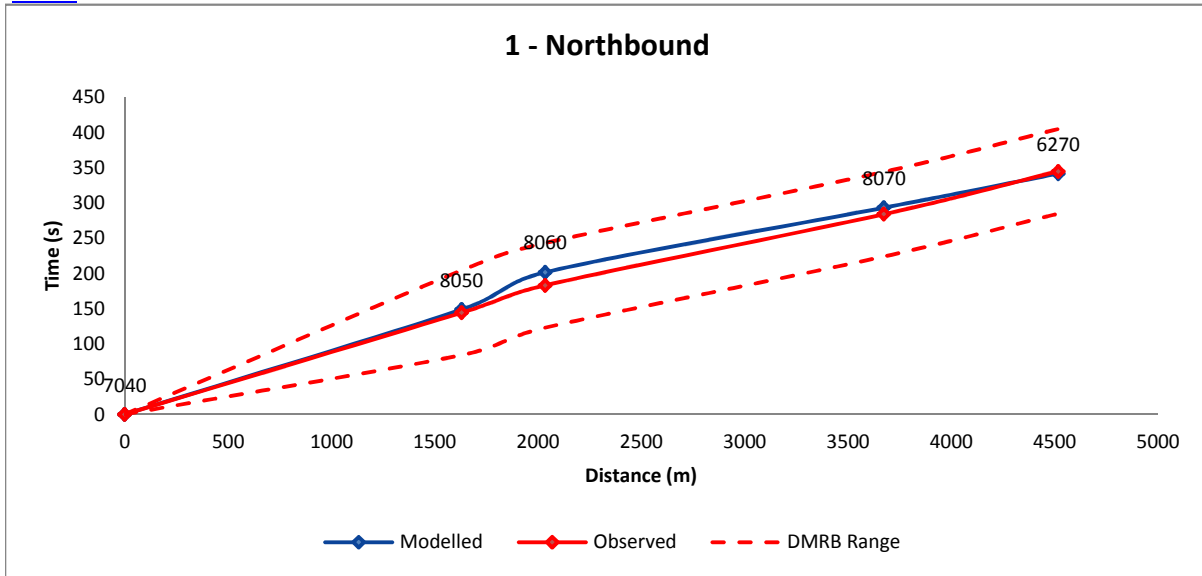


Lowestoft Journey Time Graphs - AM Peak

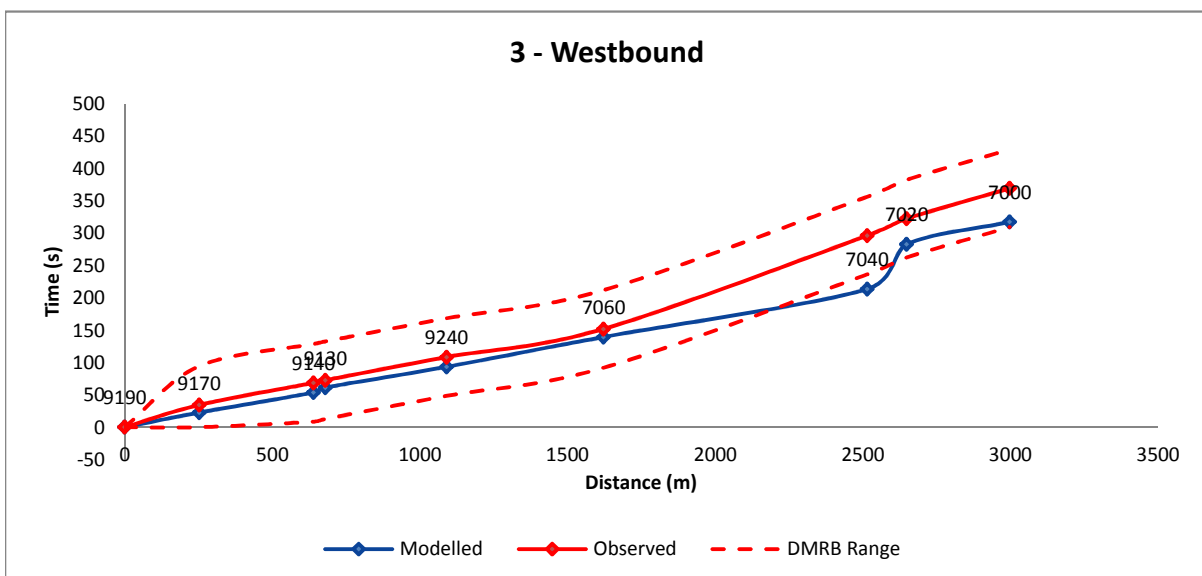
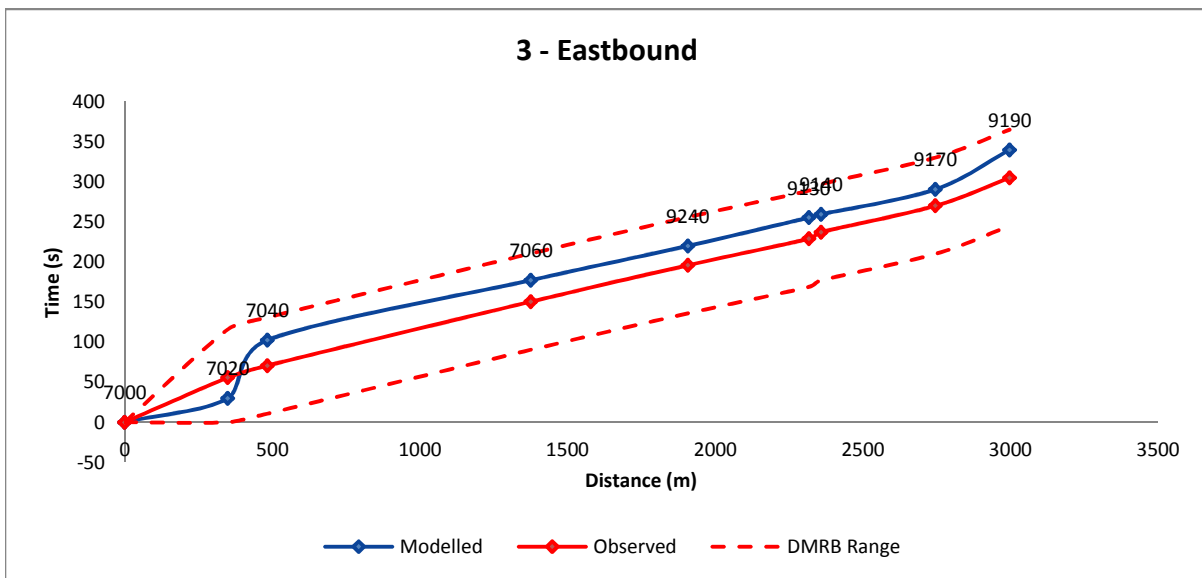
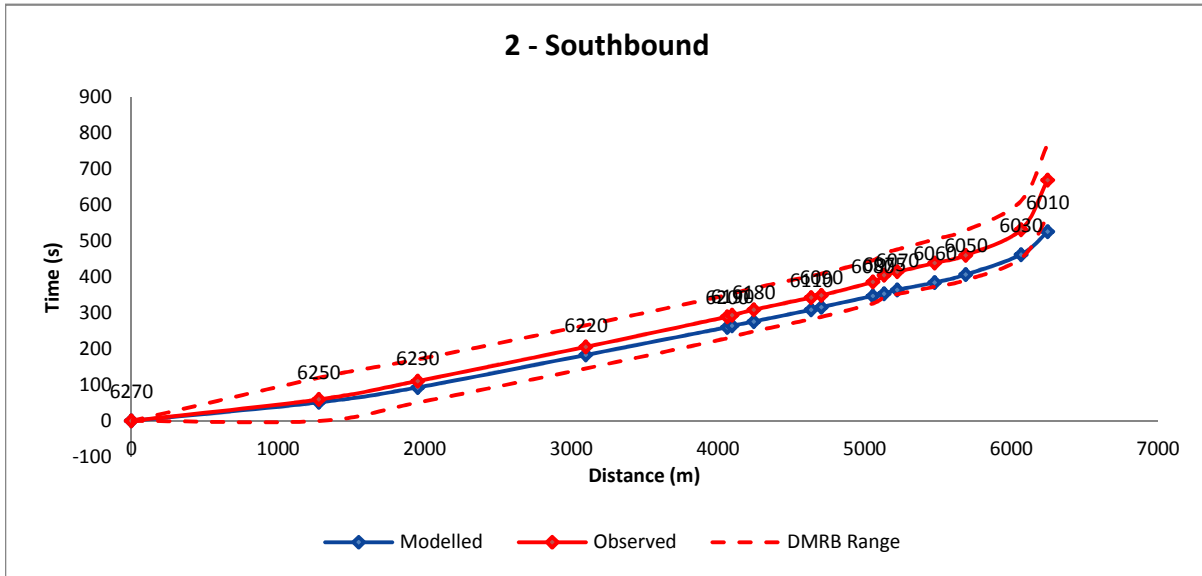


Lowestoft Journey Time Graphs - Interpeak

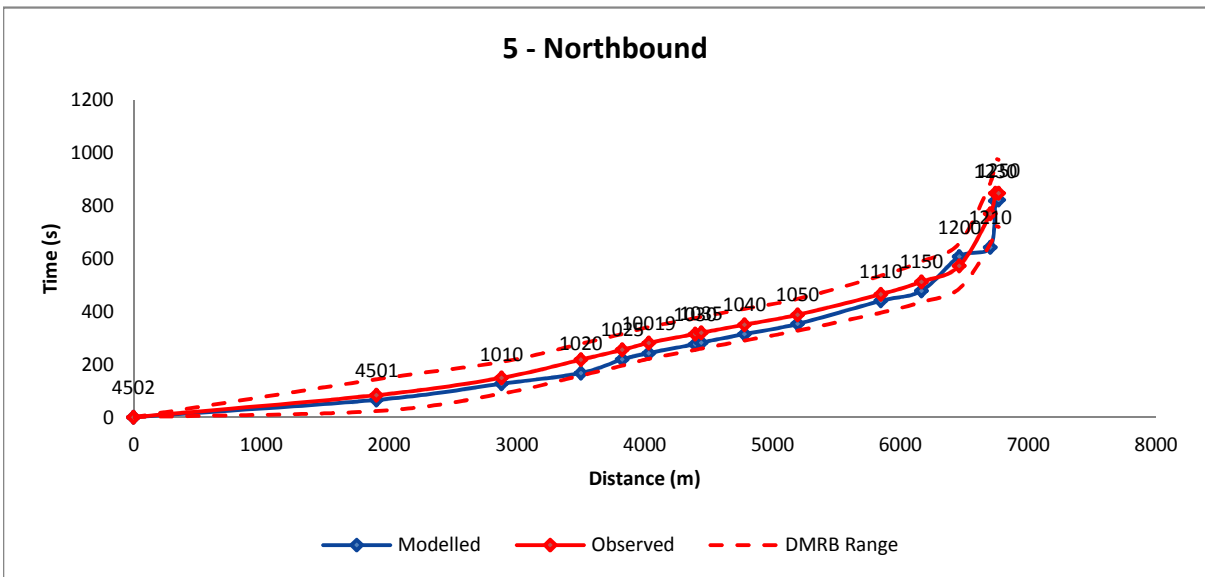
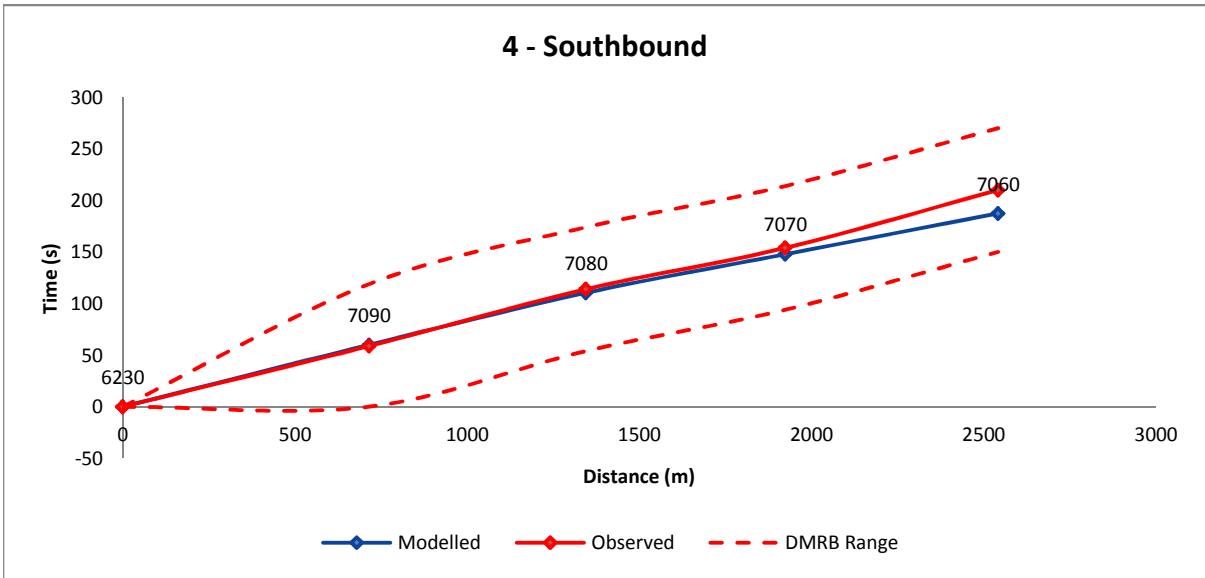
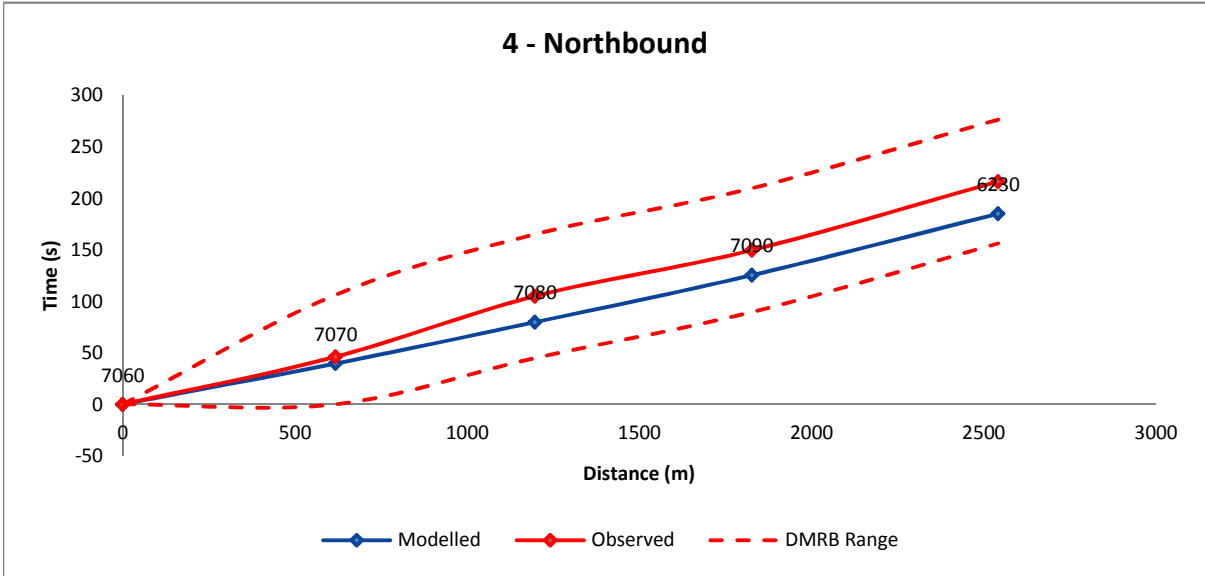
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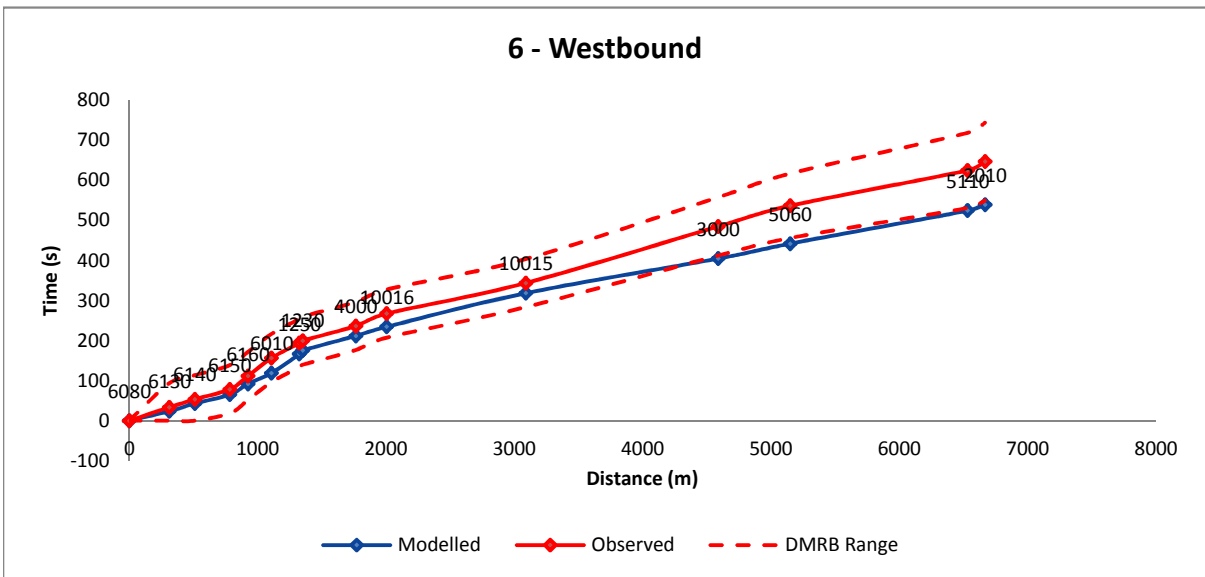
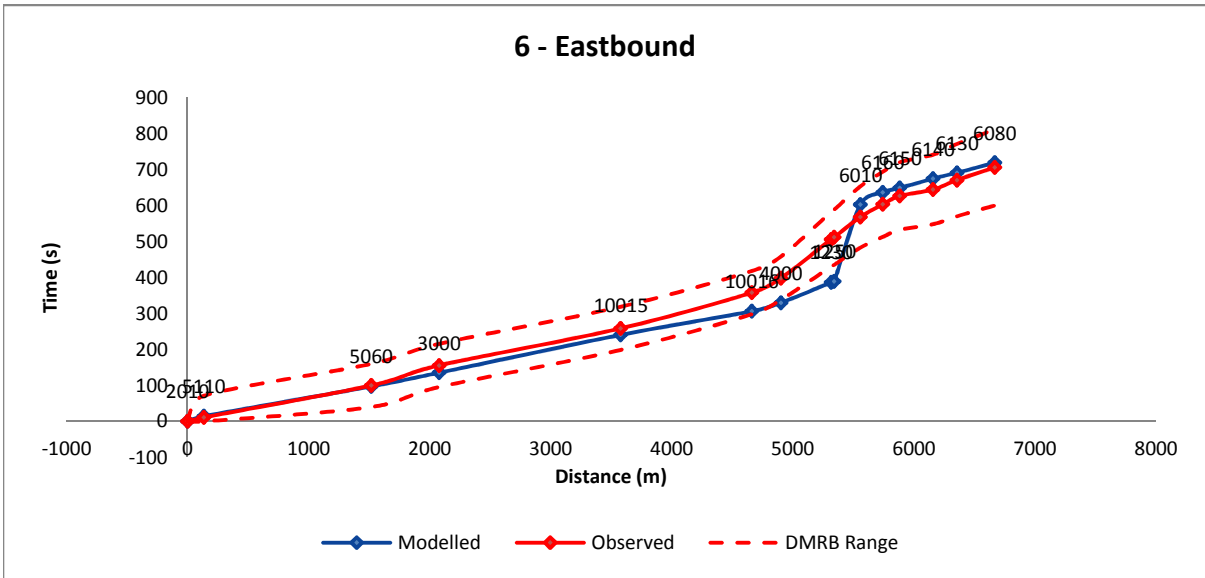
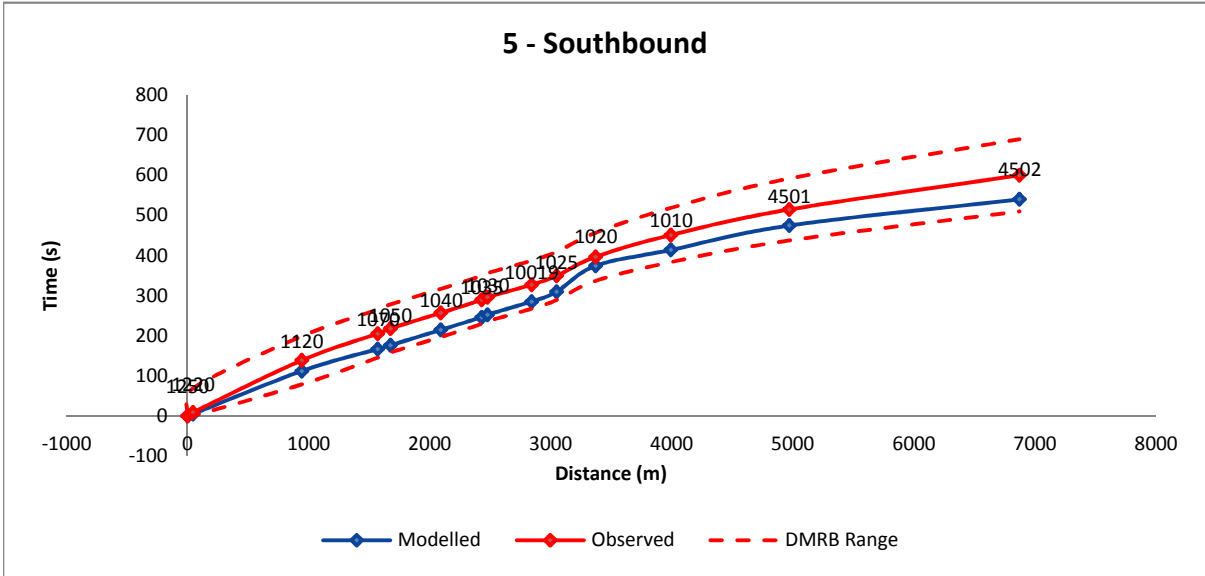
Lowestoft Journey Time Graphs - Interpeak



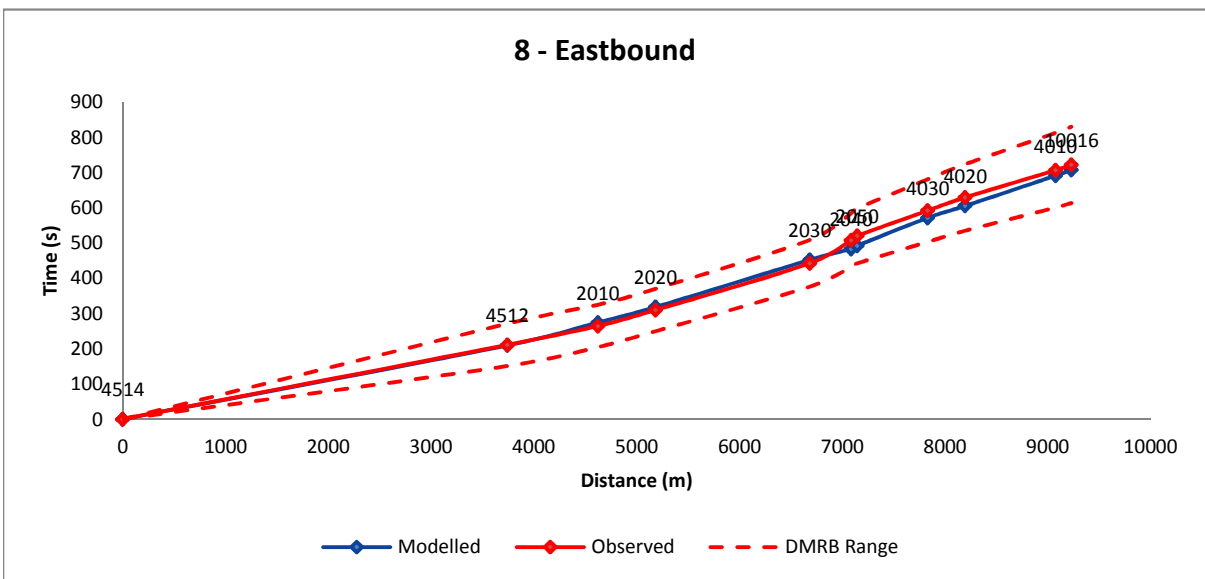
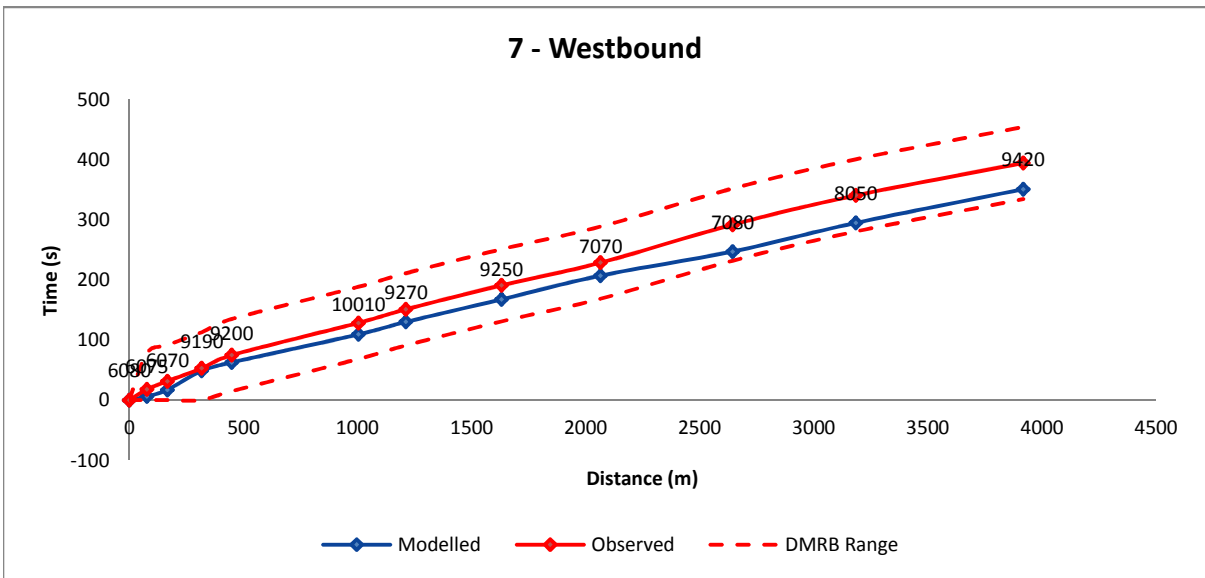
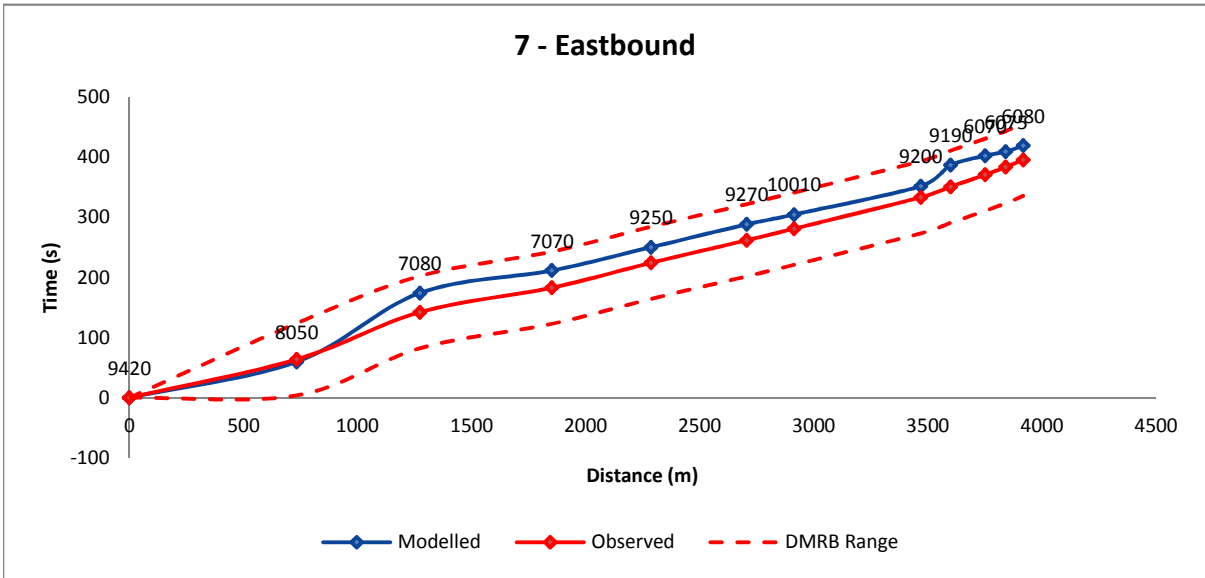
Lowestoft Journey Time Graphs - Interpeak



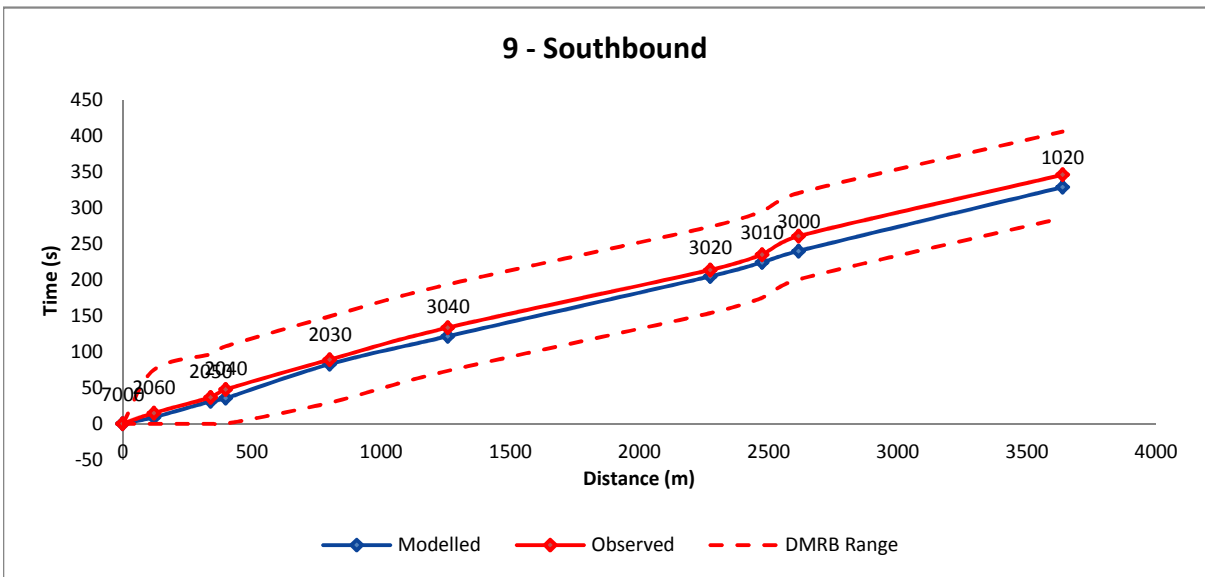
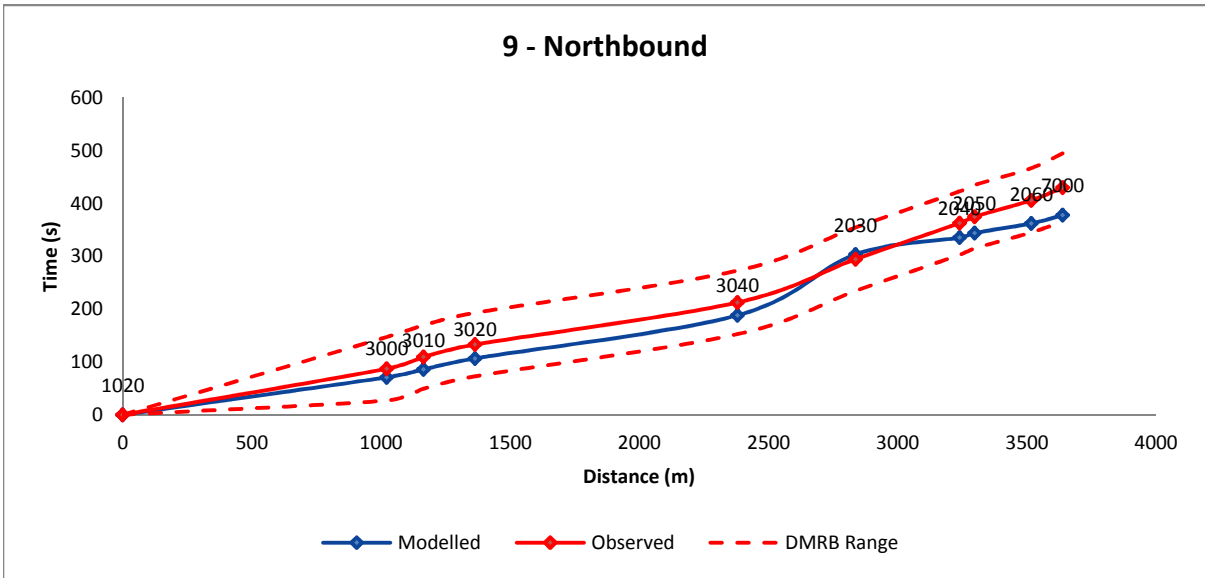
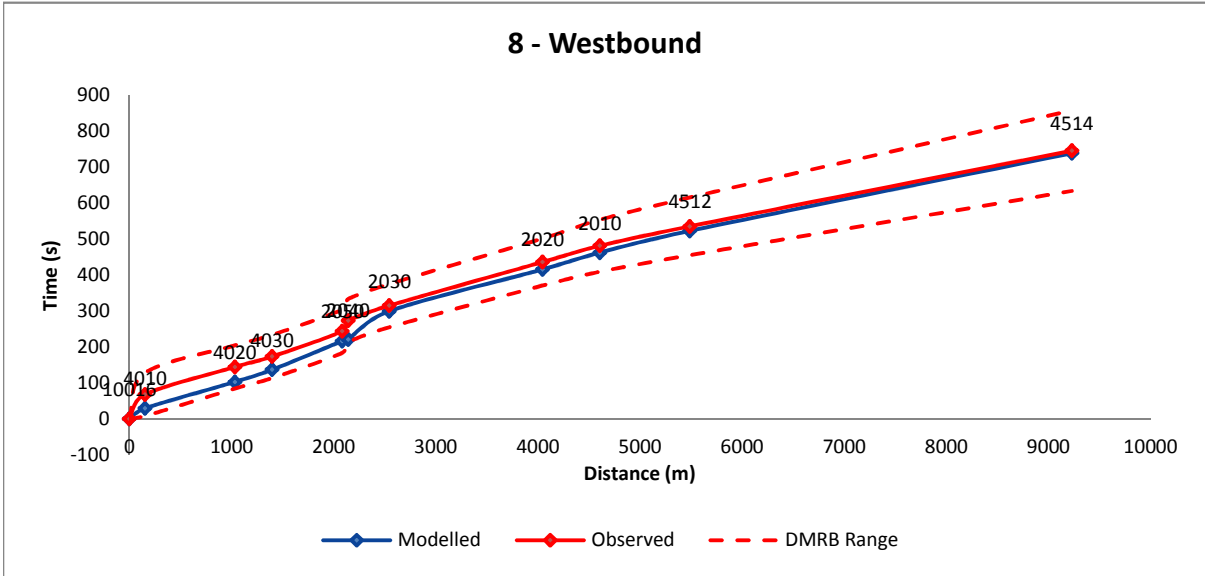
Lowestoft Journey Time Graphs - Interpeak



Lowestoft Journey Time Graphs - Interpeak

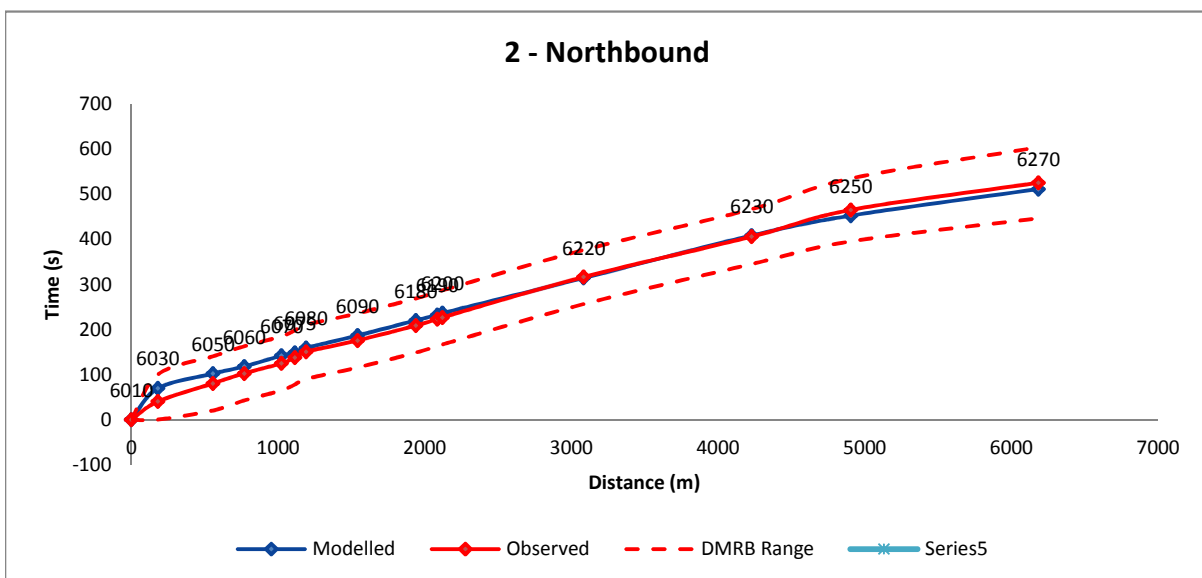
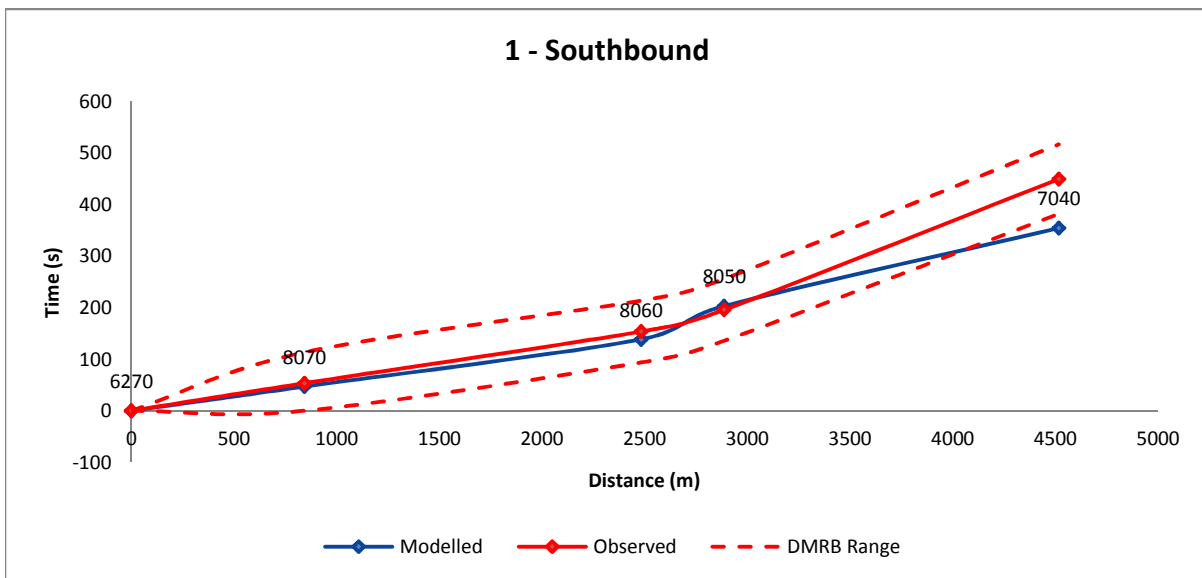
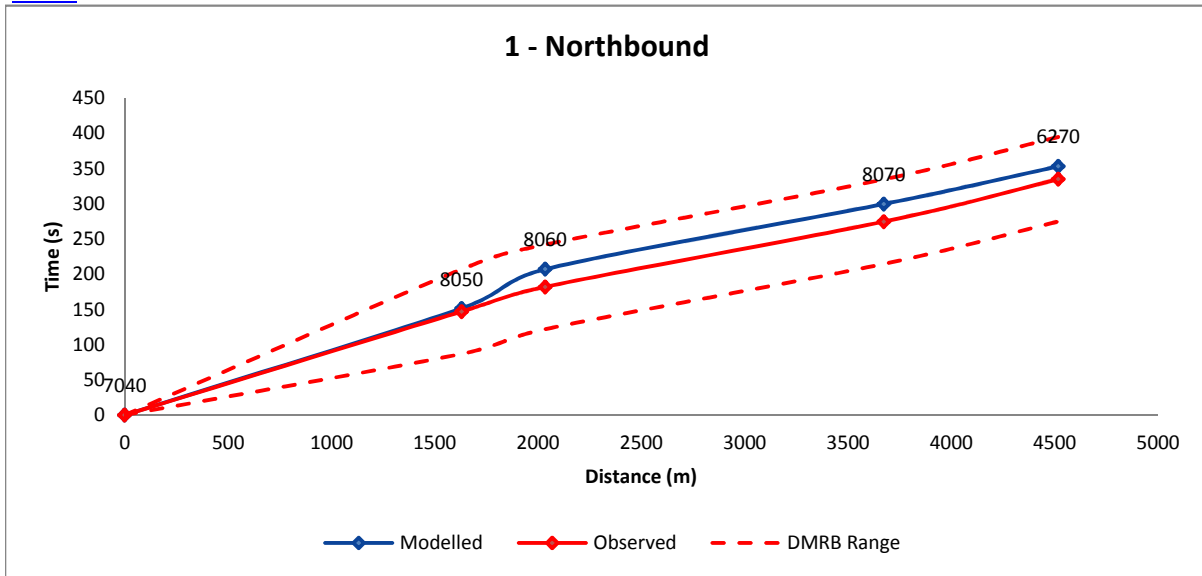


Lowestoft Journey Time Graphs - Interpeak

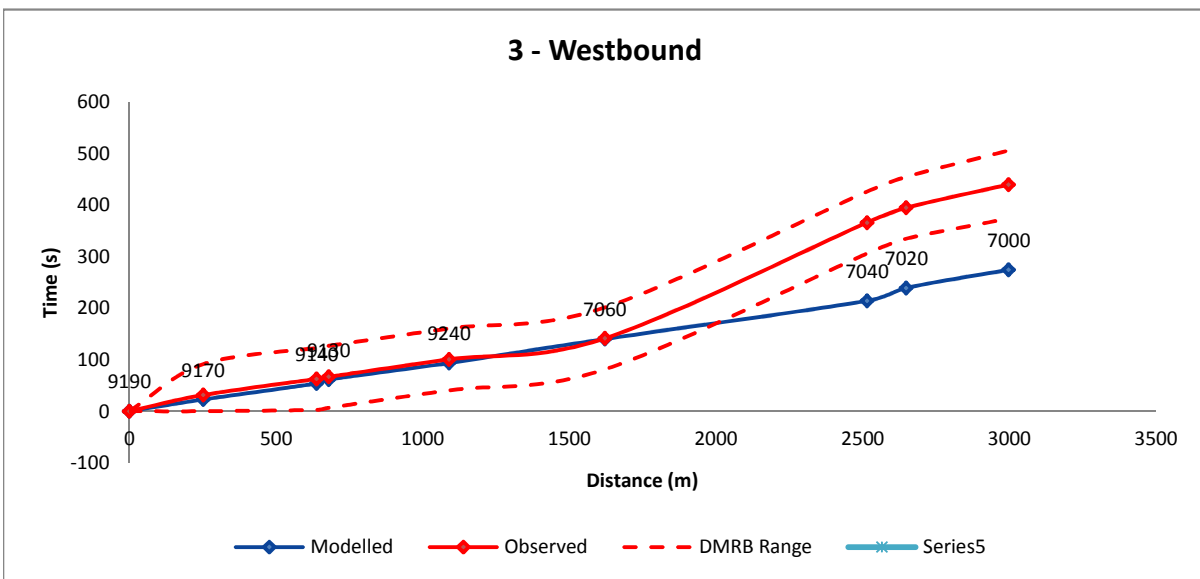
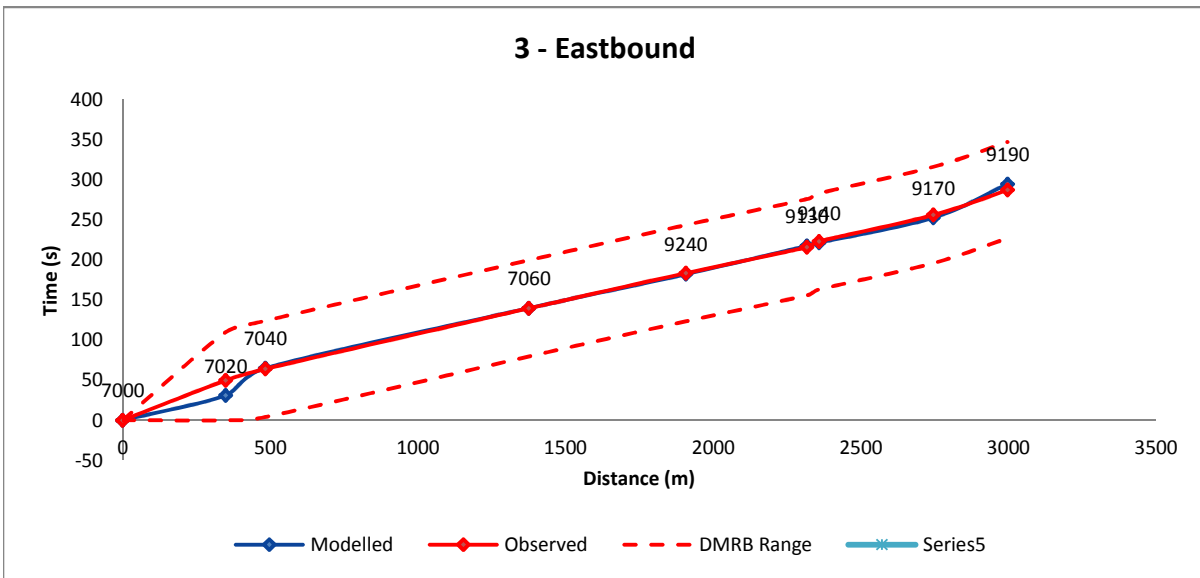
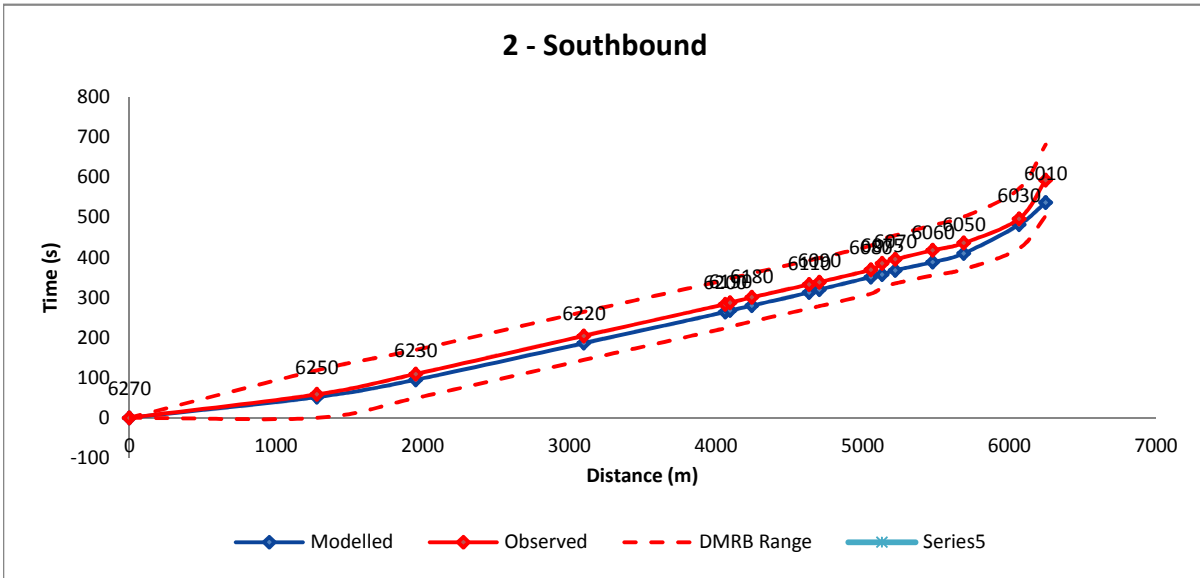


Lowestoft Journey Time Graphs - PM Peak

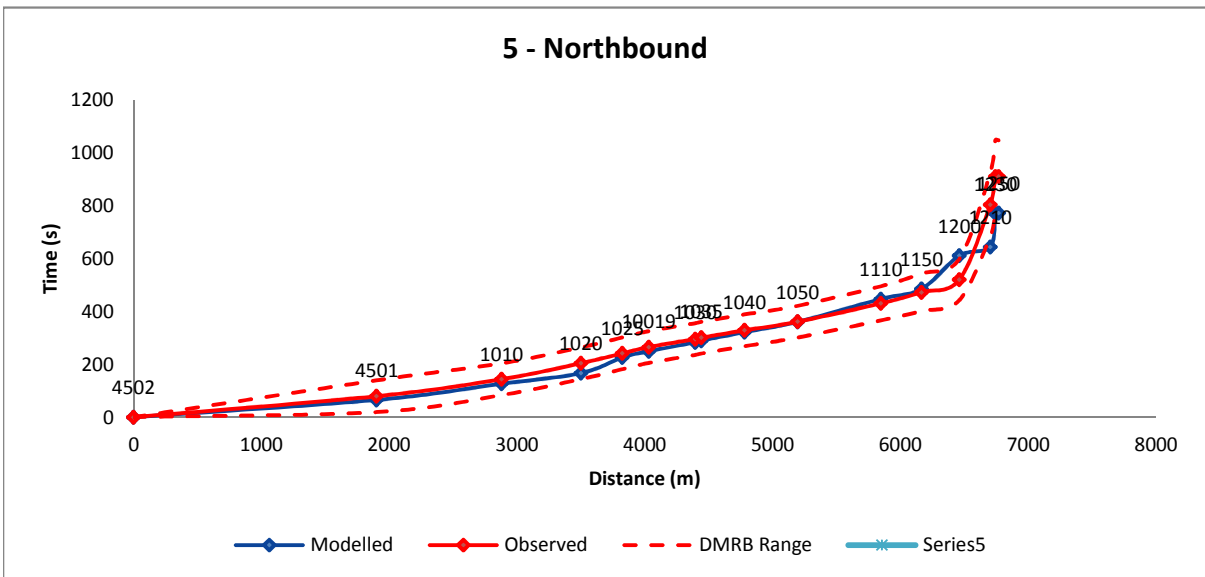
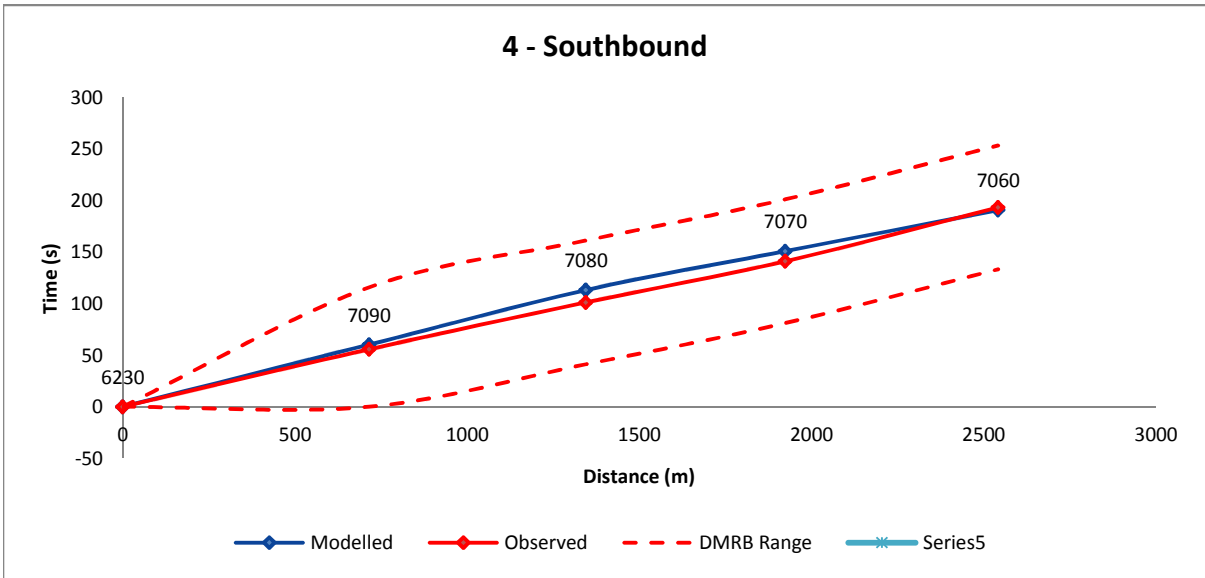
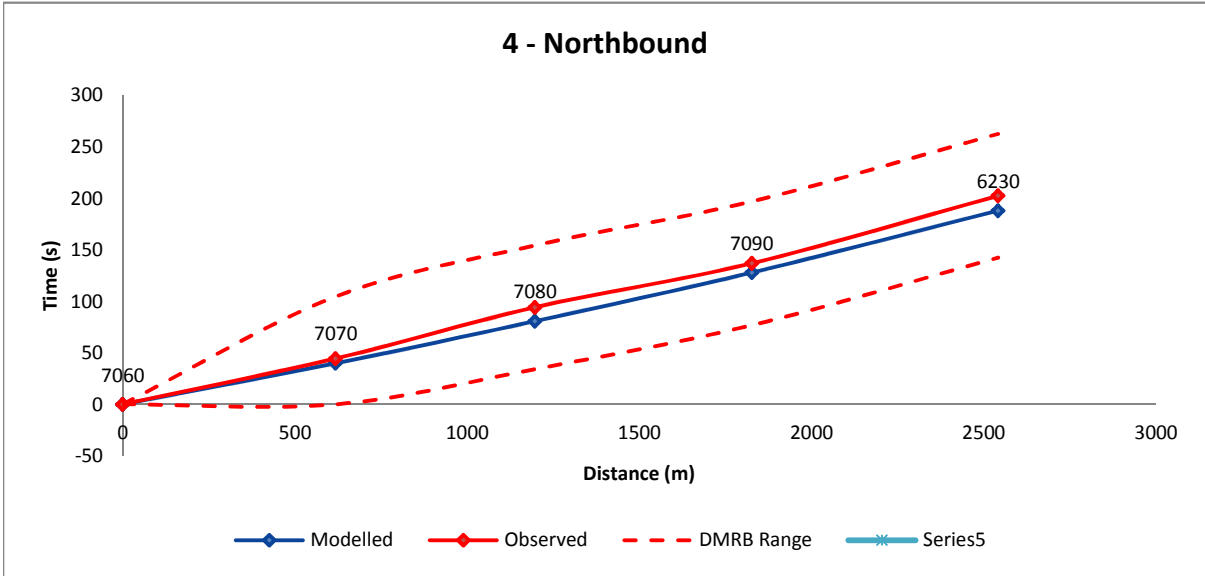
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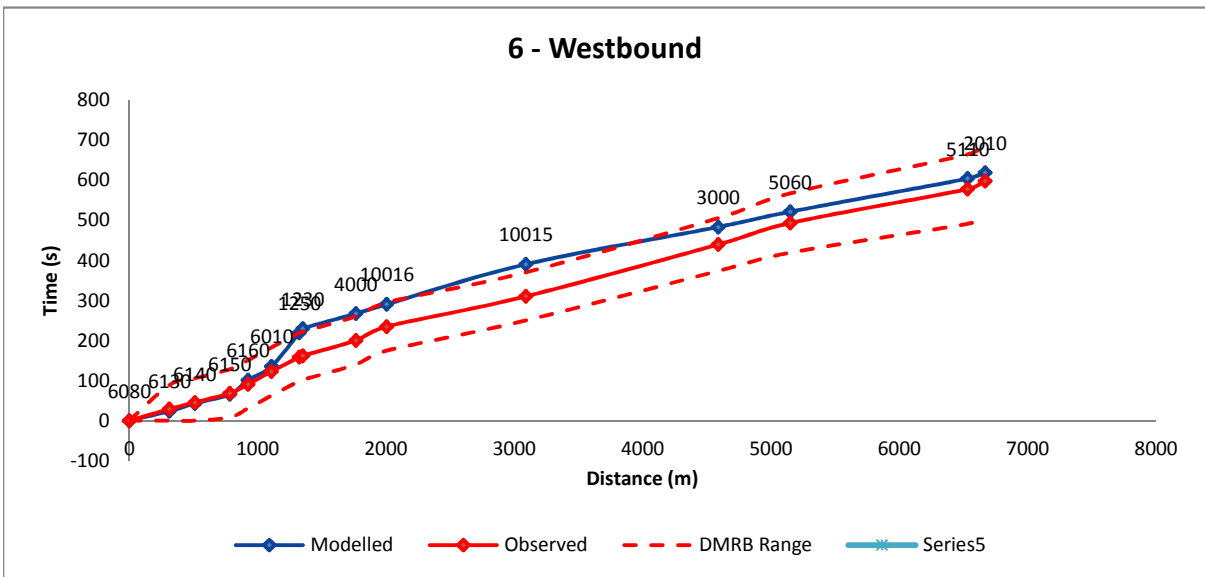
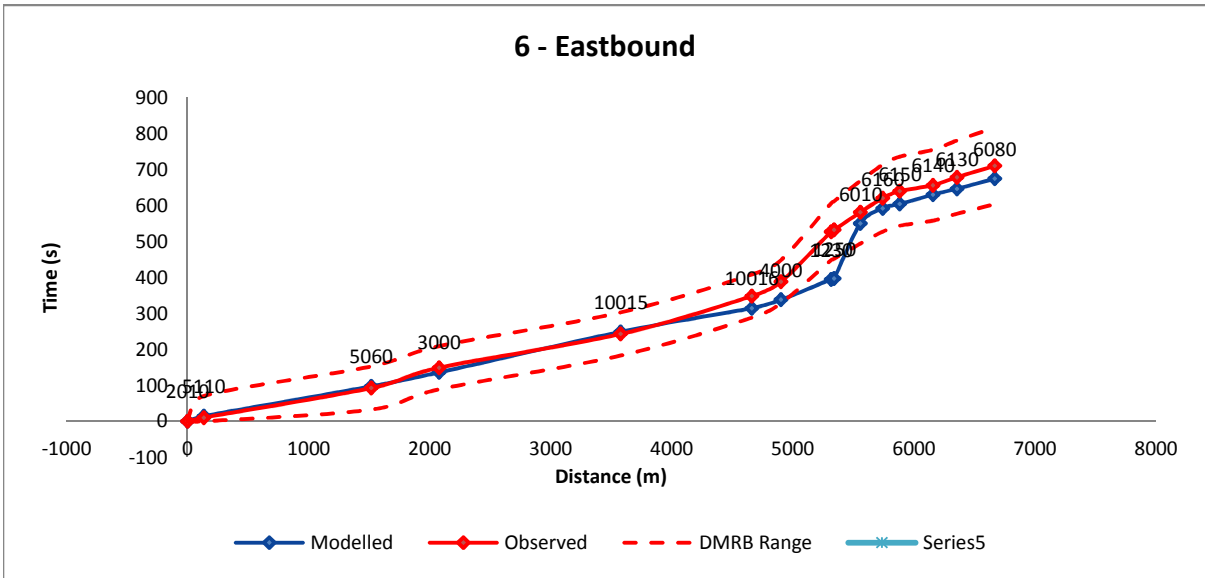
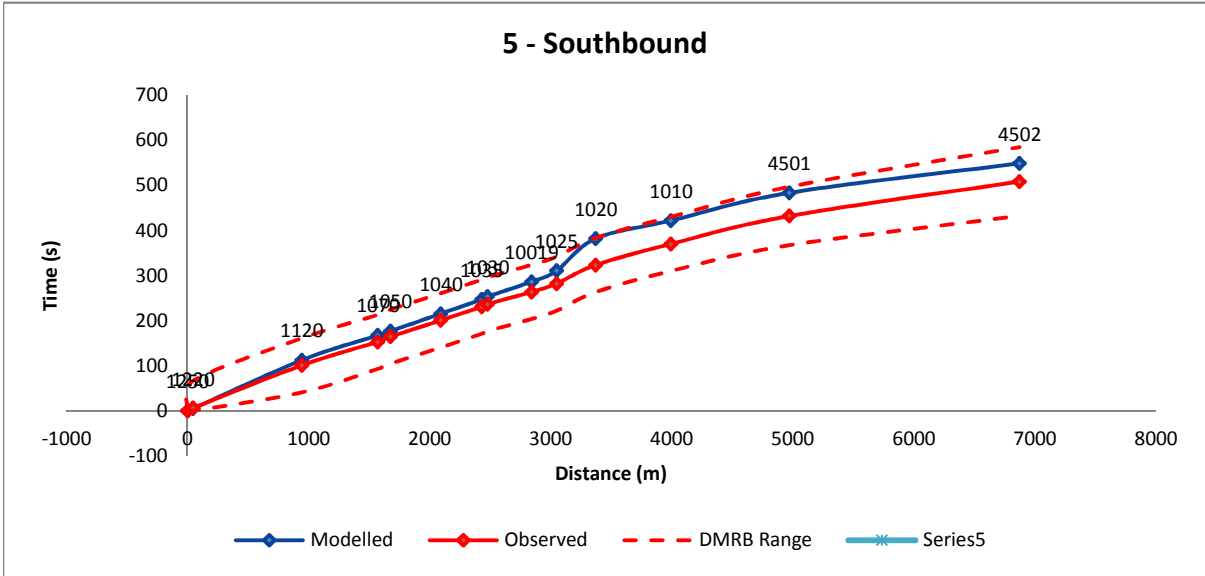
Lowestoft Journey Time Graphs - PM Peak



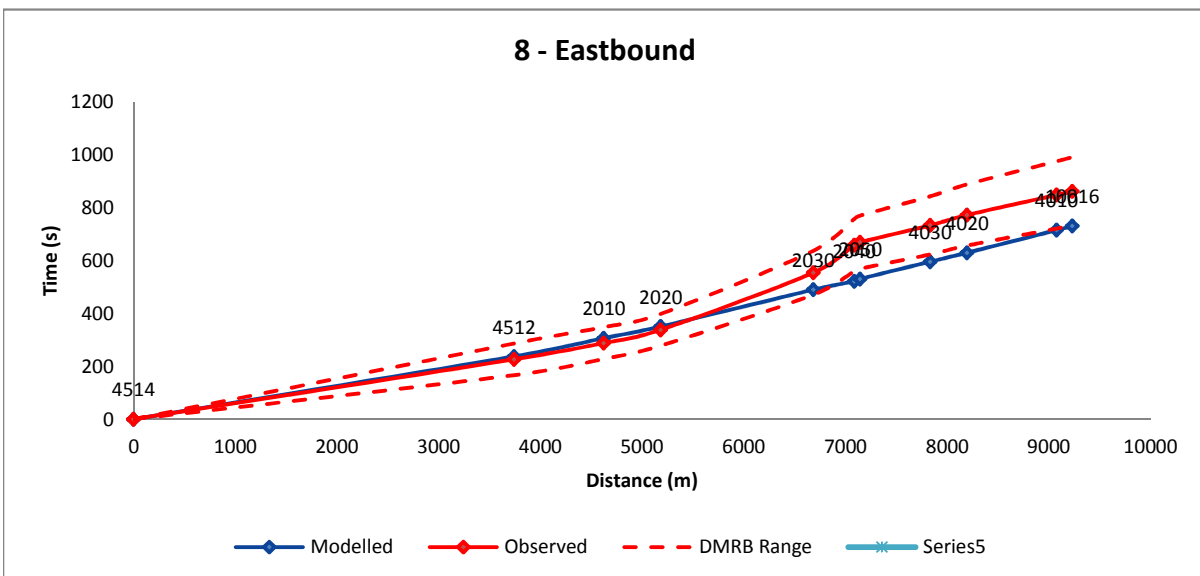
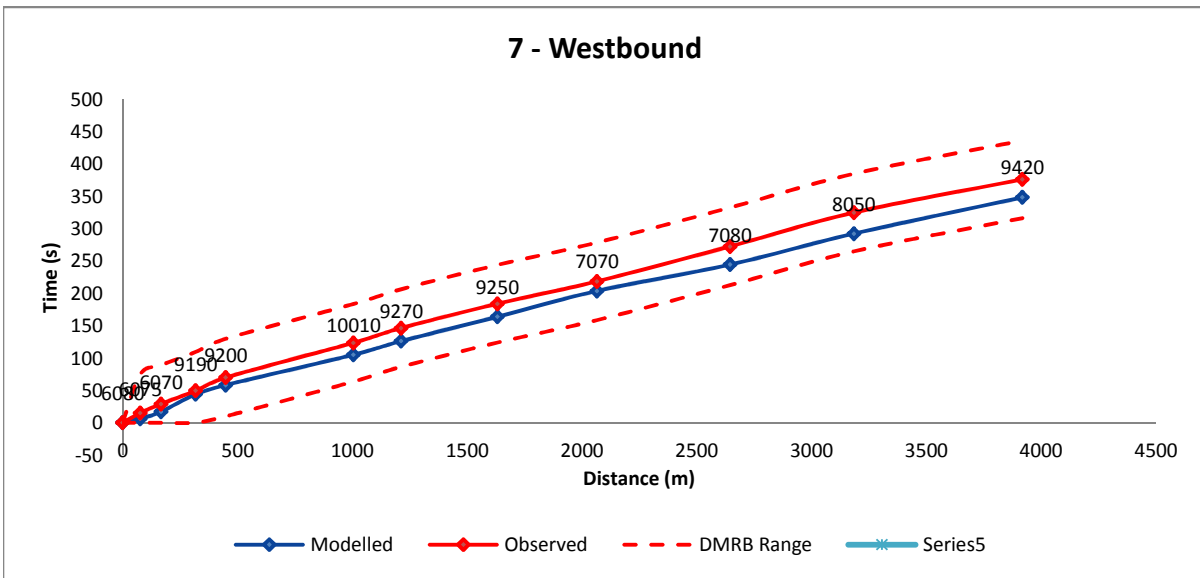
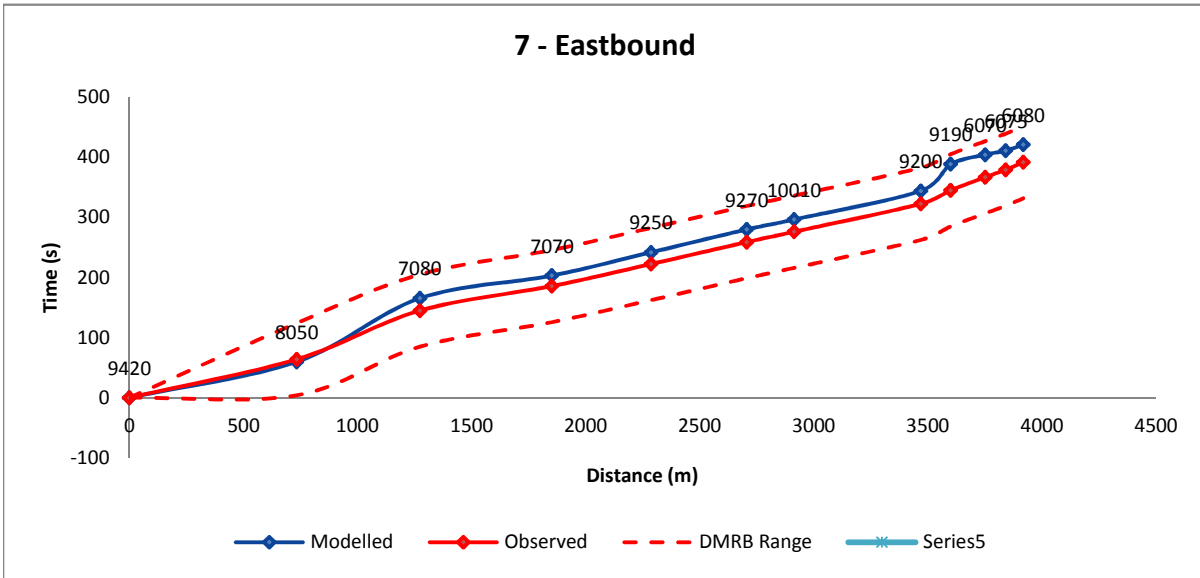
Lowestoft Journey Time Graphs - PM Peak



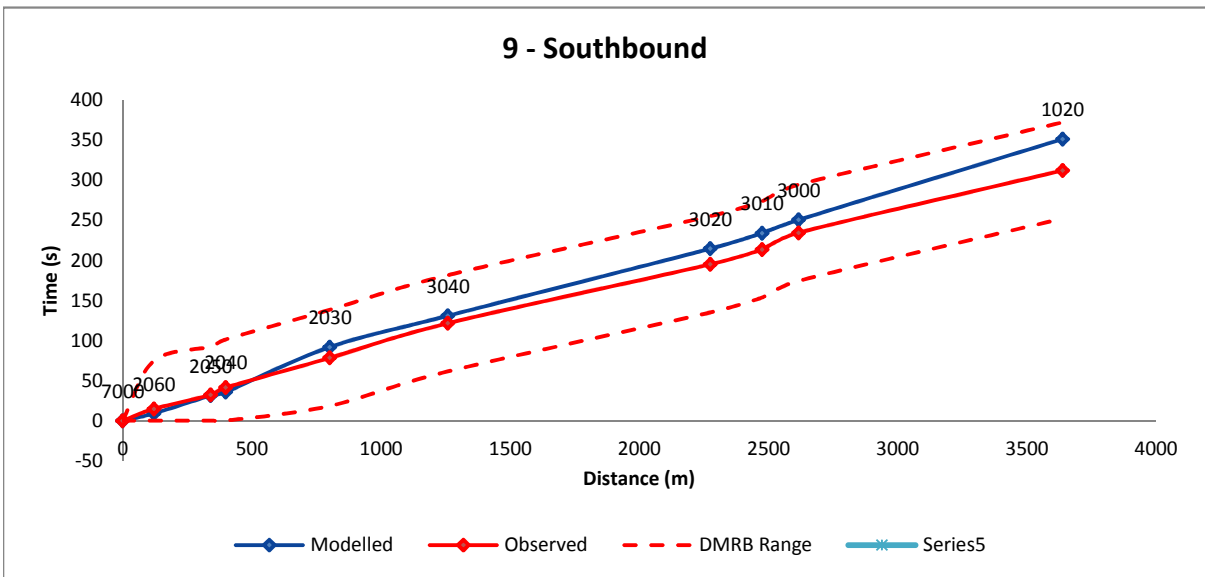
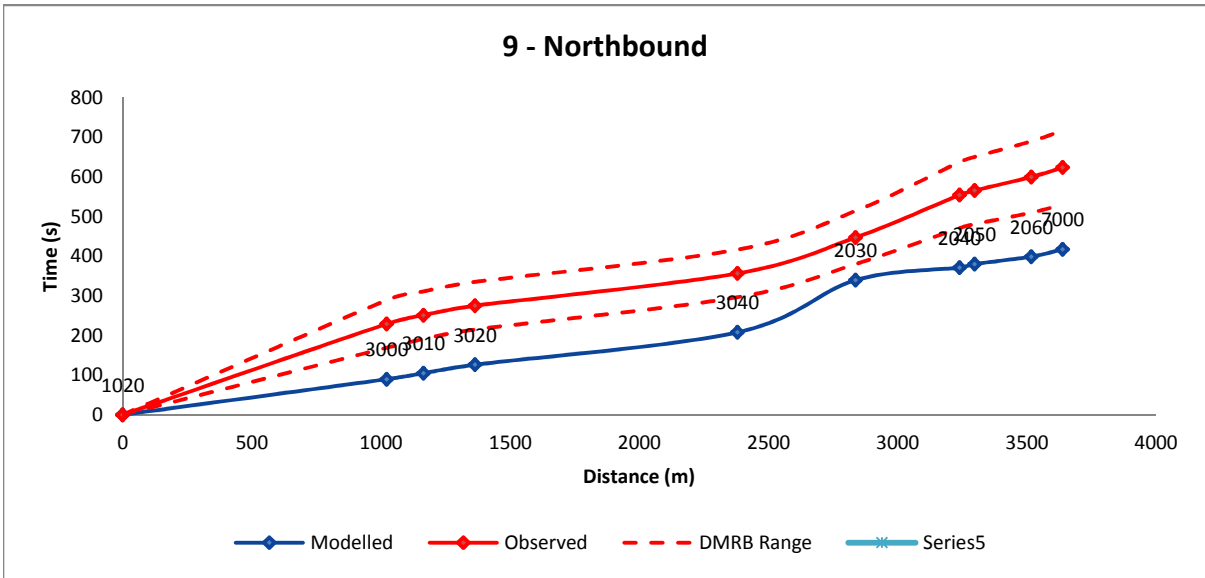
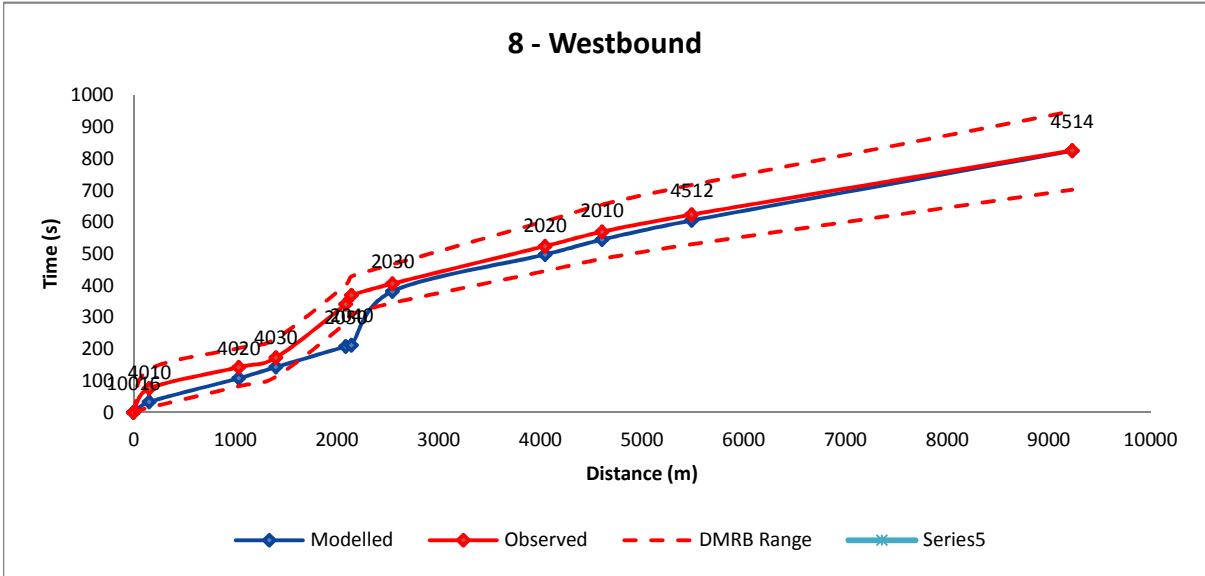
Lowestoft Journey Time Graphs - PM Peak



Lowestoft Journey Time Graphs - PM Peak



Lowestoft Journey Time Graphs - PM Peak



Appendix G

FLOW AND V/C PLOTS



Key

— Actual Flow

Contains Ordnance Survey data © Crown copyright and database right 2015.



TITLE:
**LOWESTOFT
 ACTUAL FLOW
 2015 BASE YEAR
 AM PEAK HOUR**

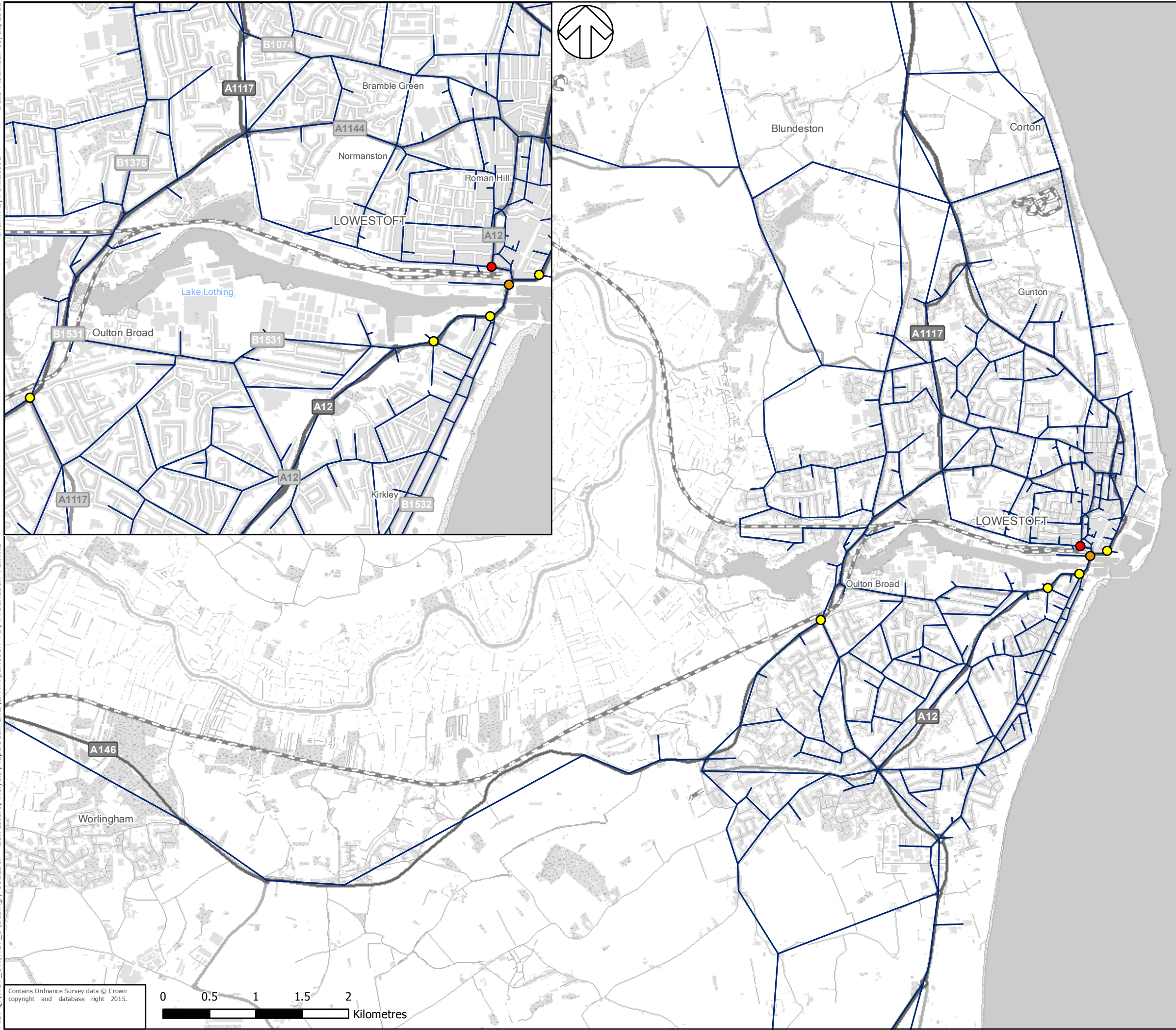
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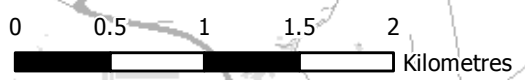
Key

V/C (%)

- 75 - 85
- 85 - 100
- > 100
- Network



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TITLE:

**LOWESTOFT
JUNCTION V/C (%)
2015 BASE YEAR
AM PEAK**

FIGURE No:



Key

— Actual Flow

Contains Ordnance Survey data © Crown copyright and database right 2015.



TITLE:
**LOWESTOFT
 ACTUAL FLOW
 2015 BASE YEAR
 IP PEAK HOUR**

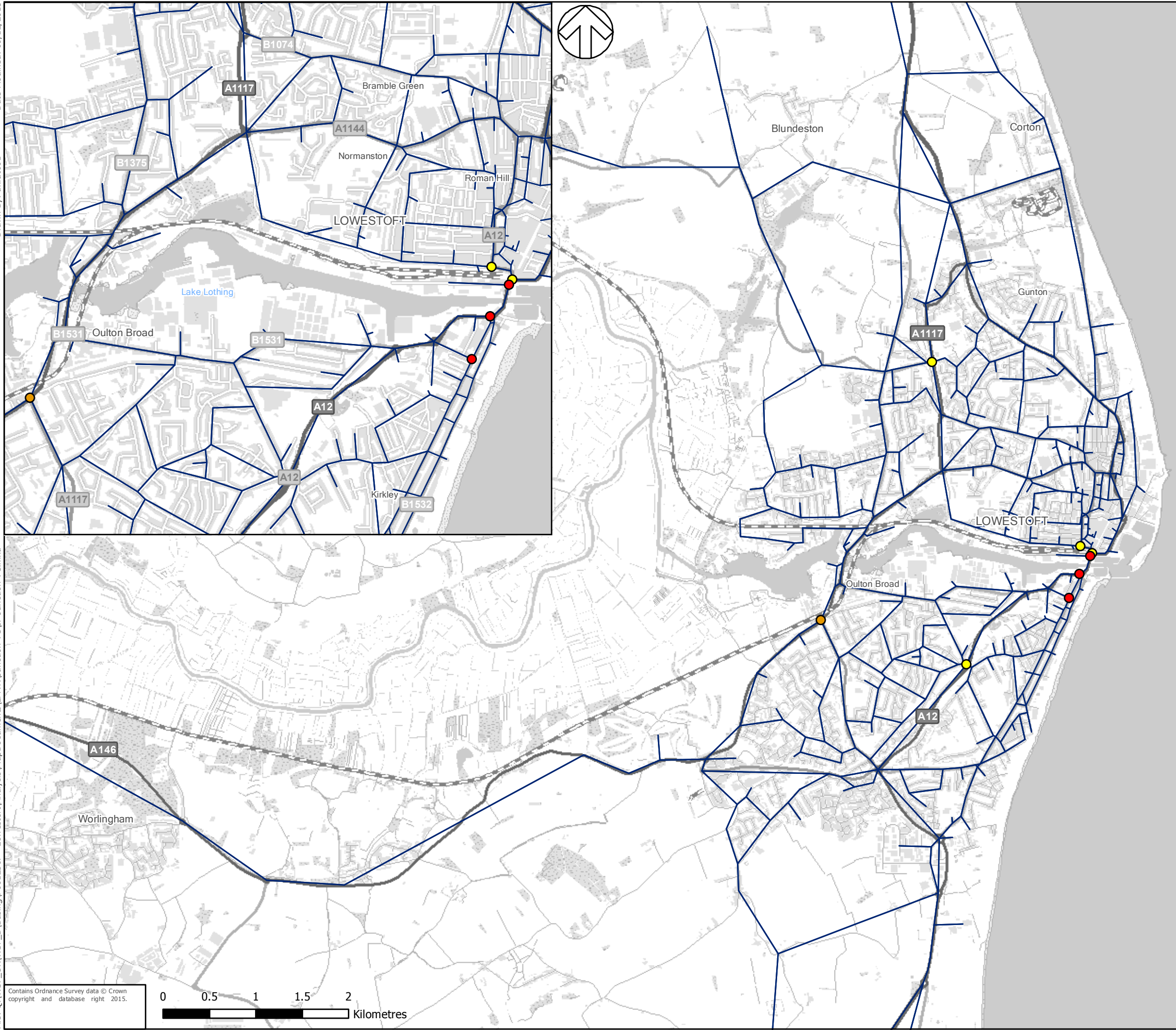
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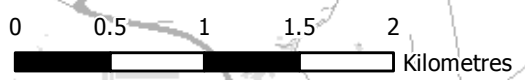
Key

V/C (%)

- 75 - 85
- 85 - 100
- > 100
- Network



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TITLE:

**LOWESTOFT
JUNCTION V/C (%)
2015 BASE YEAR
INTER PEAK**

FIGURE No:



Key

— Actual Flow

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TITLE:
**LOWESTOFT
 ACTUAL FLOW
 2015 BASE YEAR
 PM PEAK HOUR**

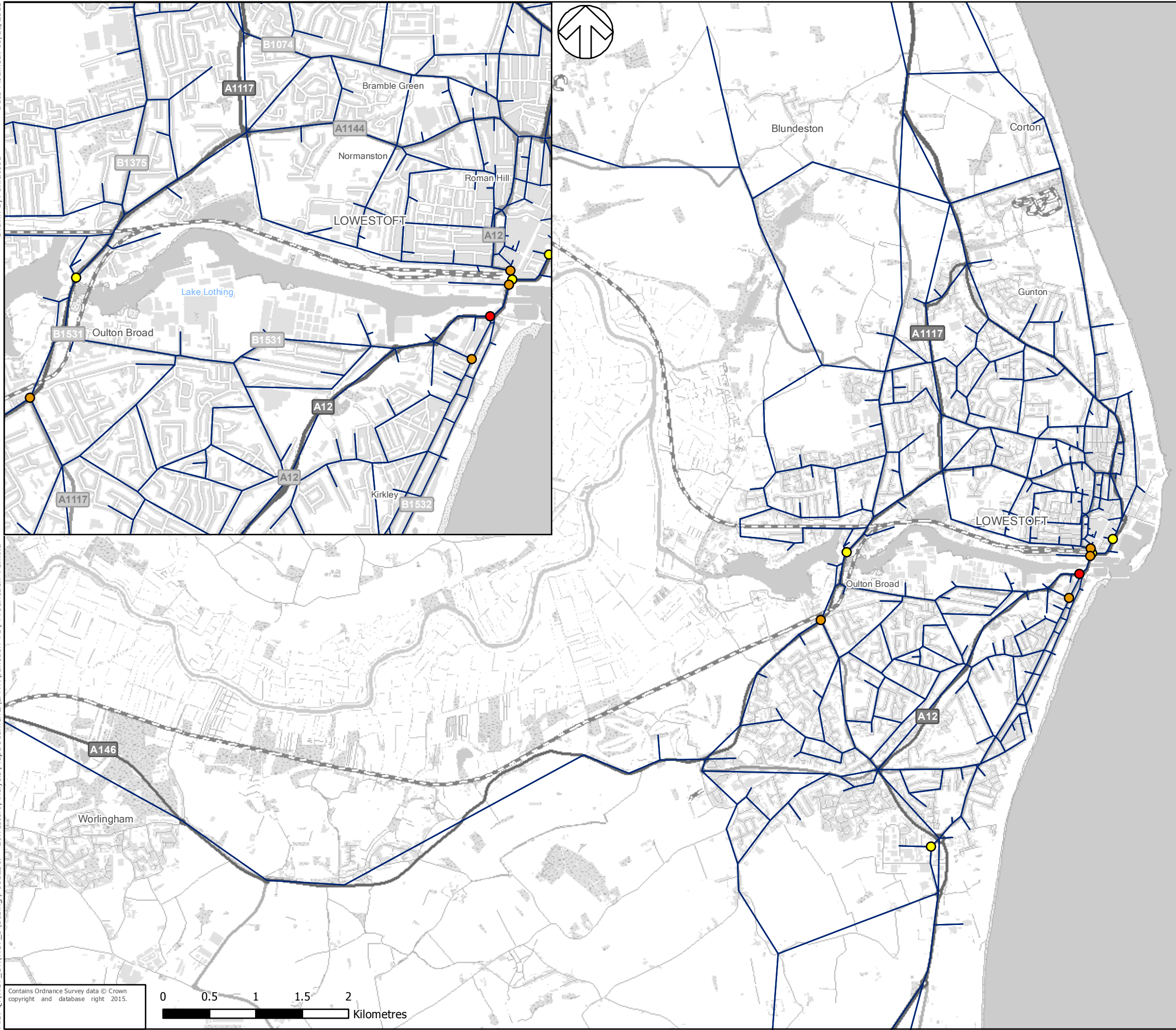
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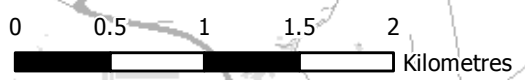
Key

V/C (%)

- 75 - 85
- 85 - 100
- > 100
- Network



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TITLE:

**LOWESTOFT
JUNCTION V/C (%)
2015 BASE YEAR
PM PEAK**

FIGURE No: